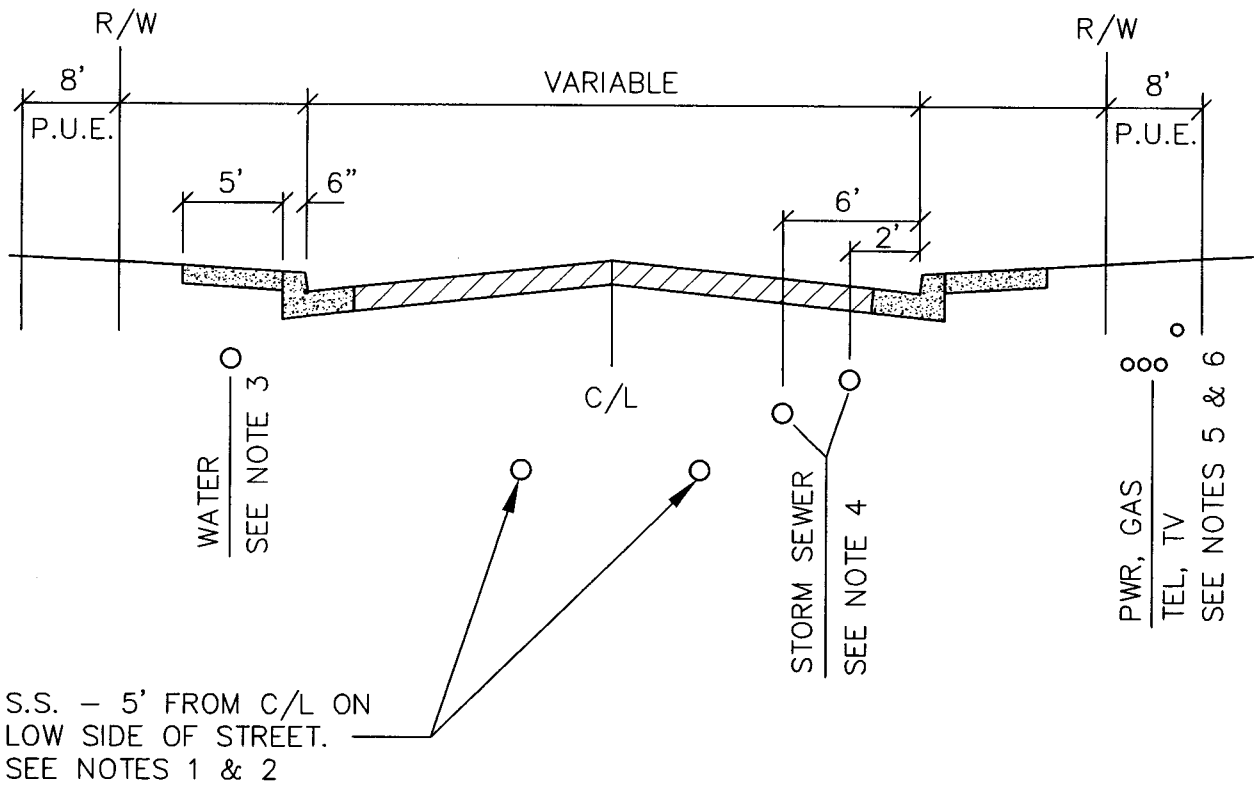


CITY OF CARLTON
Public Works Design Standards

Standard Detail Drawings

Appendix A



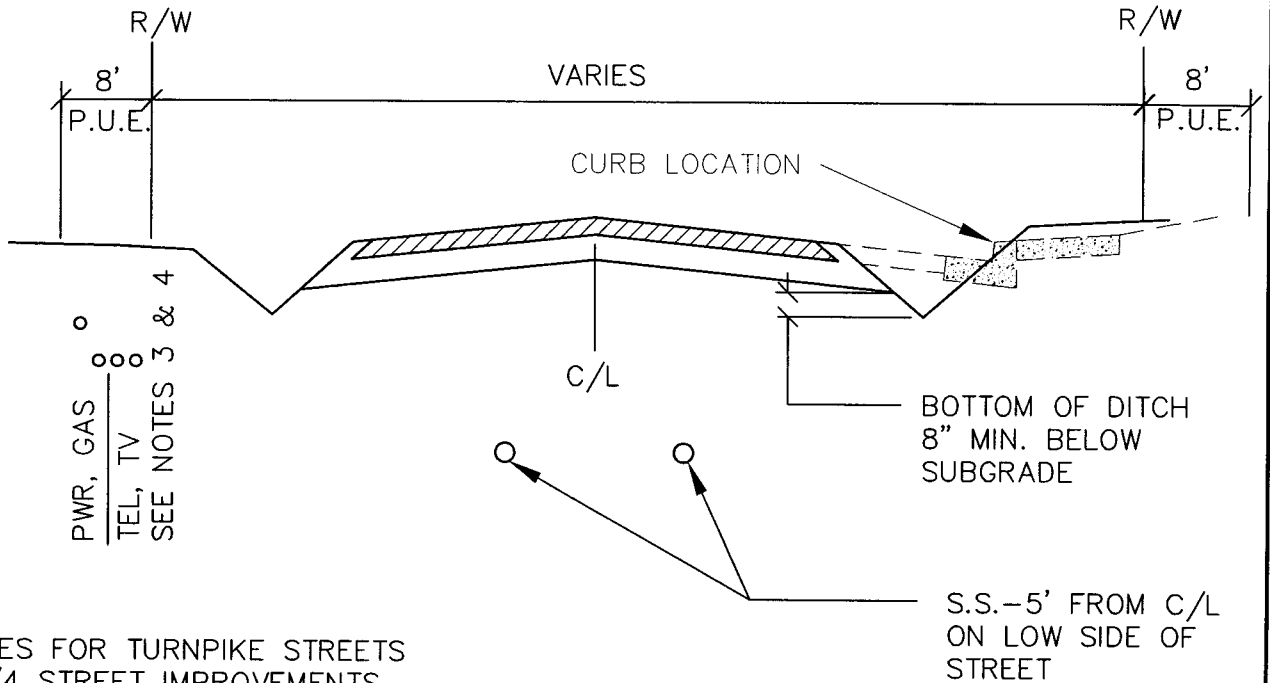
CURBED STREETS

NTS

NOTES:

1. 4' MIN. COVER REQUIRED ON SANITARY SEWER MAINS AND LATERALS.
2. LATERALS AND P/L CLEANOUTS TO BE INSTALLED DURING CONSTRUCTION OF SANITARY SEWER MAINS.
3. WATER TO BE INSTALLED 3' BEHIND FACE OF CURB ON HIGH SIDE OF STREET. 36" MIN. COVER ON ALL WATERLINES.
4. STORM SEWER TO BE INSTALLED ON LOW SIDE OF STREET:
 - a) 2' FROM FACE OF CURB FOR <4' RIM TO INVERT
 - b) 6' FROM FACE OF CURB FOR >4' RIM TO INVERT (MH SYSTEM)
5. MAINTAIN MIN. 3' HORIZ. SEPARATION BETWEEN PUBLIC UTILITIES & PARALLEL PRIVATE UTILITIES. OTHER VERT. AND HORIZ. SEPARATION DISTANCES ARE CONTROLLED BY DEQ, OHD, AND OTHER PUBLIC/PRIVATE UTILITY COMPANIES.
6. UNITY TRENCH PER UTILITY COMPANY REQUIREMENTS.

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TYP. UTILITY LOCATIONS (CURBED STREETS)	
(NTS)	
CARLTON, OR	DETAIL NO. 101



NOTE:

UTILITIES FOR TURNPIKE STREETS OR 3/4 STREET IMPROVEMENTS SHALL BE LOCATED TO ALLOW FUTURE CONSTRUCTION OF CURBED STREETS WITHOUT RELOCATING UTILITIES. SEE DETAIL 101.

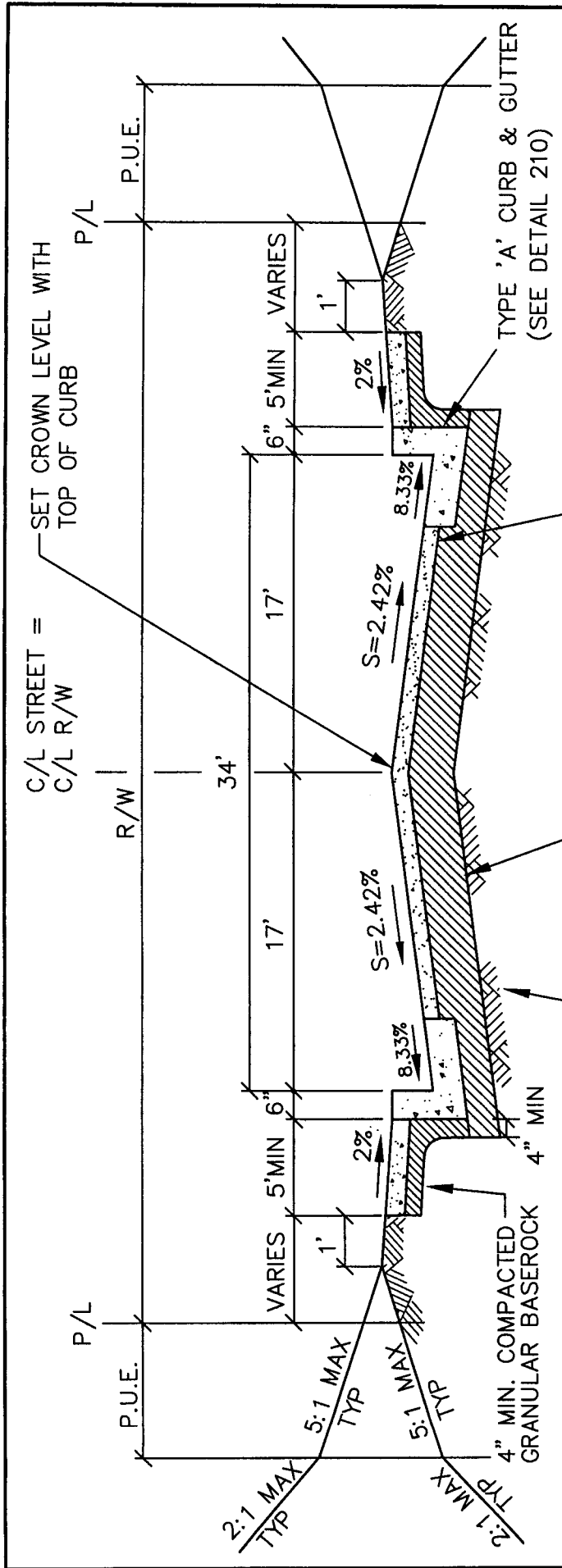
TURNPIKE STREETS

NTS

NOTES:

1. 4' MIN. COVER REQUIRED ON SANITARY SEWER MAINS AND LATERALS.
2. LATERALS AND P/L CLEANOUTS TO BE INSTALLED DURING CONSTRUCTION OF SANITARY SEWER MAINS.
3. WATER TO BE INSTALLED 3' BEHIND FACE OF CURB ON IMPROVED SIDE OR 3' BEHIND FUTURE FACE OF CURB LOCATION AS DIRECTED BY THE CITY ENGINEER.

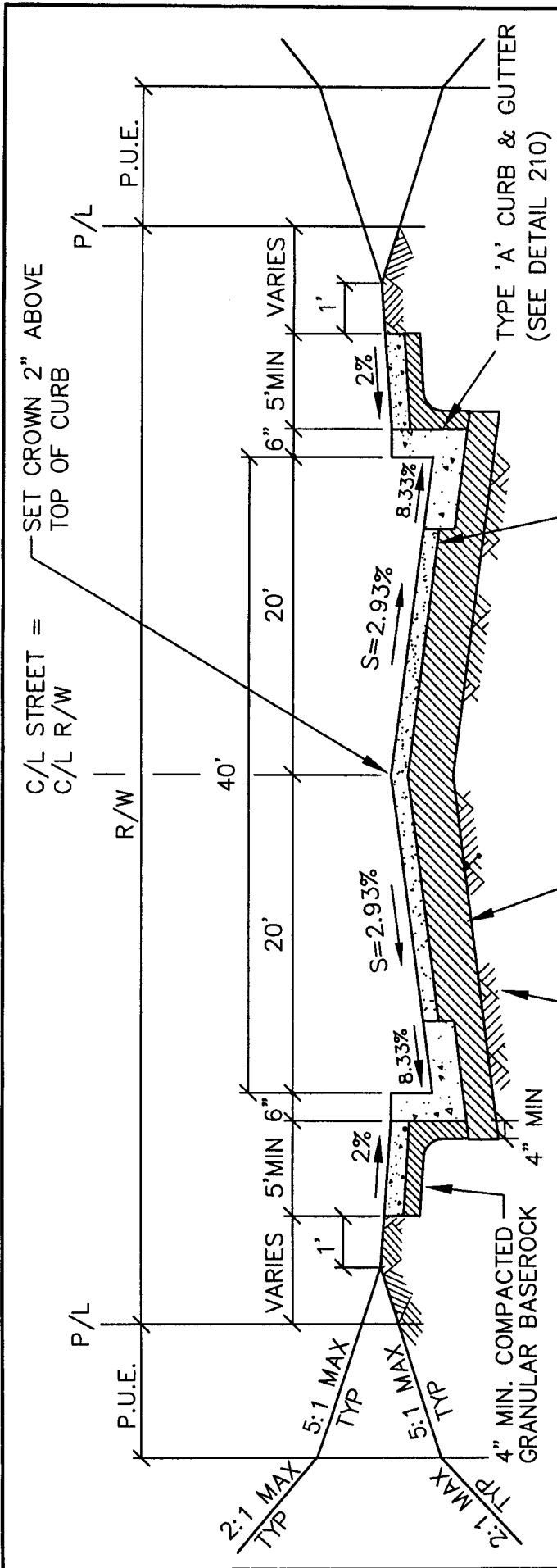
LAST REVISION DATE: DEC 2007	COPYRIGHT 1996 WESTECH ENGINEERING, INC.
TYP. UTILITY LOCATIONS (TURNPIKE AND 3/4 STREETS)	
(NTS)	
CARLTON, OR	DETAIL NO. 102



C/L STREET = C/L R/W
 SET CROWN LEVEL WITH TOP OF CURB
 P/L
 P.U.E.
 R/W
 34'
 17'
 17'
 6"
 5' MIN
 VARIES
 1'
 2%
 8.33%
 S=2.42%
 S=2.42%
 8.33%
 TYPE 'A' CURB & GUTTER (SEE DETAIL 210)
 3" A.C. PVMT. IN 2 LIFTS
 1-1/2" CL'C' OVER 1-1/2" CL'B'
 (COMPACT TO 91% OPTIMUM PER RICE STANDARD METHOD)
 4" MIN. COMPACTED GRANULAR BASEROCK
 4" MIN
 SUBGRADE, SEE NOTES BELOW.
 12" OF 1"-0" GRANULAR BASEROCK (COMPACT TO 95% OPTIMUM PER AASHTO T-180)
 ALT: 1-1/2" OF 3/4"-0" GRANULAR BASEROCK OVER 10-1/2" OF 1-1/2"-0" GRANULAR BASEROCK.

- NOTES:**
1. ALL DESIGN SUBGRADES SHALL BE COMPACTED AND PROOF-ROLLED PRIOR TO PLACEMENT OF BASEROCK. COMPACTION TESTING OF SUBGRADE MAY BE WAIVED AS OUTLINED UNDER NOTE 3.
 2. IF SUBGRADE FAILS THE PROOF-ROLL, SUBGRADE SHALL BE OVEREXCAVATED TO UNDISTURBED SOIL AND BACKFILLED WITH BASEROCK OVER GEOTEXTILE REINFORCEMENT FABRIC (AS SPECIFIED) TO ALLOW COMPACTION OF UPPER (DESIGN) BASEROCK SECTION AND TO MAINTAIN STRUCTURAL INTEGRITY OF NATIVE SUBGRADE SOILS. TYPICAL MIN. OVEREXCAVATION REQUIRED IS 12-INCHES. NO RUBBER TIRE EQUIPMENT ALLOWED ON SUBGRADE FOLLOWING OVEREXCAVATION.
 3. IF SUBGRADE PASSES PROOF-ROLL BUT CANNOT BE COMPACTED TO 95% OPTIMUM DENSITY PER AASHTO T-180 (OR IF CONTRACTOR CHOOSES NOT TO TEST), GEOTEXTILE SEPARATION FABRIC (AS SPECIFIED) SHALL BE PLACED ON THE SUBGRADE PRIOR TO PLACEMENT OF THE BASEROCK.
 4. REINFORCEMENT FABRIC: NON-WOVEN (MIRAFI 1000N, GEOTEX 1001, LINQ 250EX OR EQUAL), WOVEN (MIRAFI 550X, GEOTEX 250ST, LINQ GTF250 OR EQUAL).
 SEPARATION FABRIC: NON-WOVEN (MIRAFI 160N, GEOTEX 601, LINQ 150EX OR EQUAL), WOVEN (MIRAFI 500X, GEOTEX 200ST, LINQ GTF200 OR EQUAL).

LAST REVISION DATE: JAN 2010	COPYRIGHT 1996 WESTECH ENGINEERING, INC.
34' RESIDENTIAL STREET (LOCAL) MINIMUM SECTION (NTS)	
CARLTON, OR	DETAIL NO. 201



SUBGRADE, SEE NOTES BELOW.

12" OF 3/4"-0" GRANULAR BASEROCK
(COMPACT TO 95% OPTIMUM PER AASHTO T-180)

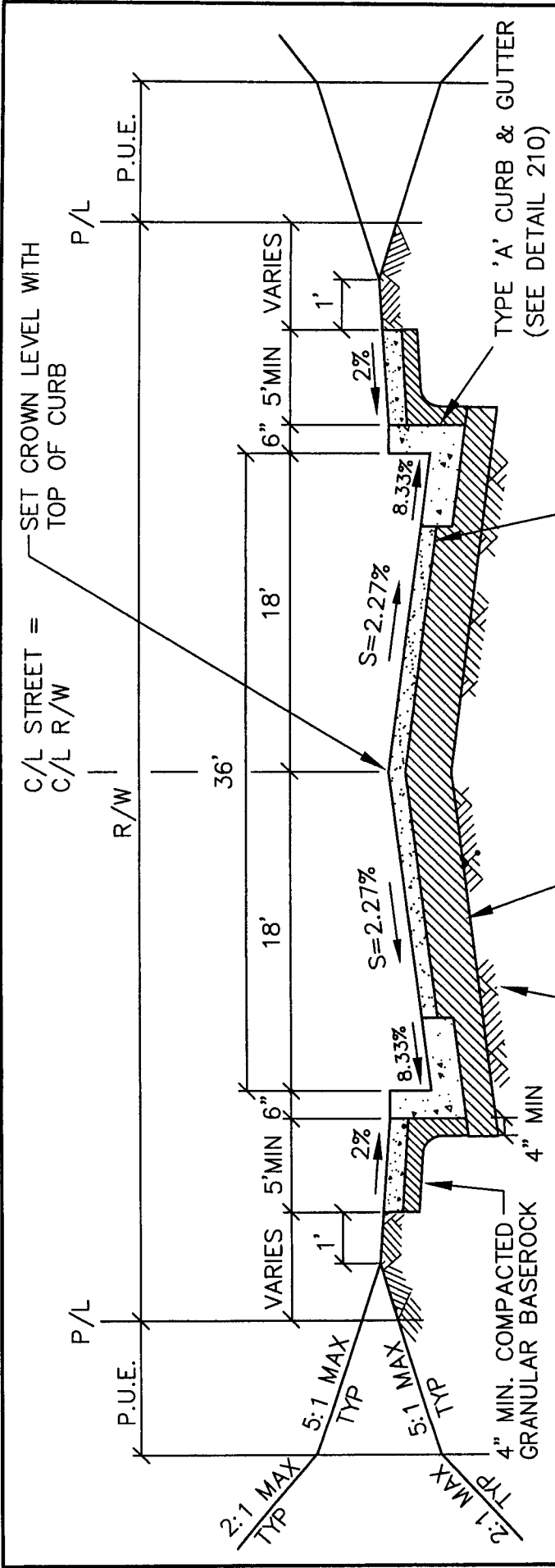
ALT: 1-1/2" OF 3/4"-0" GRANULAR BASEROCK OVER
10-1/2" OF 1"-0" GRANULAR BASEROCK.

4" A.C. PVMT. IN 2 LIFTS
2" CL.'C' OVER 2" CL.'B'
(COMPACT TO 91% OPTIMUM PER
RICE STANDARD METHOD)

NOTES:

1. ALL DESIGN SUBGRADES SHALL BE COMPACTED AND PROOF-ROLLED PRIOR TO PLACEMENT OF BASEROCK. COMPACTION TESTING OF SUBGRADE MAY BE WAIVED AS OUTLINED UNDER NOTE 3.
2. IF SUBGRADE FAILS THE PROOF-ROLL, SUBGRADE SHALL BE OVEREXCAVATED TO UNDISTURBED SOIL AND BACKFILLED WITH BASEROCK OVER GEOTEXTILE REINFORCEMENT FABRIC (AS SPECIFIED) TO ALLOW COMPACTION OF UPPER (DESIGN) BASEROCK SECTION AND TO MAINTAIN STRUCTURAL INTEGRITY OF NATIVE SUBGRADE SOILS. TYPICAL MIN. OVEREXCAVATION REQUIRED IS 12-INCHES. NO RUBBER TIRE EQUIPMENT ALLOWED ON SUBGRADE FOLLOWING OVEREXCAVATION.
3. IF SUBGRADE PASSES PROOF-ROLL BUT CANNOT BE COMPACTED TO 95% OPTIMUM DENSITY PER AASHTO T-180 (OR IF CONTRACTOR CHOOSES NOT TO TEST), GEOTEXTILE SEPARATION FABRIC (AS SPECIFIED) SHALL BE PLACED ON THE SUBGRADE PRIOR TO PLACEMENT OF THE BASEROCK.
4. REINFORCEMENT FABRIC: NON-WOVEN (MIRAFI 1000N, GEOTEX 1001, LINQ 250EX OR EQUAL), WOVEN (MIRAFI 550X, GEOTEX 250ST, LINQ GTF250 OR EQUAL).

LAST REVISION DATE: JAN 2010		COPYRIGHT 1998 WESTECH ENGINEERING, INC.	
40' MINOR COLLECTOR STREET MINIMUM SECTION (NTS)			
CARLTON, OR		DETAIL NO. 202A	



SUBGRADE, SEE NOTES BELOW.

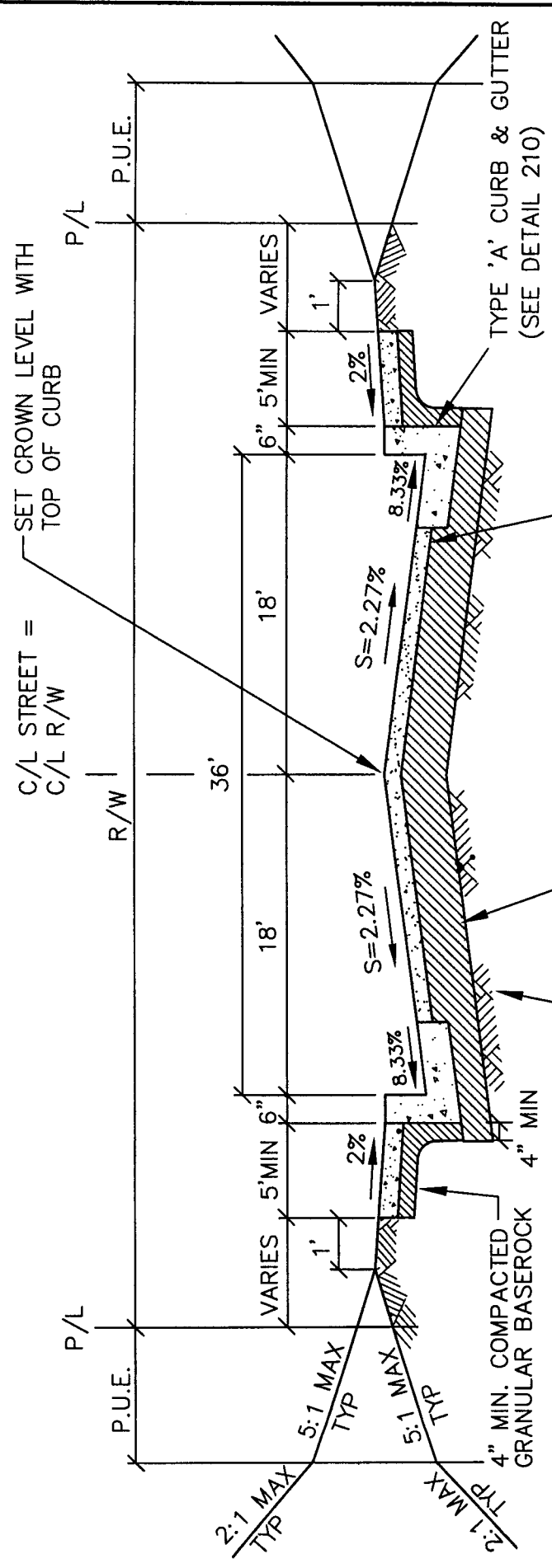
12" OF 1'-0" GRANULAR BASEROCK
(COMPACT TO 95% OPTIMUM PER AASHTO T-180)

ALT: 1-1/2" OF 3/4"-0" GRANULAR BASEROCK OVER
10-1/2" OF 1-1/2"-0" GRANULAR BASEROCK.

NOTES:

1. ALL DESIGN SUBGRADES SHALL BE COMPACTED AND PROOF-ROLLED PRIOR TO PLACEMENT OF BASEROCK. COMPACTION TESTING OF SUBGRADE MAY BE WAIVED AS OUTLINED UNDER NOTE 3.
2. IF SUBGRADE FAILS THE PROOF-ROLL, SUBGRADE SHALL BE OVEREXCAVATED TO UNDISTURBED SOIL AND BACKFILLED WITH BASEROCK OVER GEOTEXTILE REINFORCEMENT FABRIC (AS SPECIFIED) TO ALLOW COMPACTION OF UPPER (DESIGN) BASEROCK SECTION AND TO MAINTAIN STRUCTURAL INTEGRITY OF NATIVE SUBGRADE SOILS. TYPICAL MIN. OVEREXCAVATION REQUIRED IS 12-INCHES. NO RUBBER TIRE EQUIPMENT ALLOWED ON SUBGRADE FOLLOWING OVEREXCAVATION.
3. IF SUBGRADE PASSES PROOF-ROLL BUT CANNOT BE COMPACTED TO 95% OPTIMUM DENSITY PER AASHTO T-180 (OR IF CONTRACTOR CHOOSES NOT TO TEST), GEOTEXTILE SEPARATION FABRIC (AS SPECIFIED) SHALL BE PLACED ON THE SUBGRADE PRIOR TO PLACEMENT OF THE BASEROCK.
4. REINFORCEMENT FABRIC: NON-WOVEN (MIRAFI 1000N, GEOTEX 1001, LINQ 250EX OR EQUAL), WOVEN (MIRAFI 550X, GEOTEX 250ST, LINQ GTF250 OR EQUAL).

LAST REVISION DATE: JAN 2010	COPYRIGHT 1996 WESTECH ENGINEERING, INC.
36' COMMERCIAL STREET (NON-COLLECTOR) MINIMUM SECTION (NTS)	
CARLTON, OR	DETAIL NO. 203



SET CROWN LEVEL WITH TOP OF CURB

C/L STREET = C/L R/W

P/L

P.U.E.

R/W

36'

18'

18'

5' MIN

6"

2%

2%

2%

8.33%

8.33%

4" MIN. COMPACTED GRANULAR BASEROCK

4" MIN

TYPE 'A' CURB & GUTTER (SEE DETAIL 210)

4" A.C. PVMT. IN 2 LIFTS
2" CL.'C' OVER 2" CL.'B'
(COMPACT TO 91% OPTIMUM PER RICE STANDARD METHOD)

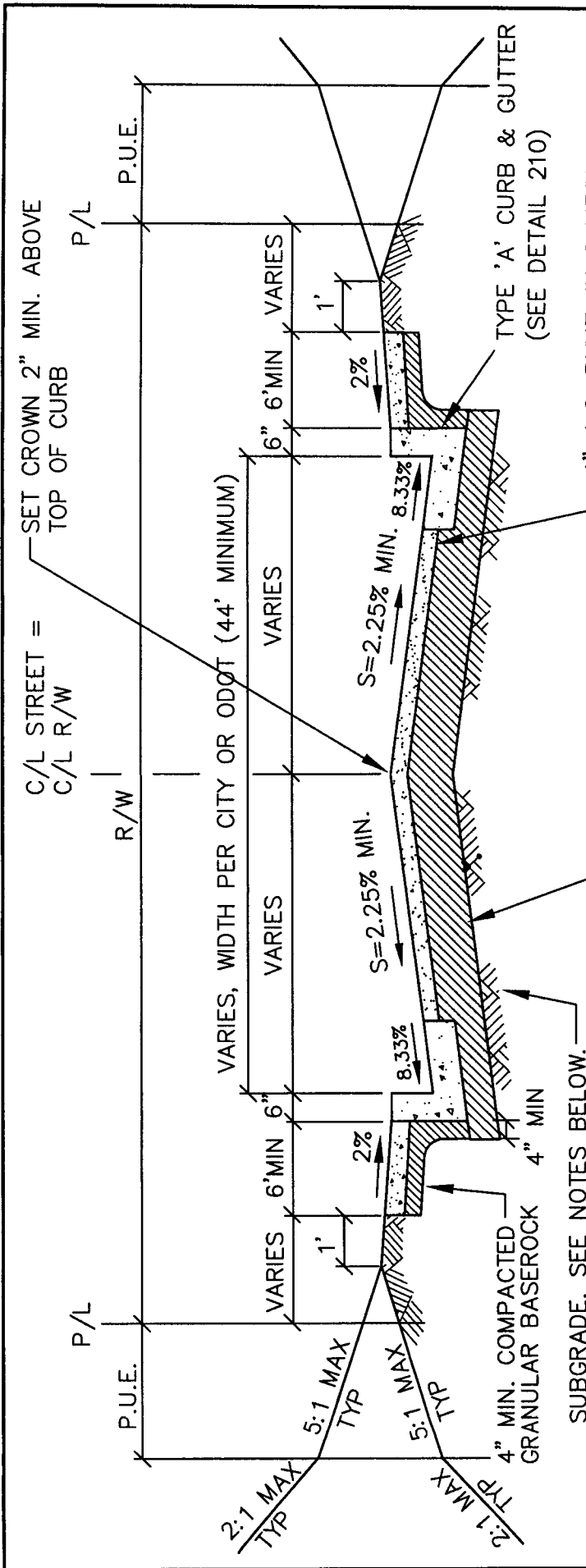
SUBGRADE, SEE NOTES BELOW.

15" OF 1"-0" GRANULAR BASEROCK
(COMPACT TO 95% OPTIMUM PER AASHTO T-180)

ALT: 1-1/2" OF 3/4"-0" GRANULAR BASEROCK OVER
13-1/2" OF 1-1/2"-0" GRANULAR BASEROCK.

- NOTES:
1. ALL DESIGN SUBGRADES SHALL BE COMPACTED AND PROOF-ROLLED PRIOR TO PLACEMENT OF BASEROCK. COMPACTION TESTING OF SUBGRADE MAY BE WAIVED AS OUTLINED UNDER NOTE 3.
 2. IF SUBGRADE FAILS THE PROOF-ROLL, SUBGRADE SHALL BE OVEREXCAVATED TO UNDISTURBED SOIL AND BACKFILLED WITH BASEROCK OVER GEOTEXTILE REINFORCEMENT FABRIC (AS SPECIFIED) TO ALLOW COMPACTION OF UPPER (DESIGN) BASEROCK SECTION AND TO MAINTAIN STRUCTURAL INTEGRITY OF NATIVE SUBGRADE SOILS. TYPICAL MIN. OVEREXCAVATION REQUIRED IS 12-INCHES. NO RUBBER Tired EQUIPMENT ALLOWED ON SUBGRADE FOLLOWING OVEREXCAVATION.
 3. IF SUBGRADE PASSES PROOF-ROLL BUT CANNOT BE COMPACTED TO 95% OPTIMUM DENSITY PER AASHTO T-180 (OR IF CONTRACTOR CHOOSES NOT TO TEST), GEOTEXTILE SEPARATION FABRIC (AS SPECIFIED) SHALL BE PLACED ON THE SUBGRADE PRIOR TO PLACEMENT OF THE BASEROCK.
 4. REINFORCEMENT FABRIC: NON-WOVEN (MIRAFI 1000N, GEOTEX 1001, LINQ 250EX OR EQUAL), WOVEN (MIRAFI 550X, GEOTEX 250ST, LINQ GTF250 OR EQUAL).

LAST REVISION DATE: JAN 2010	COPYRIGHT 1996 WESTECH ENGINEERING, INC.
36' INDUSTRIAL STREET (NON-COLLECTOR) MINIMUM SECTION (NTS)	
CARLTON, OR	DETAIL NO. 203A



4" A.C. PVMT. IN 2 LIFTS
 2" CL.'C' OVER 2" CL.'B'
 (COMPACT TO 91% OPTIMUM PER
 RICE STANDARD METHOD)

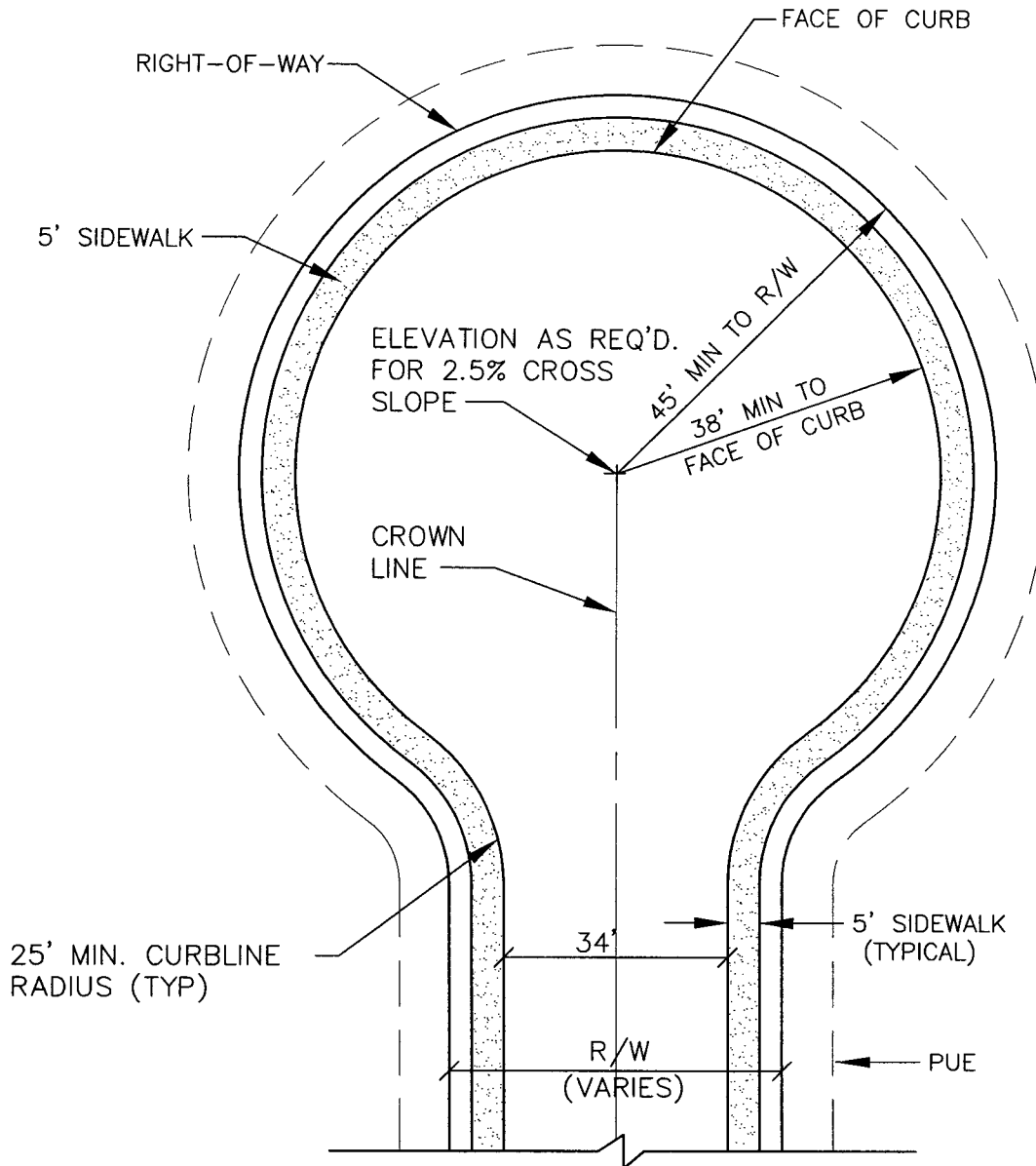
15" OF 1"-0" GRANULAR BASEROCK
 (COMPACT TO 95% OPTIMUM PER AASHTO T-180)

ALT: 1-1/2" OF 3/4"-0" GRANULAR BASEROCK OVER
 13-1/2" OF 1-1/2"-0" GRANULAR BASEROCK.

NOTES:

1. ALL DESIGN SUBGRADES SHALL BE COMPACTED AND PROOF-ROLLED PRIOR TO PLACEMENT OF BASEROCK. COMPACTION TESTING OF SUBGRADE MAY BE WAIVED AS OUTLINED UNDER NOTE 3.
2. IF SUBGRADE FAILS THE PROOF-ROLL, SUBGRADE SHALL BE OVEREXCAVATED TO UNDISTURBED SOIL AND BACKFILLED WITH BASEROCK OVER GEOTEXTILE REINFORCEMENT FABRIC (AS SPECIFIED) TO ALLOW COMPACTION OF UPPER (DESIGN) BASEROCK SECTION AND TO MAINTAIN STRUCTURAL INTEGRITY OF NATIVE SUBGRADE SOILS. TYPICAL MIN. OVEREXCAVATION REQUIRED IS 12-INCHES. NO RUBBER Tired EQUIPMENT ALLOWED ON SUBGRADE FOLLOWING OVEREXCAVATION.
3. IF SUBGRADE PASSES PROOF-ROLL BUT CANNOT BE COMPACTED TO 95% OPTIMUM DENSITY PER AASHTO T-180 (OR IF CONTRACTOR CHOOSES NOT TO TEST), GEOTEXTILE SEPARATION FABRIC (AS SPECIFIED) SHALL BE PLACED ON THE SUBGRADE PRIOR TO PLACEMENT OF THE BASEROCK.
4. REINFORCEMENT FABRIC: NON-WOVEN (MIRAFI 1000N, GEOTEX 1001, LINQ 250EX OR EQUAL), WOVEN (MIRAFI 550X, GEOTEX 250ST, LINQ GTF250 OR EQUAL).
 SEPARATION FABRIC: NON-WOVEN (MIRAFI 160N, GEOTEX 601, LINQ 150EX OR EQUAL), WOVEN (MIRAFI 500X, GEOTEX 200ST, LINQ GTF200 OR EQUAL).

LAST REVISION DATE: JAN 2010		COPYRIGHT 1996 WESTECH ENGINEERING, INC.	
ARTERIAL STREET MINIMUM SECTION			
(NTS)			
CARLTON, OR		DETAIL NO. 204	

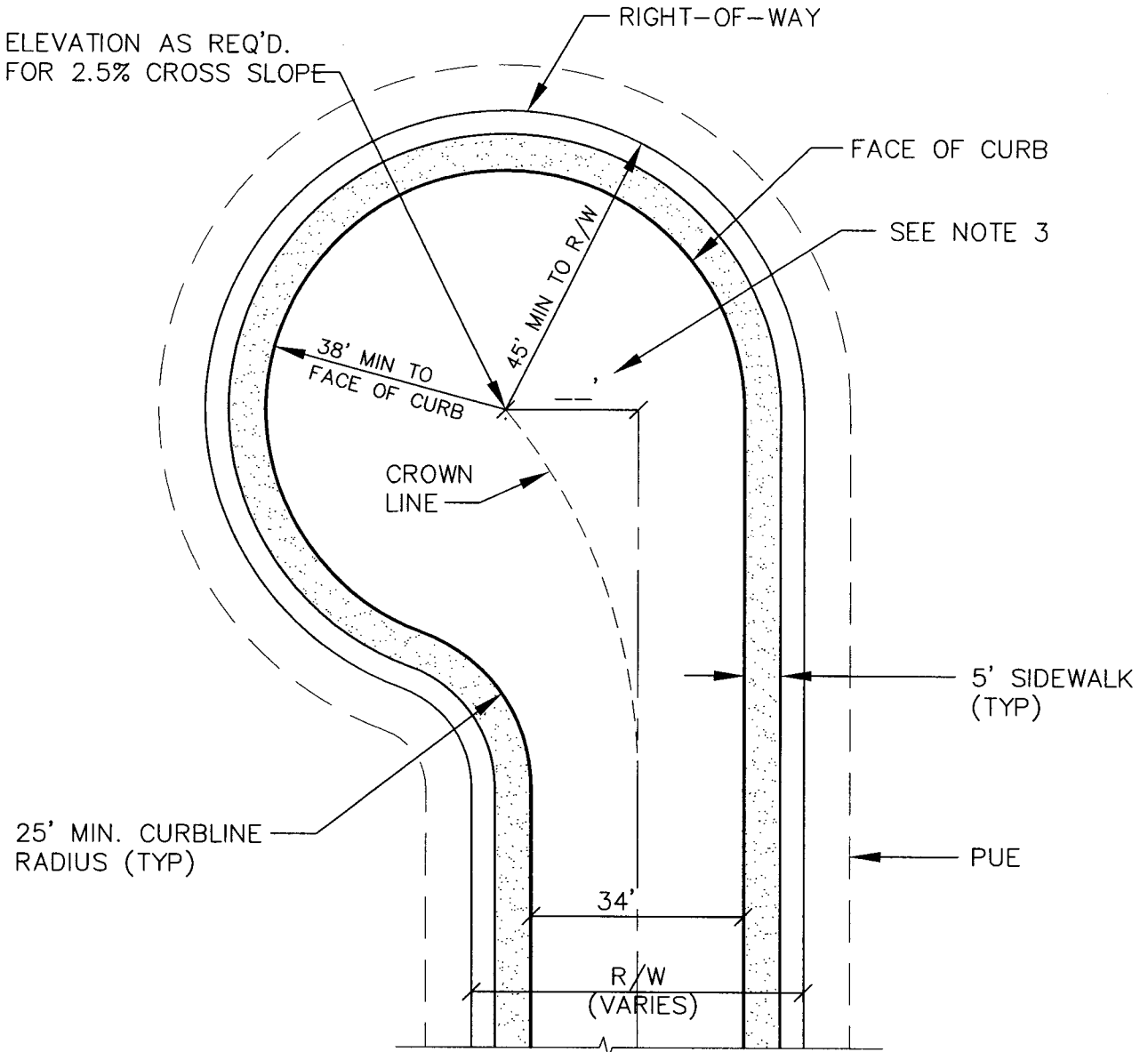


NOTES:

1. 2.5% MIN. CROSS SLOPE REQUIRED FROM CENTER OF BULB TO GUTTER.
2. MAINTAIN CROWN LINE TO CENTER OF CUL-DE-SAC BULB.

LAST REVISION DATE: FEB 2008	COPYRIGHT 1996 WESTECH ENGINEERING, INC.
STANDARD CUL-DE-SAC (RESIDENTIAL)	
(NTS)	
CARLTON, OR	DETAIL NO. 205

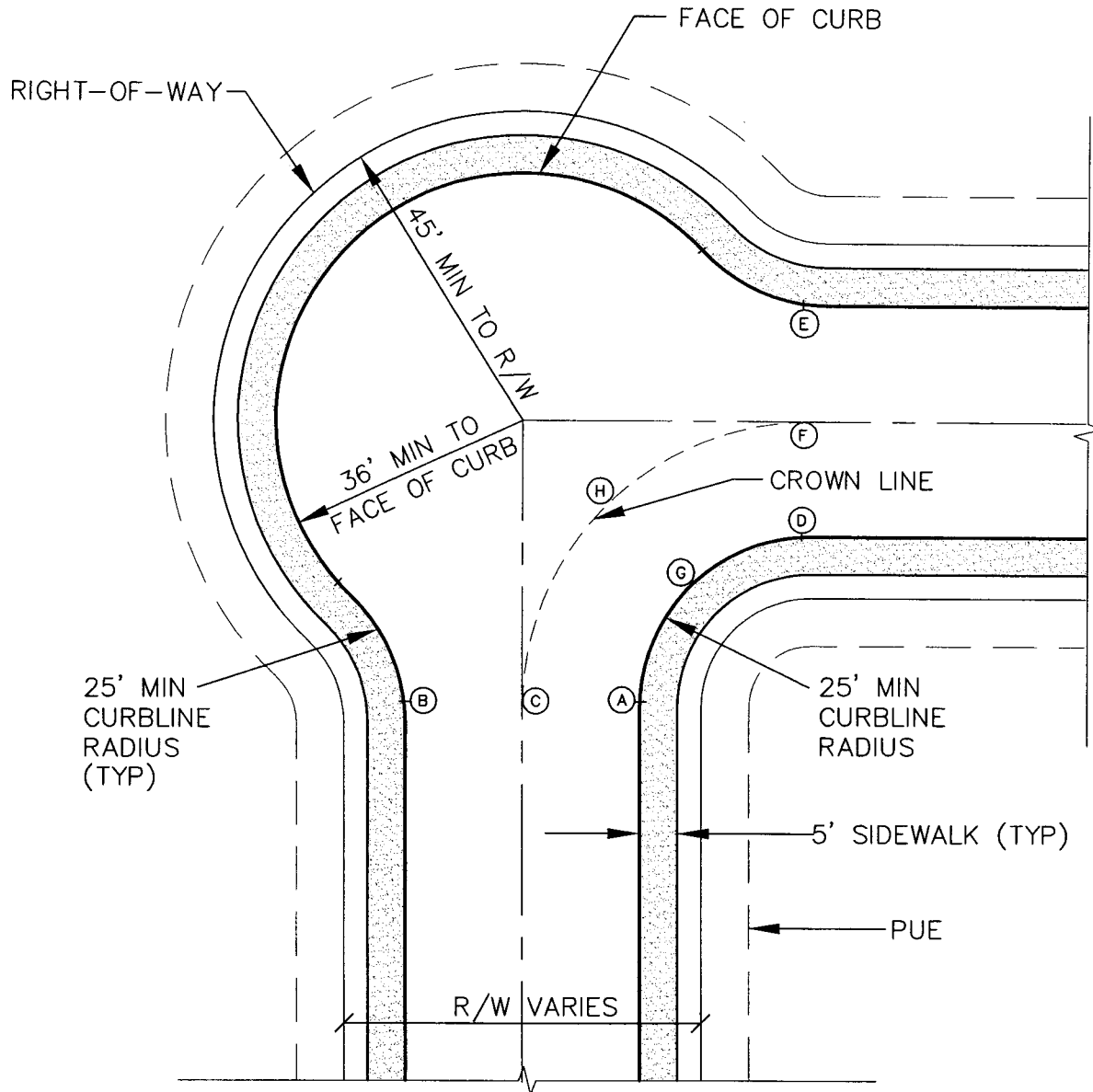
ELEVATION AS REQ'D.
FOR 2.5% CROSS SLOPE



NOTES:

1. 2.5% MIN. CROSS SLOPE REQUIRED FROM CENTER OF BULB TO GUTTER.
2. MAINTAIN CROWN LINE TO CENTER OF CUL-DE-SAC BULB.
3. OFFSET FROM ROADWAY CENTERLINE TO CENTER OF BULB = RADIUS MINUS ONE-HALF STREET WIDTH.

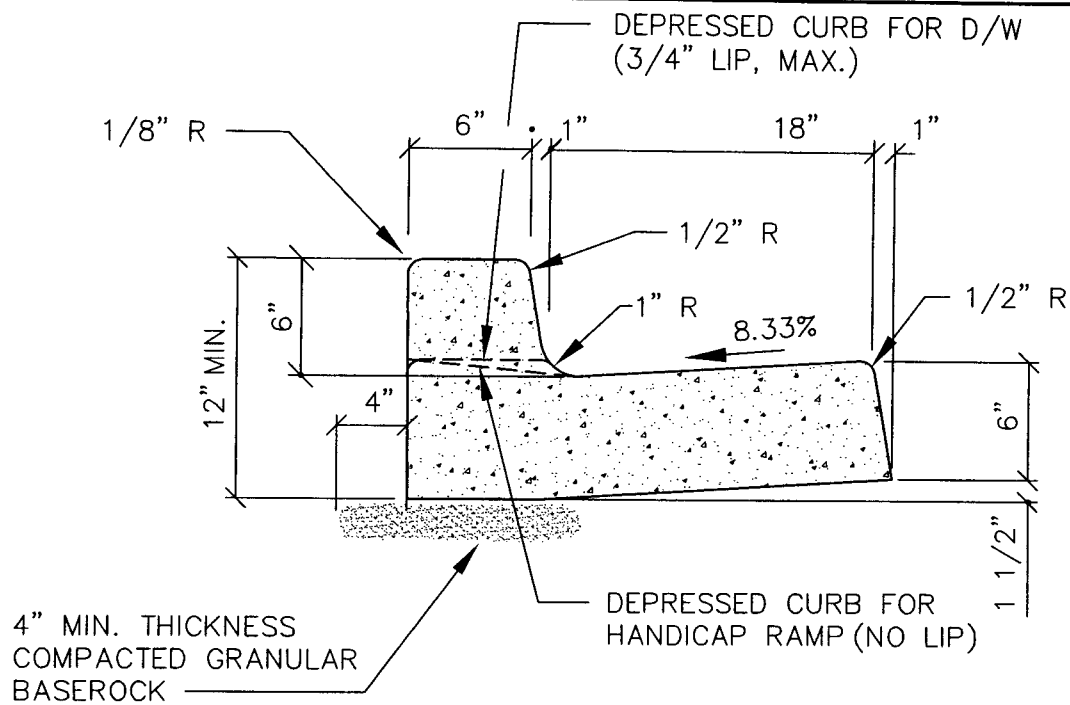
LAST REVISION DATE: FEB 2008	COPYRIGHT 1996 WESTECH ENGINEERING, INC.
OFFSET CUL-DE-SAC (RESIDENTIAL) (NTS)	
CARLTON, OR	DETAIL NO. 206



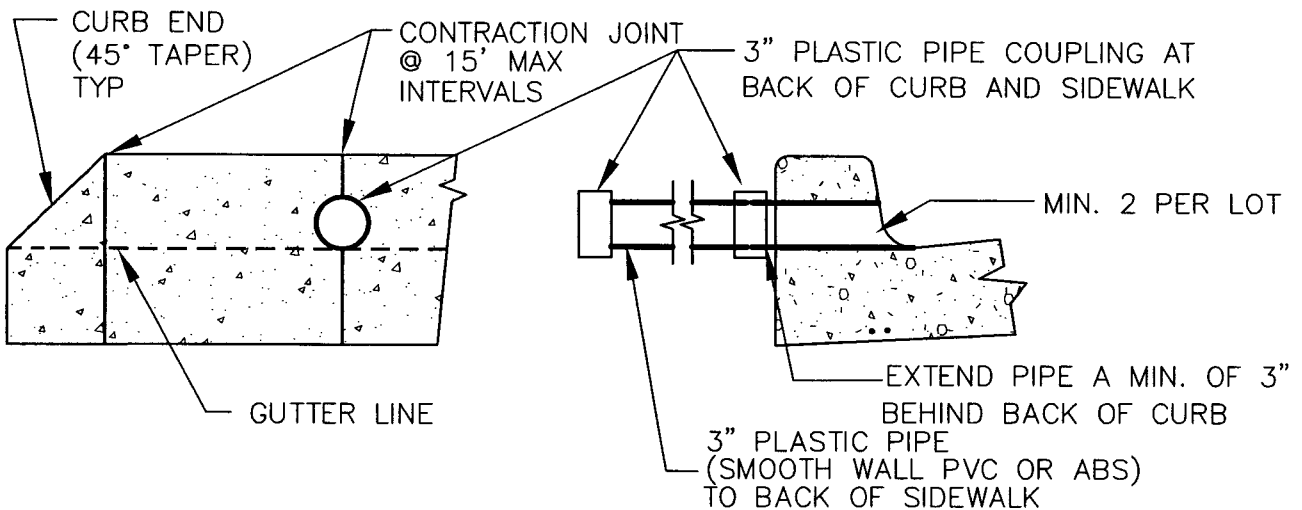
NOTES:

1. TOP CURB @ A = TOP CURB @ B = CROWN @ C
2. TOP CURB @ D = TOP CURB @ E = CROWN @ F
3. MIN. GUTTER SLOPE FROM E TO B = 0.75%
4. SET CROWN @ H 0.25' MIN. ABOVE TOP CURB @ G (4% MIN. CROSS SLOPE FROM H TO G)

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EYEBROW CUL-DE-SAC (RESIDENTIAL) (NTS)	
CARLTON, OR	DETAIL NO. 207



TYPE A CURB & GUTTER

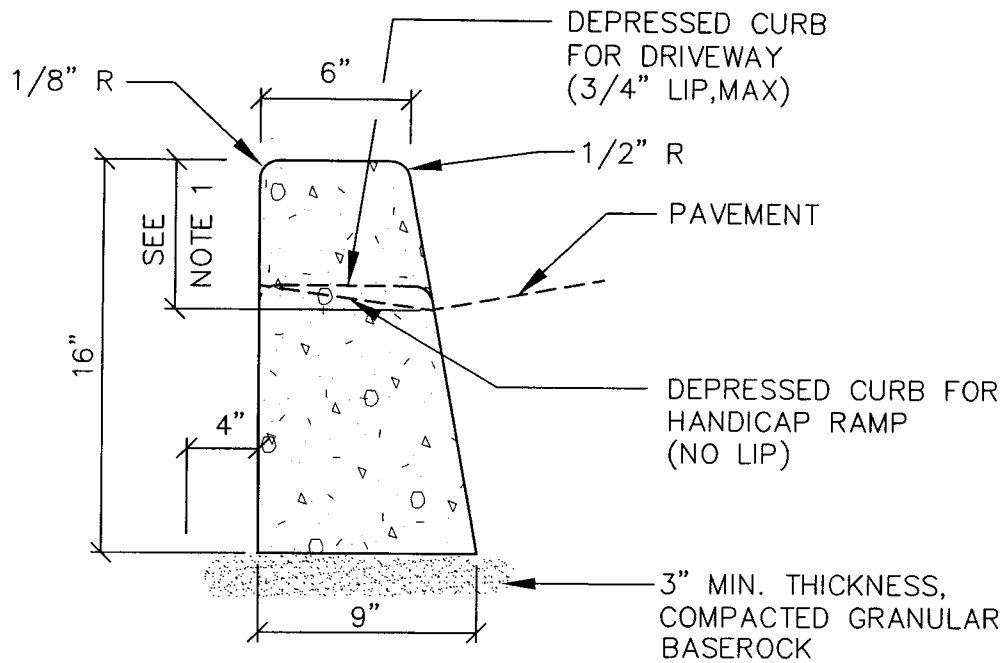


WEEP HOLE THROUGH CURB

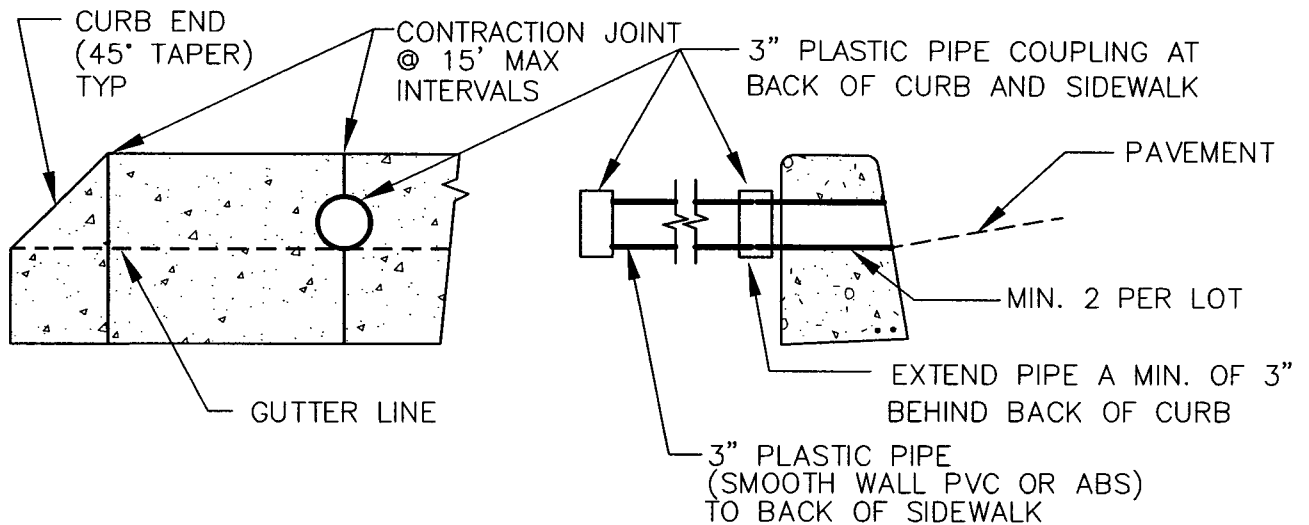
NOTES:

1. CONTRACTION JOINTS SHALL BE PLACED AT 15' MIN. INTERVALS AND SHALL EXTEND AT LEAST 50% THROUGH THE CURB OR CURB AND GUTTER.
2. A CONTRACTION JOINT SHALL BE PLACED ALONG AND OVER WEEP HOLE THROUGH THE SIDEWALK.
3. ALL CONCRETE SHALL BE 3300 PSI @ 28 DAYS.
4. WHERE SIDEWALKS ARE TO BE CONSTRUCTED, EXTEND 3" PIPE TO BACK OF SIDEWALK LOCATION & INSTALL COUPLING.
5. INSTALL MIN. 2 WEEP HOLES ON ALL LOTS. ONE WEEP HOLE TO BE AT LOW POINT OF LOT, 5' FROM P/L. WEEPHOLES IN EXISTING CURBS SHALL BE CORE DRILLED.

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TYPE 'A' CURB AND GUTTER AND WEEP HOLE (NTS)	
CARLTON, OR	DETAIL NO. 210



TYPE 'C' CURB

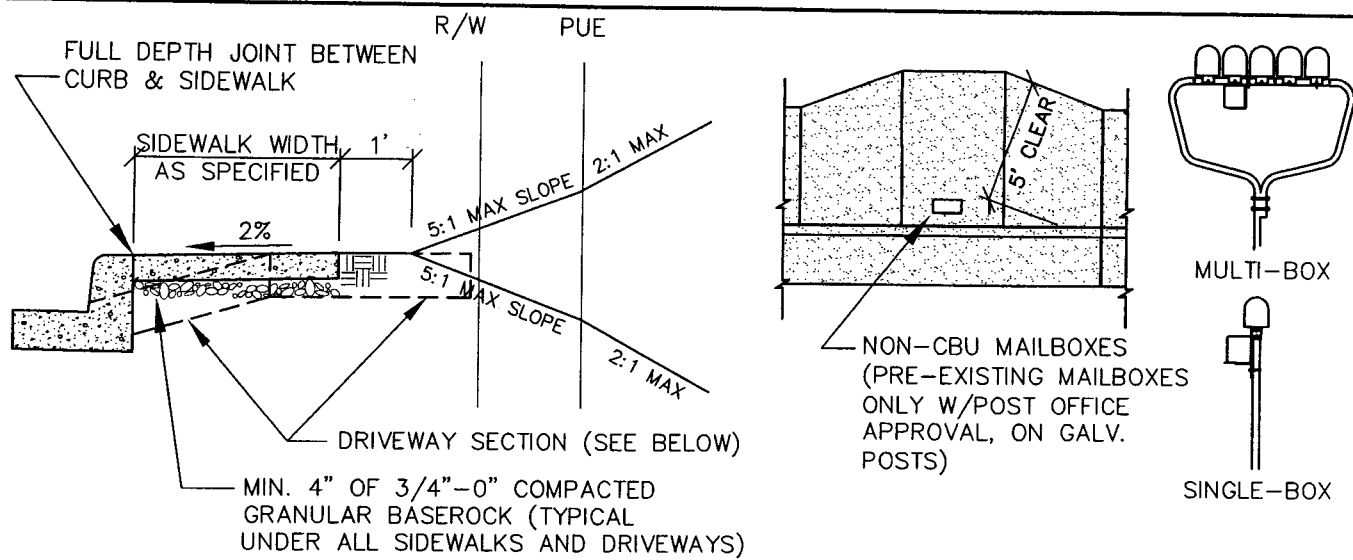


WEEP HOLE THROUGH CURB

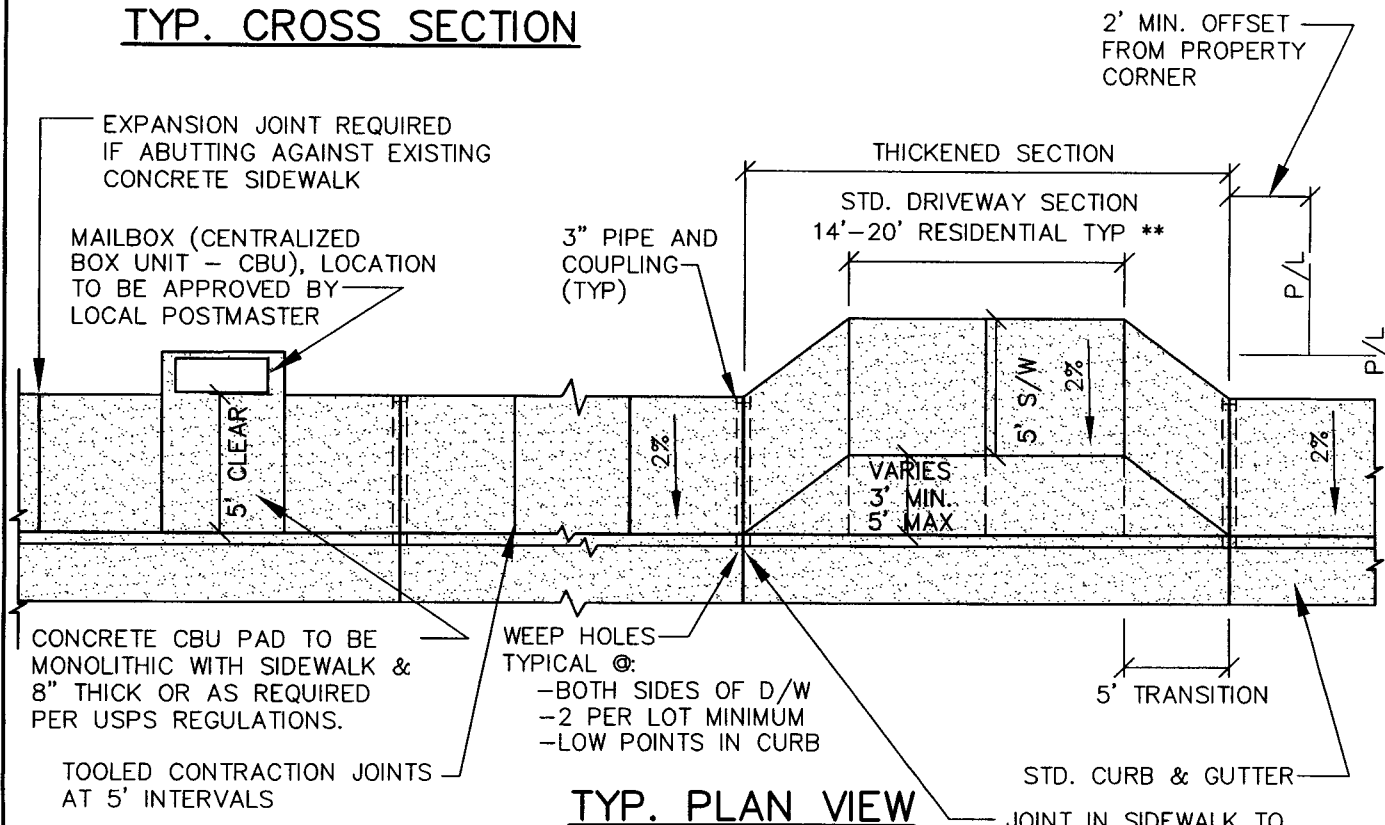
NOTES

1. 7" CURB EXPOSURE FOR ARTERIAL & COLLECTOR STREETS WHERE TYPE C CURB ALLOWED. 6" EXPOSURE ALL OTHER PUBLIC STREETS, PRIVATE STREETS & PARKING LOTS.
2. A CONTRACTION JOINT SHALL BE PLACED ALONG AND OVER WEEP HOLE THROUGH THE SIDEWALK.
3. ALL CONCRETE SHALL BE 3300 PSI @ 28 DAYS.
4. WHERE SIDEWALKS ARE TO BE CONSTRUCTED, EXTEND 3" PIPE TO BACK OF SIDEWALK LOCATION & INSTALL COUPLING.
5. INSTALL MIN. 2 WEEP HOLES ON ALL LOTS. ONE WEEP HOLE TO BE AT LOW POINT OF LOT, 5' FROM P/L. WEEP HOLES IN EXISTING CURBS SHALL BE CORE DRILLED.

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TYPE 'C' CURB AND WEEPHOLE	
(NTS)	
CARLTON, OR	DETAIL NO. 211



TYP. CROSS SECTION



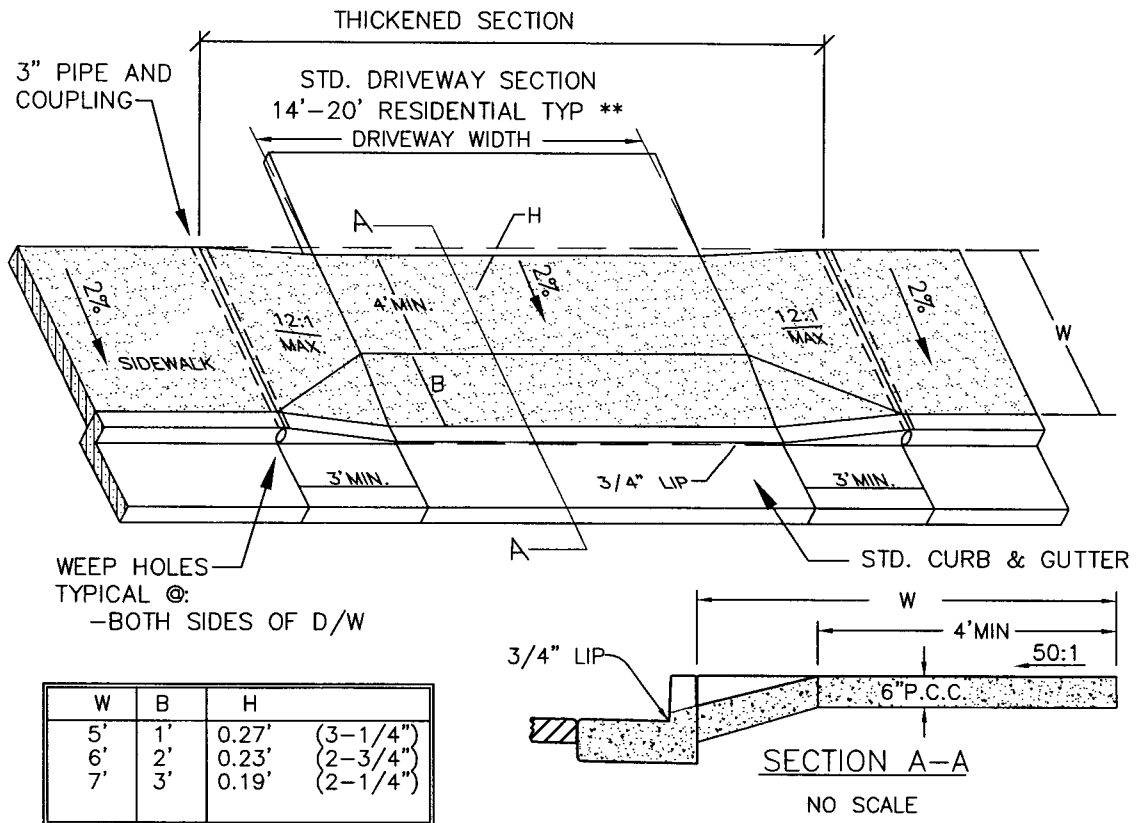
TYP. PLAN VIEW

NOTES:

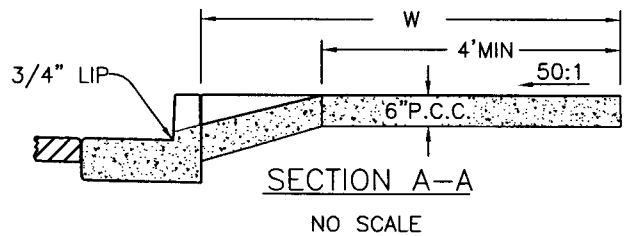
1. CONCRETE DEPTH FOR STANDARD SIDEWALKS SHALL BE 4" MIN.
2. DRIVEWAY SECTIONS INCLUDING TRANSITIONS & SIDEWALKS THROUGH DRIVEWAYS SHALL BE 6" MIN. THICKNESS. COMMERCIAL DRIVEWAYS SHALL BE 8" MIN. THICK.
3. SIDEWALKS 8' & WIDER SHALL HAVE A LONGITUDINAL CONTRACTION JOINT AT THE MIDPOINT.
4. CONCRETE SHALL BE 3300 PSI @ 28 DAYS.
5. PCC APRONS SHALL BE JOINTED TO MATCH SIDEWALK PATTERN.
6. SIDEWALKS SHALL BE LOCATED ENTIRELY WITHIN RIGHT-OF-WAY OR SIDEWALK EASEMENTS, INCLUDING SIDEWALKS AT DRIVEWAY APRONS.

**20' MAXIMUM @ CURBLINE PER CDC 2.203.06

LAST REVISION DATE: FEB 2008	COPYRIGHT 1996 WESTECH ENGINEERING, INC.
CURBLINE SIDEWALKS AND DRIVEWAY APRONS	
(NTS)	
CARLTON, OR	DETAIL NO. 212



W	B	H	
5'	1'	0.27'	(3-1/4")
6'	2'	0.23'	(2-3/4")
7'	3'	0.19'	(2-1/4")

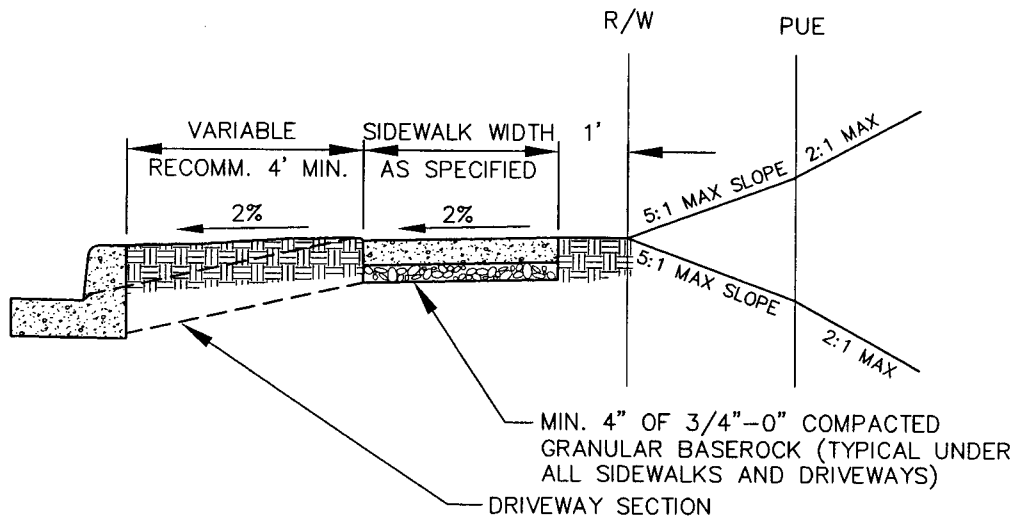


NOTES:

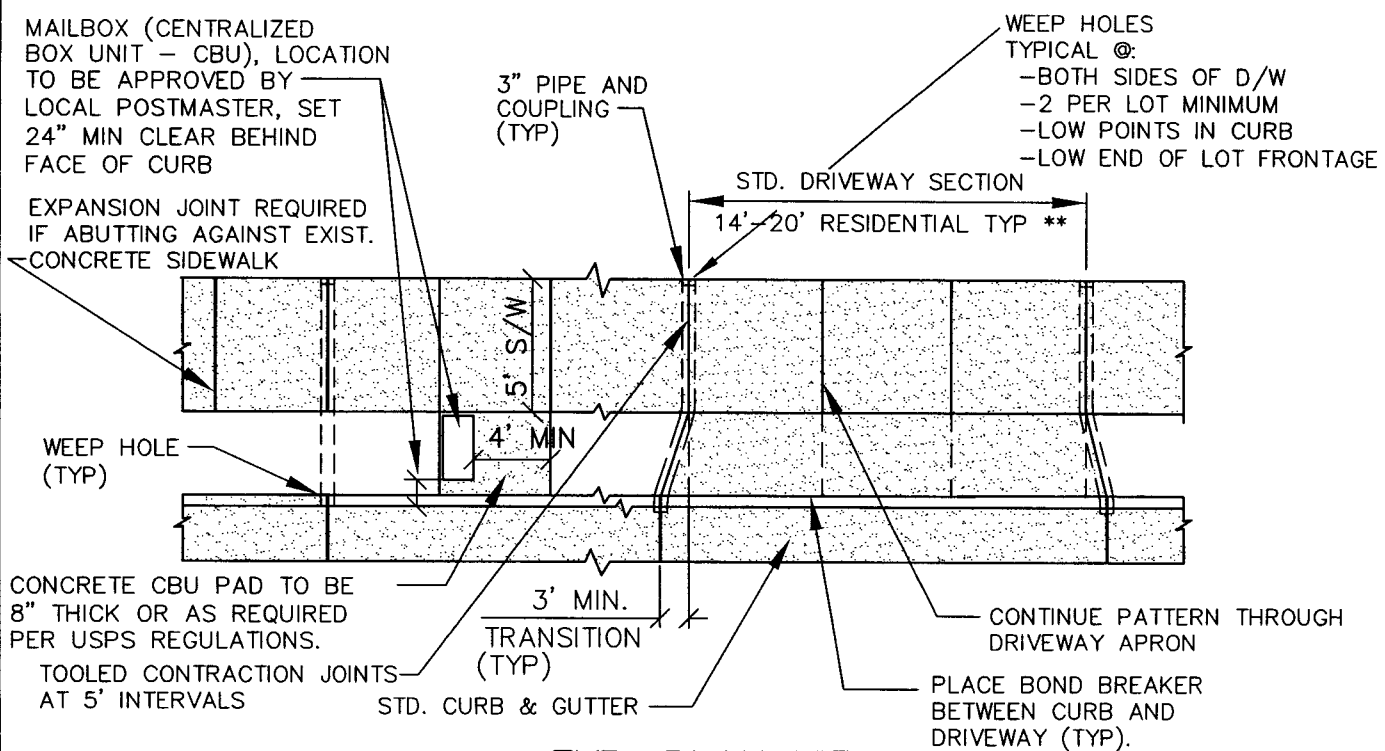
1. SEE DETAIL 212 FOR STANDARD APRON & SIDEWALK DETAILS. USE OF THIS DETAIL REQUIRES SPECIFIC APPROVAL BY PUBLIC WORKS PRIOR TO FORMING.
2. CONCRETE DEPTH FOR STANDARD SIDEWALKS SHALL BE 4" MIN.
3. DRIVEWAY SECTIONS INCLUDING SIDEWALKS THROUGH DRIVEWAYS SHALL BE 6" MIN. THICKNESS.
4. CONCRETE SHALL BE 3300 PSI @ 28 DAYS.
5. PCC APRONS SHALL BE JOINTED TO MATCH SIDEWALK PATTERN.
6. SIDEWALKS SHALL BE LOCATED ENTIRELY WITHIN RIGHT-OF-WAY OR SIDEWALK EASEMENTS, INCLUDING SIDEWALKS AT DRIVEWAY APRONS.
7. 2% CROSS SLOPE IS MEASURED FROM HORIZONTAL. 12:1 SIDEWALK SLOPE IS RELATIVE TO THE RUNNING SLOPE OF THE SIDEWALK.

**20' MAXIMUM @ CURBLINE PER CDC 2.203.06

LAST REVISION DATE: FEB 2008	COPYRIGHT 1996 WESTECH ENGINEERING, INC.
RESIDENTIAL D/W APRON CURBLINE SIDEWALK STEEP UPHILL LOTS ONLY (NTS)	
CARLTON, OR	DETAIL NO. 212A



TYP. CROSS SECTION



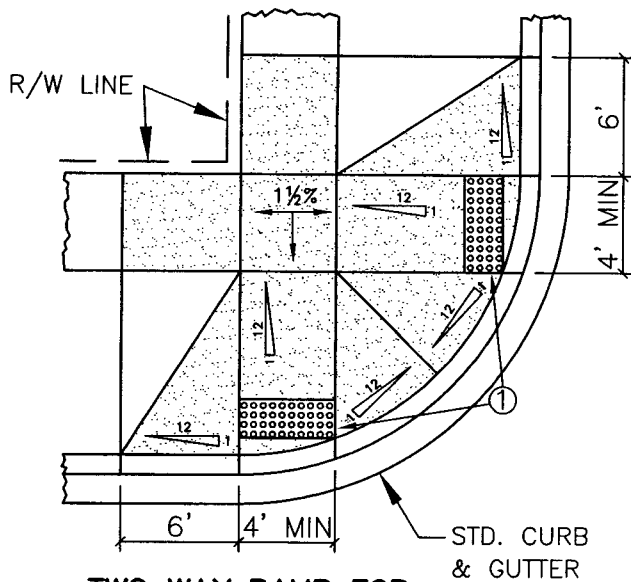
TYP. PLAN VIEW

NOTES:

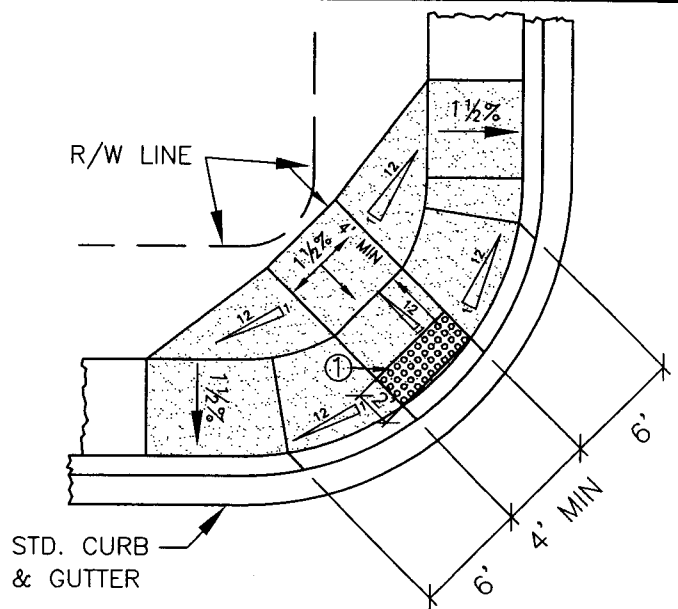
1. CONCRETE DEPTH FOR STANDARD SIDEWALKS SHALL BE 4" MIN.
2. RESIDENTIAL DRIVEWAY SECTIONS INCLUDING SIDEWALKS THROUGH DRIVEWAYS SHALL BE 6" MIN. THICKNESS. COMMERCIAL D/W SHALL BE 8" MIN. THICKNESS
2. CONCRETE FOR COMMERCIAL USE AND ALLEY APPROACHES SHALL BE 8" MIN. THICKNESS
3. SIDEWALKS 10' & WIDER SHALL HAVE A LONGITUDINAL CONTRACTION JOINT 5' ON CENTER.
4. PCC APRONS SHALL BE JOINTED TO MATCH SIDEWALK PATTERN.
5. CONCRETE SHALL BE 3300 PSI @ 28 DAYS.

**20' MAXIMUM @ CURBLINE PER CDC 2.203.06

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PROPERTY LINE SIDEWALKS AND DRIVEWAY APRONS	
(NTS)	
CARLTON, OR	DETAIL NO. 213



TWO WAY RAMP FOR PROPERTY LINE SIDEWALKS



CENTER RAMP FOR CURB LINE SIDEWALK CURB GRADE >2%

- ① CONSTRUCT TRUNCATED DOME DETECTABLE WARNING SURFACE WITH PARALLEL ALIGNMENT
 SPACING: D=1.6" MIN. TO 2.40" MAX
 0.65" MIN CLEAR BETWEEN DOME BASES

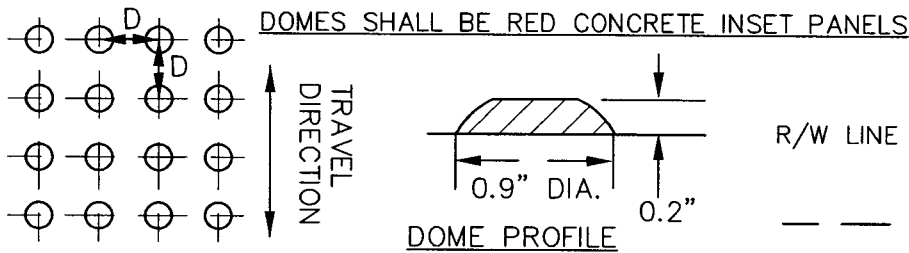
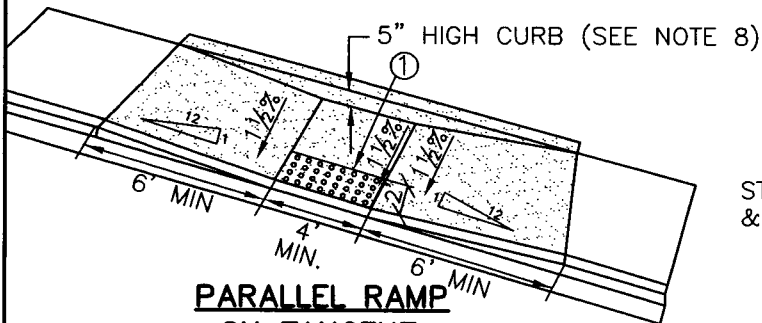
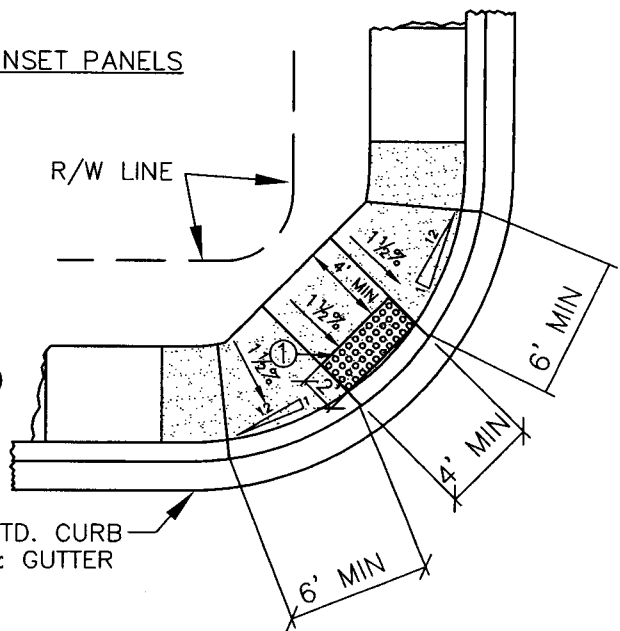


FIGURE A: TRUNCATED DOME DETAIL



PARALLEL RAMP ON TANGENT



CENTER RAMP FOR CURB LINE SIDEWALK CURB GRADE 2% MAX

NOTES:

1. SEE FIGURE A FOR RAMP TEXTURE DETAIL.
2. SEE TYPICAL STREET SECTIONS FOR SIDEWALK WIDTH.
3. ALL RAMPS AND TRANSITIONS SHALL BE ADA COMPLIANT.
4. LANDINGS SHALL HAVE A MIN. WIDTH & DEPTH OF 4 FEET
5. THE 2% CROSS SLOPES (1:50) SHOWN ARE MEASURED FROM HORIZONTAL.
6. SHADED AREAS TO BE CONSTRUCTED W/STREET IMPROV.
7. DROP CURBS FOR HANDICAP RAMPS SHALL BE CONSTRUCTED WITH NO LIP AT THE GUTTER LINE.
8. PROVIDE TANGENT RAMPS ON LOW SIDE OF STREET WITH A 6-INCH WIDE CONCRETE CURB AS SHOWN UNLESS A CATCH BASIN IS PROVIDED AT UPHILL END OF RAMP.
9. DOMES PANELS TO BE MASCO CASTINTACT OR EQUAL.

LAST REVISION DATE:

DEC 2007

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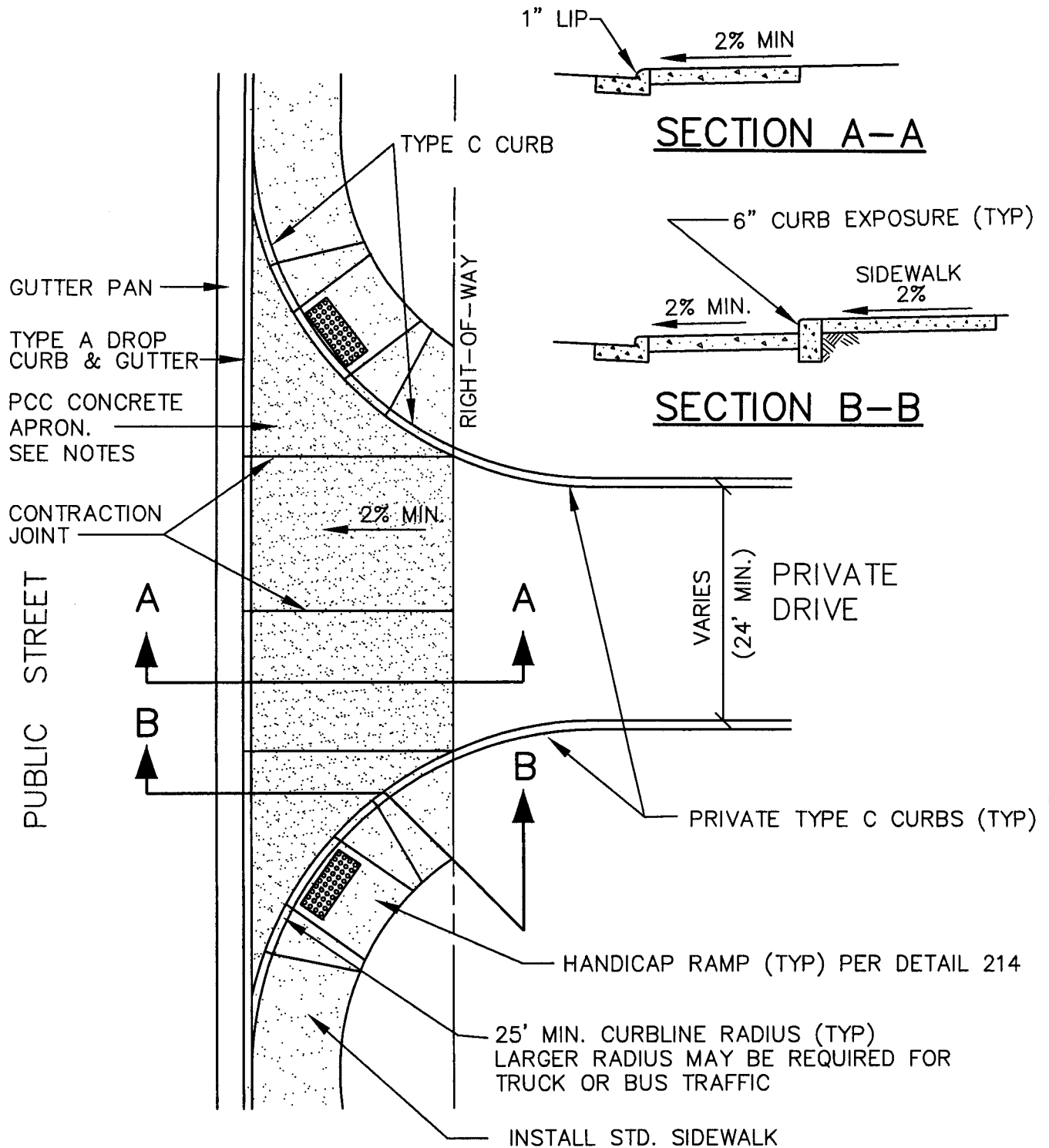
HANDICAP RAMPS

(NTS)

DETAIL NO.

CARLTON, OR

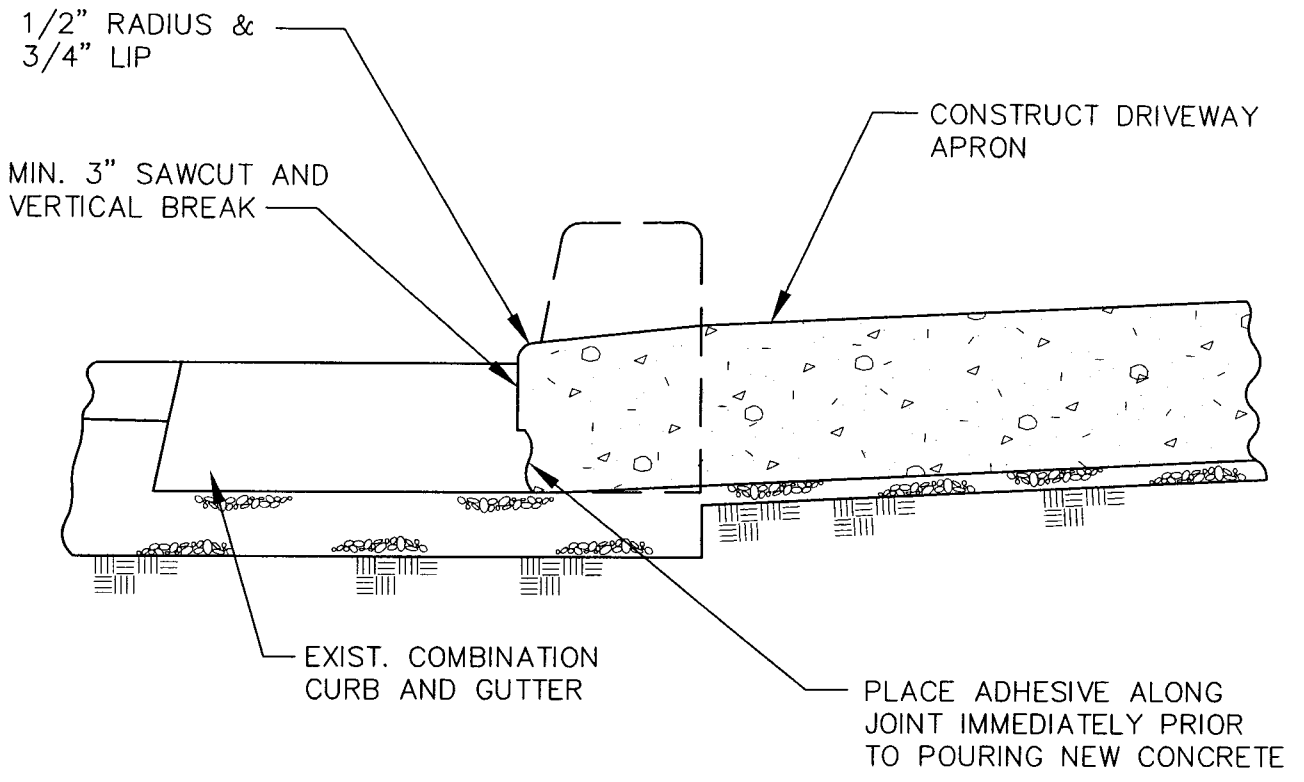
214



NOTES:

1. CONCRETE APRON TO HAVE A MINIMUM THICKNESS OF 8" CLASS 3300 PCC WITH 6" X 6" 10 GA. WELDED WIRE MESH.

LAST REVISION DATE: JAN 2010	COPYRIGHT 1996 WESTECH ENGINEERING, INC.
COMMERCIAL DRIVEWAY APPROACH	
(NTS)	
CARLTON, OR	DETAIL NO. 216

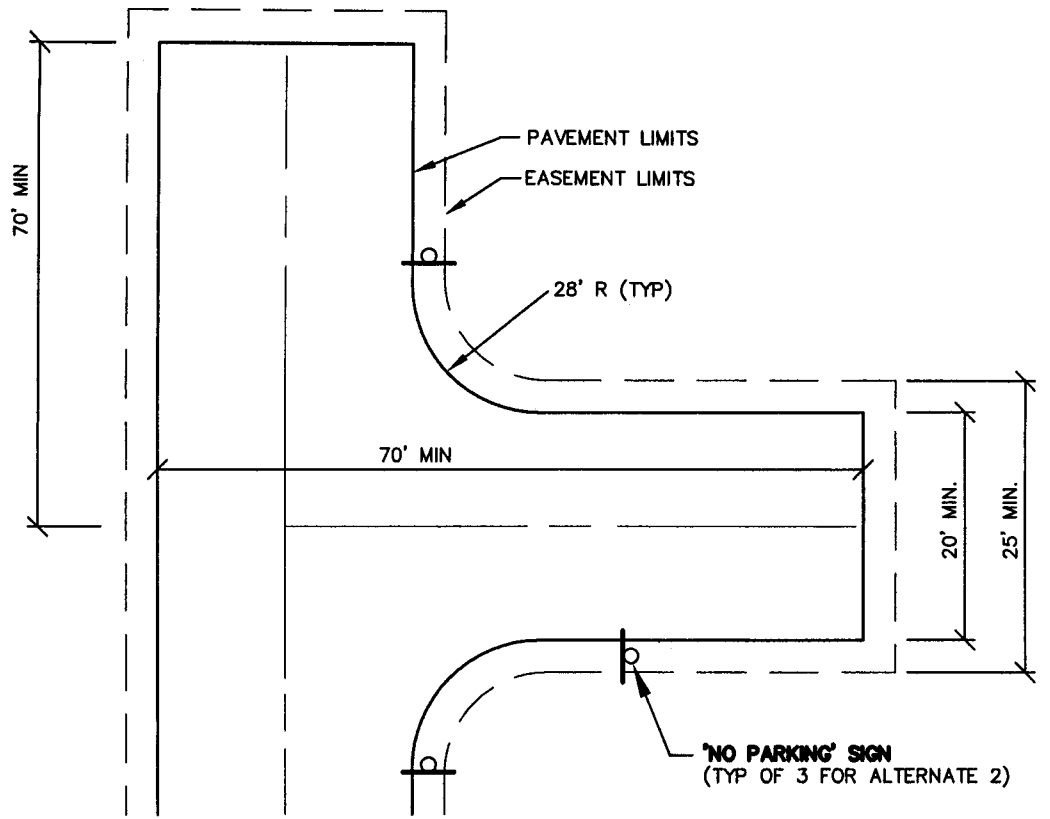


NOTES:

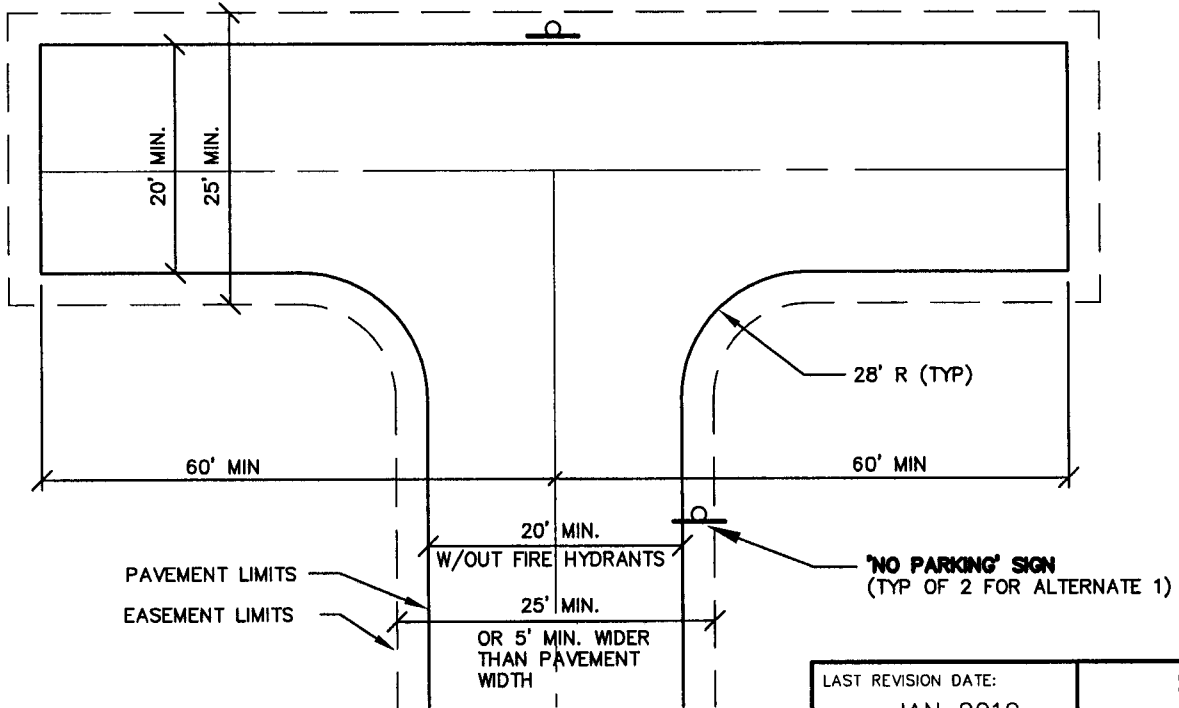
1. ONLY ALLOWED ON EXISTING PAVED STREETS.
2. SAWCUT THROUGH GUTTER PAN SHALL BE MADE AS CLOSE TO CURB FACE AS POSSIBLE.
3. COMPLETE CURB AND GUTTER SHALL NOT BE REMOVED UNLESS APPROVED BY THE CITY ENGINEER.
4. WHEN TYPE 'C' CURBS ARE REMOVED, A MINIMUM OF 2 FEET OF PAVEMENT FROM THE FACE OF CURB SHOULD BE REMOVED AND REPLACED PER DETAIL 302A UNLESS OTHERWISE APPROVED BY THE CITY.

LAST REVISION DATE: DEC 2007	COPYRIGHT 1996 WESTECH ENGINEERING, INC.
CURB KNOCKOUT FOR NEW DRIVEWAYS ON EXISTING CURBED STREETS (NTS)	
CARLTON, OR	DETAIL NO. 217

FIRE CODE NOTE:
 ALL FIRE LANES,
 TURNAROUNDS AND
 ASSOCIATED
 IMPROVEMENTS SHALL
 COMPLY WITH THE
 MOST CURRENT
 VERSION OF THE
 OREGON FIRE CODE
 (OFC).



ALT 2

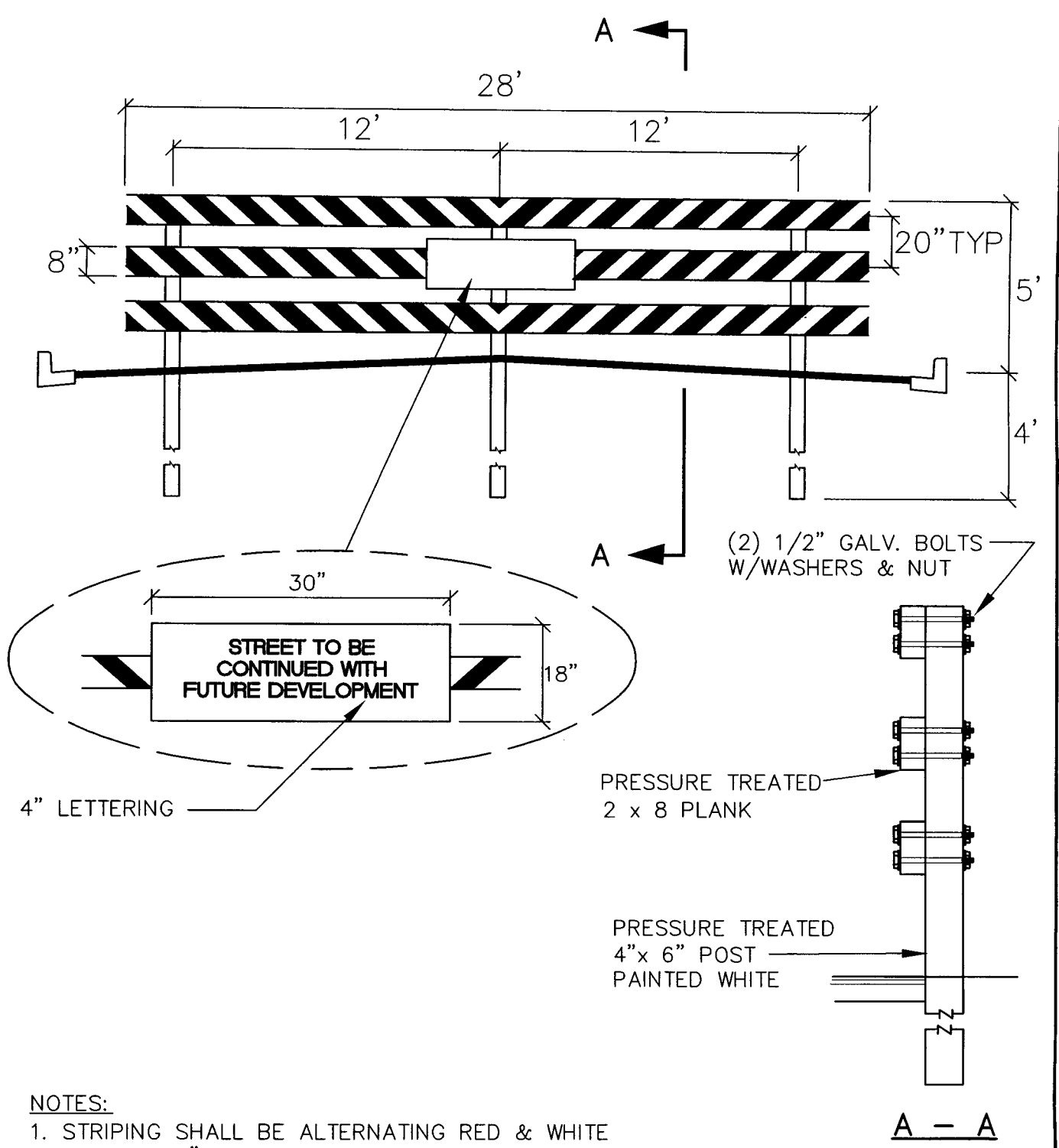


ALT 1

NOTES:

1. 'NO PARKING, FIRE LANE' SIGNS REQUIRED WITHIN LIMITS OF ACCESS EASEMENT AND TURNAROUND.
2. THESE ARE TYPICAL MINIMUM DESIGNS AS REQUIRED BY THE 2007 OFC D103.4 & FIG D103.1. ALTERNATE DESIGNS SHALL MEET THE APPROVAL OF THE LOCAL FIRE MARSHALL.
3. PAVEMENT DIMENSIONS SHOWN REFER TO TOTAL DRIVABLE WIDTH BETWEEN CURBS IF PRESENT.
4. MIN. 26' PAVEMENT WIDTH AT FIRE HYDRANTS (OFC D103.1).

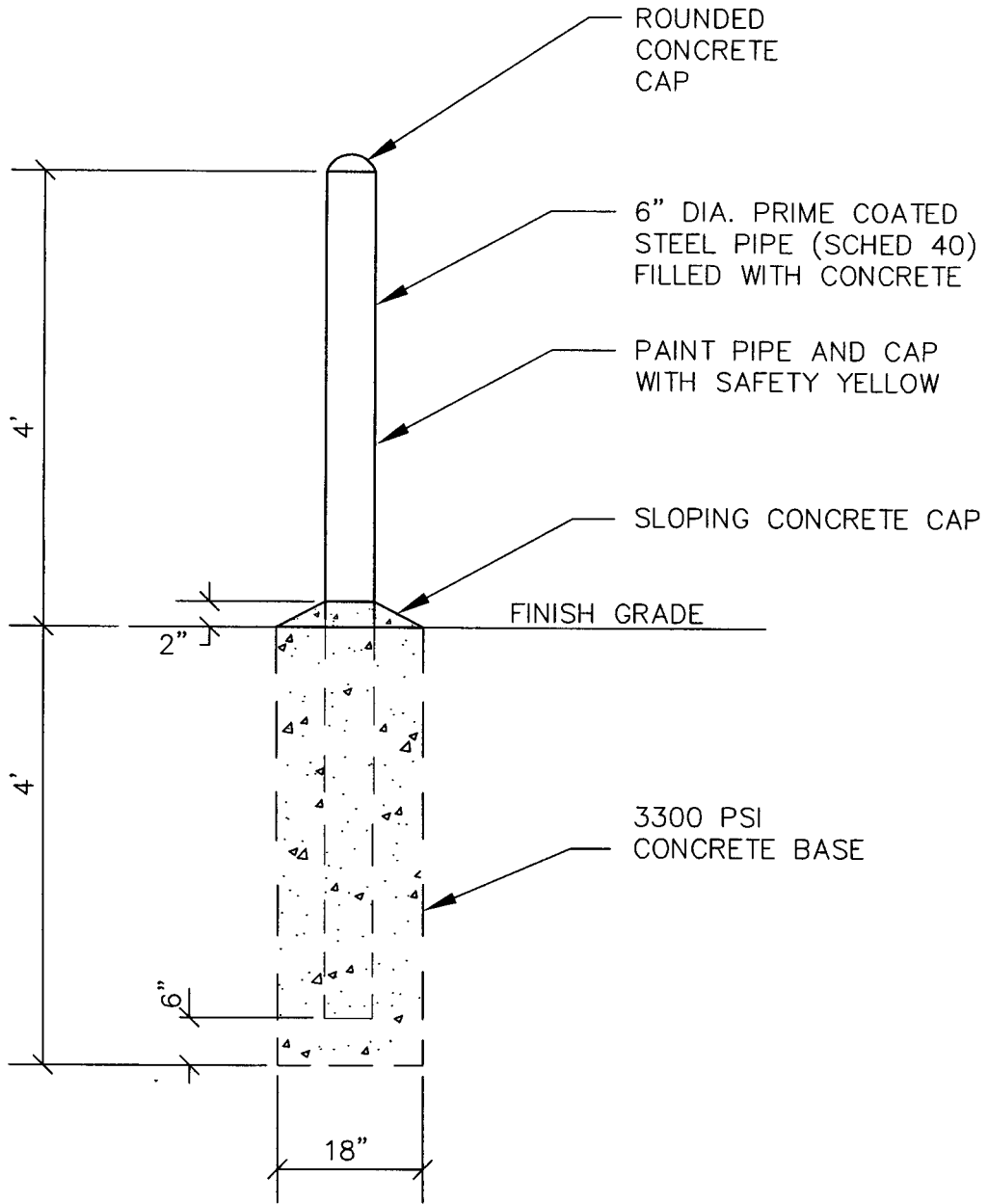
LAST REVISION DATE: JAN 2010	COPYRIGHT 1996 WESTECH ENGINEERING, INC.
HAMMERHEAD TURNAROUND (FIRE LANES)	
(NTS)	
CARLTON, OR	DETAIL NO. 220



NOTES:

1. STRIPING SHALL BE ALTERNATING RED & WHITE STRIPES 6" WIDE & AT A 45° ANGLE.
2. STRIPING SHALL BE EITHER RETRO-REFLECTIVE TAPE OR PAINTED WITH A SEALED RETRO-REFLECTIVE SURFACE.
3. BARRICADE SHALL BE LOCATED WITHIN THE RESERVE STRIP, IF PRESENT.

LAST REVISION DATE: DEC 2007	COPYRIGHT 1996 WESTECH ENGINEERING, INC.
STREET BARRICADE (STUB STREETS)	
(NTS)	
CARLTON, OR	DETAIL NO. 225



LAST REVISION DATE:
DEC 2007

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WESTECH ENGINEERING, INC.

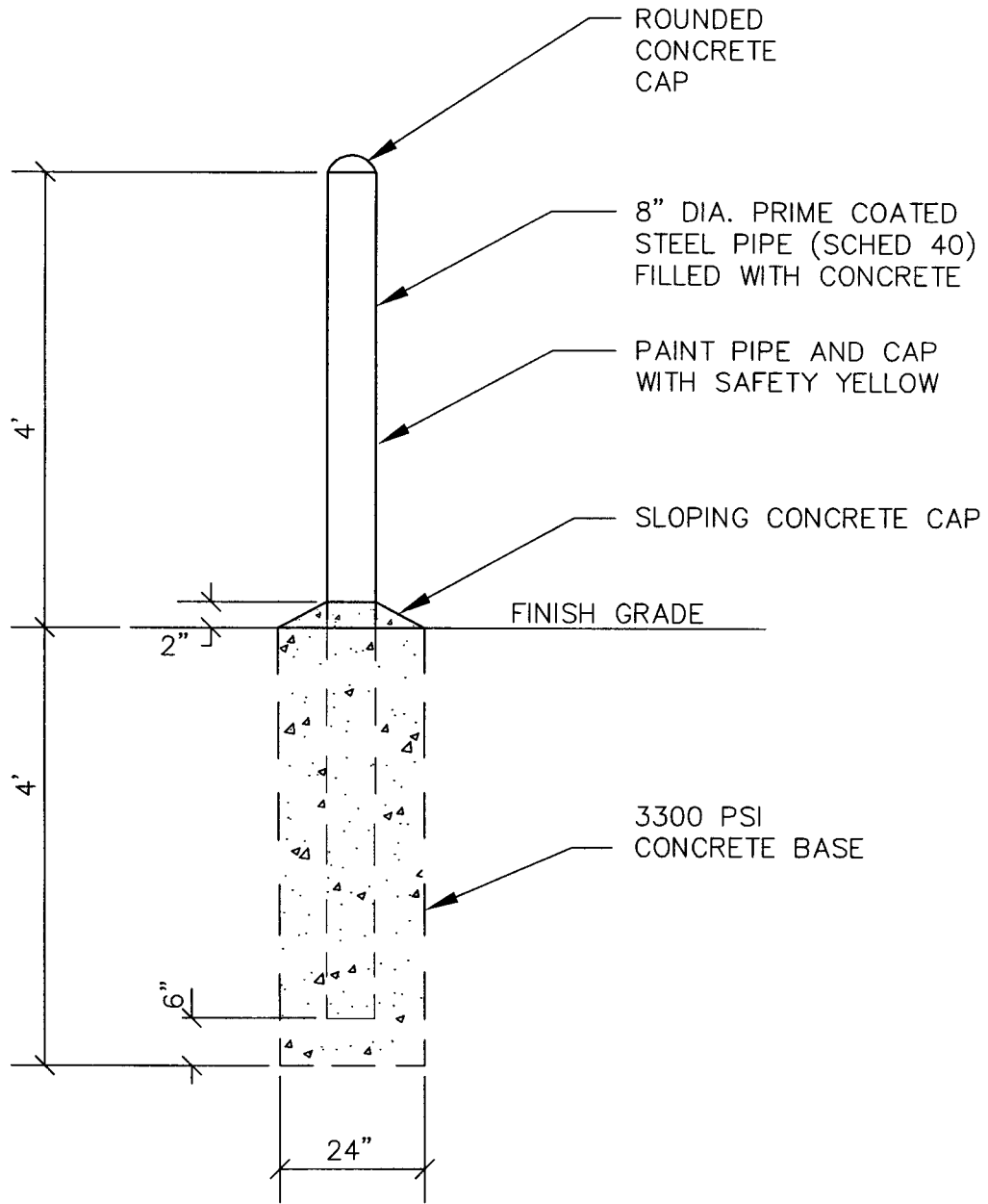
**6-INCH BOLLARD
(GUARD POST)**

(NTS)

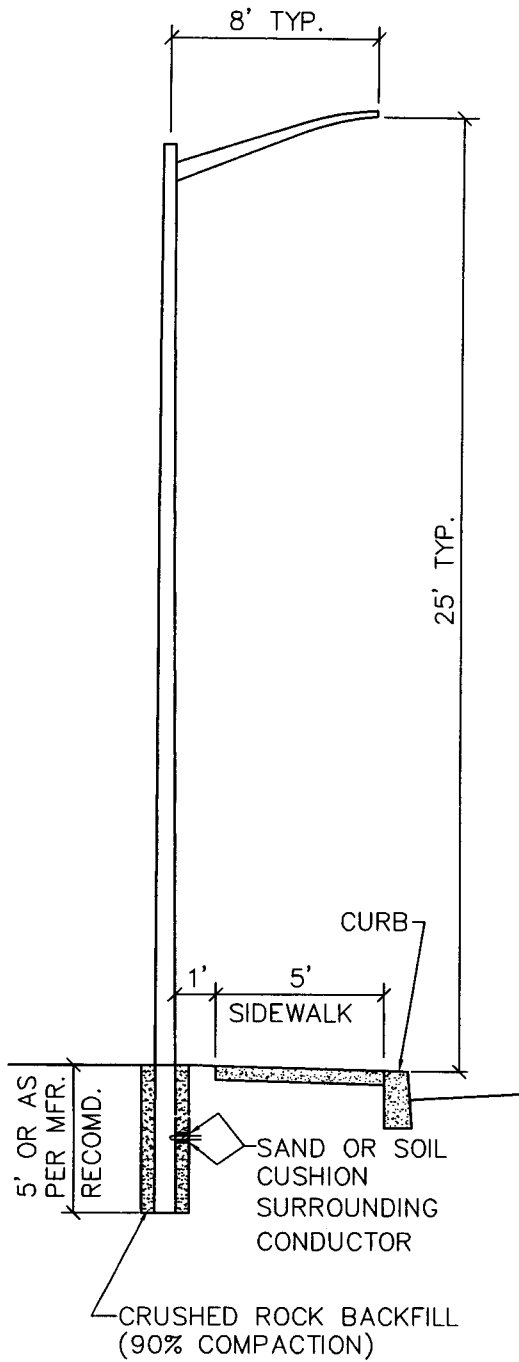
CARLTON, OR

DETAIL NO.

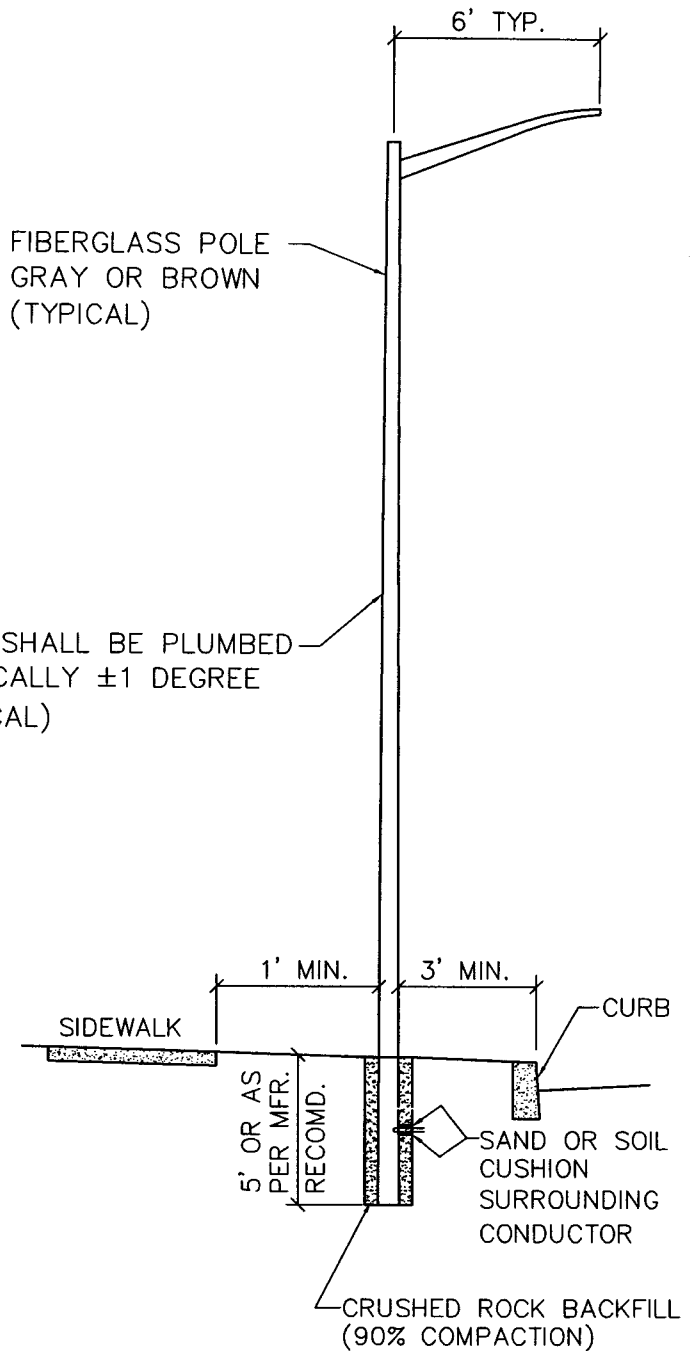
226



LAST REVISION DATE: DEC 2007	COPYRIGHT 1996 WESTECH ENGINEERING, INC.
8-INCH BOLLARD (GUARD POST)	
(NTS)	
CARLTON, OR	DETAIL NO. 227



TYPICAL LAMP POST
CROSS SECTION TYPE ONE



TYPICAL LAMP POST
CROSS SECTION TYPE TWO

FIBERGLASS POLE
GRAY OR BROWN
(TYPICAL)

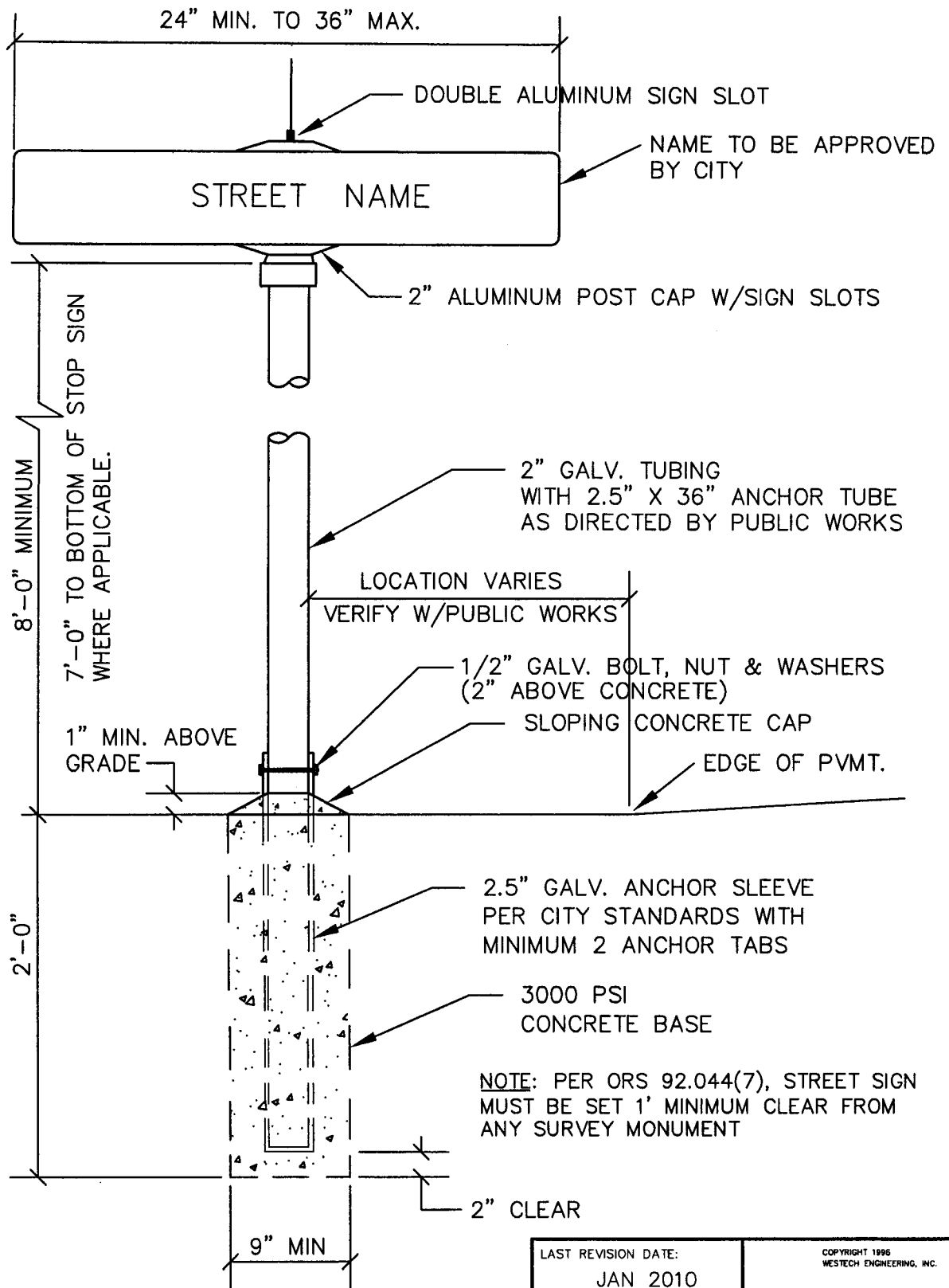
POLE SHALL BE PLUMBED
VERTICALLY ± 1 DEGREE
(TYPICAL)

NOTES:

1. CONTRACTOR TO COORDINATE W/LOCAL POWER COMPANY FOR MATERIALS AND WORKMANSHIP REQUIREMENTS.

NOTE: PER ORS 92.044(7), STREET LIGHT MUST BE SET 1' MINIMUM CLEAR FROM ANY SURVEY MONUMENT

LAST REVISION DATE: MAR 2008	COPYRIGHT 1996 WESTECH ENGINEERING, INC.
TYPICAL STREET LAMP POST	
(NTS)	
CARLTON, OR	DETAIL NO. 230



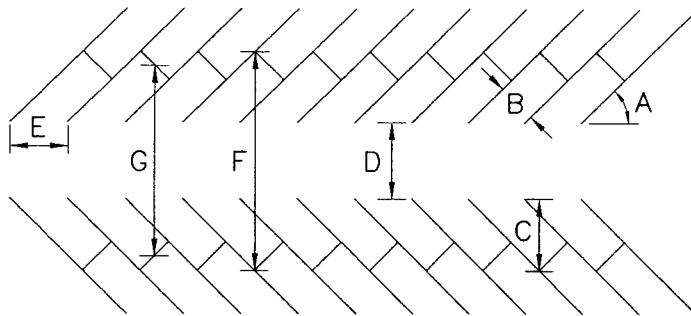
NOTES:

1. ALL NEWLY PLATTED STREETS TO BE SIGNED IN ACCORDANCE WITH CITY STANDARDS.
2. SIGN PANELS TO CONFORM TO SECTION 00940 OF OSHD SPECIFICATIONS AS TO MATERIALS.
3. ALL SIGNS SHALL BE IN CONFORMANCE WITH THE STATE OF OREGON UNIFORM TRAFFIC MANUAL.

LAST REVISION DATE: JAN 2010	COPYRIGHT 1996 WESTECH ENGINEERING, INC.
SIGN POST FOR STREET SIGNS, STOP SIGNS & TRAFFIC CONTROL SIGNS (NTS)	
CARLTON, OR	DETAIL NO. 232

OFF-STREET PARKING DIMENSIONS

STALL WIDTH DIMENSIONS MAY BE DISTRIBUTED AS FOLLOWS:
 60% STANDARD SPACES, 40% MAXIMUM COMPACT SPACES.
 ALL COMPACT SPACES SHALL BE PERMANENTLY LABELED.



- A- PARKING ANGLE
- B- STALL WIDTH
- C- STALL TO CURB DEPTH
- D- AISLE WIDTH BETWEEN STALL LINES
- E- STALL WIDTH PARALLEL TO AISLE
- F- MODULE WIDTH (FRONT OF STALL TO FRONT OF STALL)
- G- MODULE WIDTH (FRONT OF STALL TO FRONT OF STALL AT BUMPER MIDPOINT)

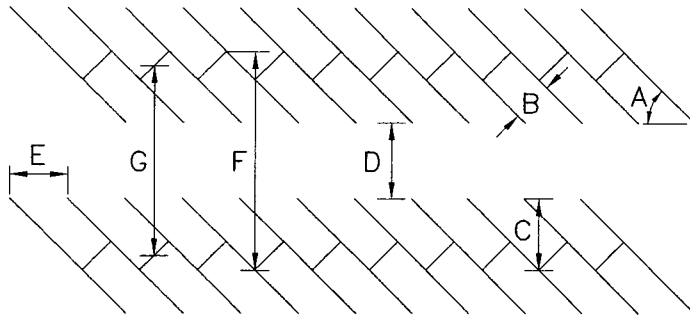
<u>OFF-STREET PARKING MATRIX</u>													
MINIMUM PARKING SPACE AND AISLE DIMENSIONS (FT)													
ONE WAY TRAFFIC FLOW													
COMPACT (8.5' x 16')							STANDARD (9' x 19')						
A	B	C	D	E	F	G	B	C	D	E	F	G	
0°	8.5	8.5	12.0	19.0	28.0	—	9.0	9.0	12.0	22.0	28.0	—	
30°	8.5	15.4	12.0	17.0	41.7	34.4	9.0	17.3	12.0	18.0	45.6	37.8	
45°	8.5	17.3	13.0	12.0	47.6	41.6	9.0	19.8	13.0	12.7	52.6	46.2	
60°	8.5	18.1	18.0	9.8	54.2	50.0	9.0	21.0	18.0	10.4	60.0	55.7	
70°	8.5	17.9	19.0	9.0	54.9	52.0	9.0	21.0	19.0	9.6	61.0	57.8	
90°	8.5	16.0	24.0	8.5	56.0	56.0	9.0	19.0	24.0	9.0	62.0	62.0	

NOTE:
 WHERE DRIVE AISLE "D" IS A FIRE LANE, WIDTHS SHALL CONFORM WITH THE OREGON FIRE CODE MINIMUMS OF 20 FEET IN ALL CASES (26 FOOT MINIMUM ADJACENT TO FIRE HYDRANTS), PER OFC 503.2.1 & D103.1.

LAST REVISION DATE: MAY 2009	COPYRIGHT 1996 WESTECH ENGINEERING, INC.
OFFSTREET PARKING DIMENSIONS ONE WAY TRAFFIC FLOW (NTS)	
CARLTON, OR	DETAIL NO. 235

OFF-STREET PARKING DIMENSIONS

STALL WIDTH DIMENSIONS MAY BE DISTRIBUTED AS FOLLOWS:
 60% STANDARD SPACES, 40% MAXIMUM COMPACT SPACES.
 ALL COMPACT SPACES SHALL BE PERMANENTLY LABELED.



- A- PARKING ANGLE
- B- STALL WIDTH
- C- STALL TO CURB DEPTH
- D- AISLE WIDTH BETWEEN STALL LINES
- E- STALL WIDTH PARALLEL TO AISLE
- F- MODULE WIDTH (FRONT OF STALL TO FRONT OF STALL)
- G- MODULE WIDTH (FRONT OF STALL TO FRONT OF STALL AT BUMPER MIDPOINT)

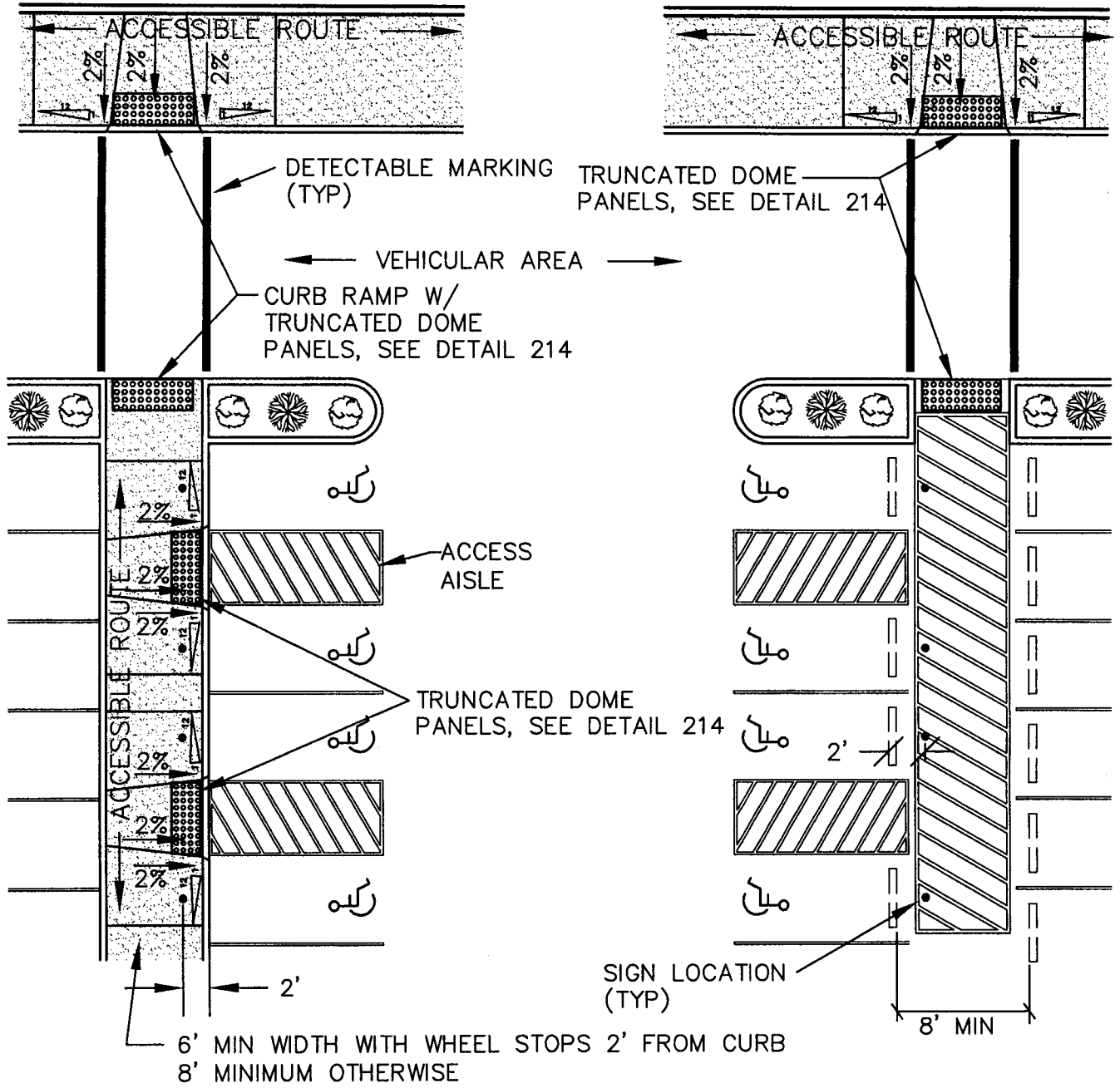
<u>OFF-STREET PARKING MATRIX</u>													
MINIMUM PARKING SPACE AND AISLE DIMENSIONS (FT)													
TWO WAY TRAFFIC FLOW													
COMPACT (8.5' x 16')							STANDARD (9' x 19')						
A	B	C	D	E	F	G	B	C	D	E	F	G	
0°	8.5	8.5	24.0	19.0	28.0	—	9.0	9.0	24.0	22.0	28.0	—	
30°	8.5	15.4	24.0	17.0	41.7	34.4	9.0	17.3	24.0	18.0	45.6	37.8	
45°	8.5	17.3	24.0	12.0	47.6	41.6	9.0	19.8	24.0	12.7	52.6	46.2	
60°	8.5	18.1	24.0	9.8	54.2	50.0	9.0	21.0	24.0	10.4	60.0	55.7	
70°	8.5	17.9	24.0	9.0	54.9	52.0	9.0	21.0	24.0	9.6	61.0	57.8	
90°	8.5	16.0	24.0	8.5	56.0	56.0	9.0	19.0	24.0	9.0	62.0	62.0	

NOTE:
 WHERE DRIVE AISLE "D" IS A FIRE LANE, WIDTHS SHALL CONFORM WITH THE OREGON FIRE CODE MINIMUMS OF 20 FEET IN ALL CASES (26 FOOT MINIMUM ADJACENT TO FIRE HYDRANTS), PER OFC 503.2.1 & D103.1.

LAST REVISION DATE: MAY 2009	<small>COPYRIGHT 1996 WESTECH ENGINEERING, INC.</small>
OFFSTREET PARKING DIMENSIONS TWO WAY TRAFFIC FLOW (NTS)	
CARLTON, OR	DETAIL NO. 236

BUILDING

BUILDING



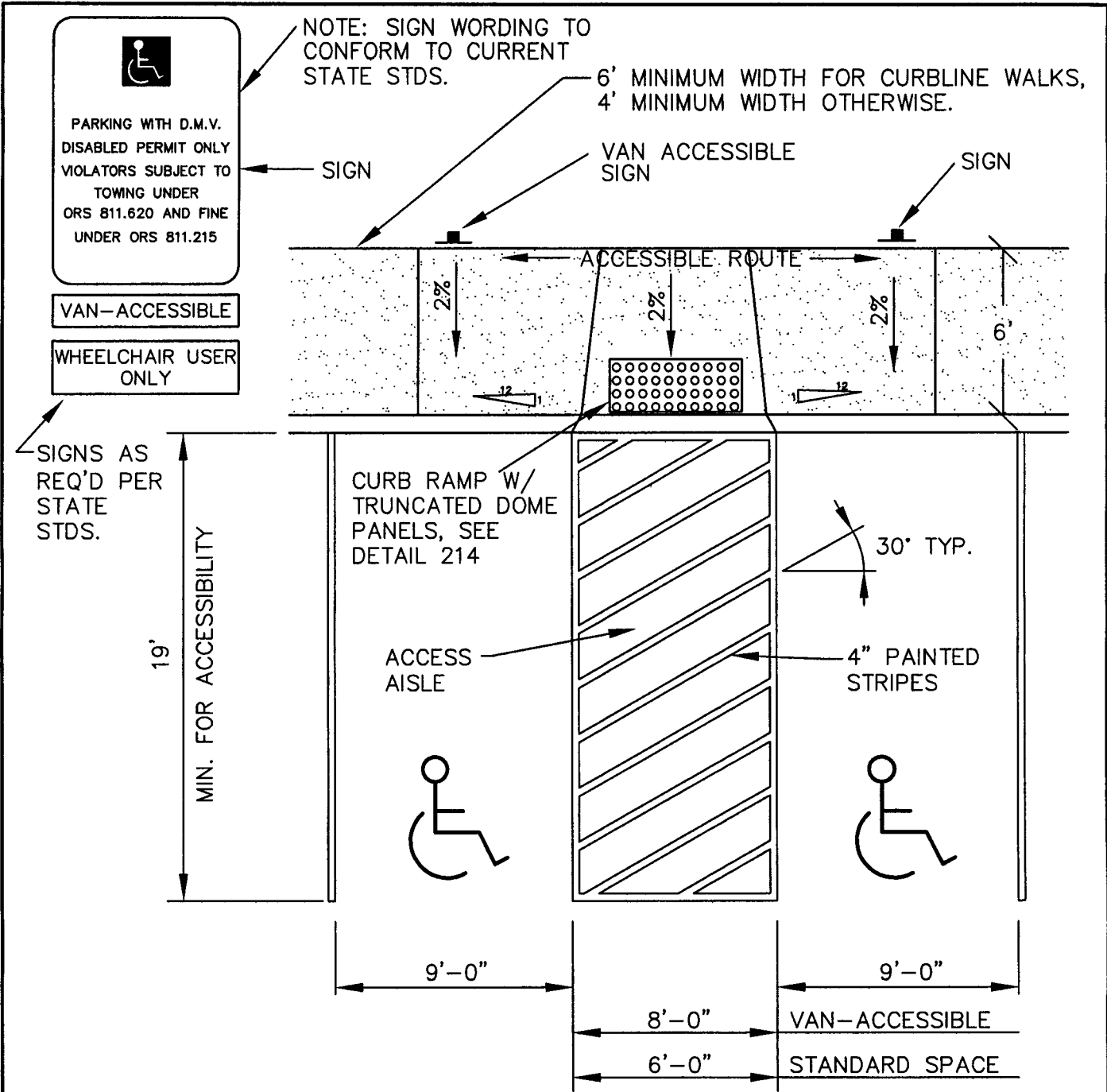
ACCESSIBLE PARKING PLAN ①

ACCESSIBLE PARKING PLAN ②

NOTES:

- 1. SEE DETAIL 237 FOR ACCESSIBLE PARKING PARKING SPACE LAYOUT.

LAST REVISION DATE: JAN 2010	
ACCESSIBLE ROUTES AND CROSSINGS IN VEHICULAR AREAS (NTS)	
CARLTON, OR	DETAIL NO. 238

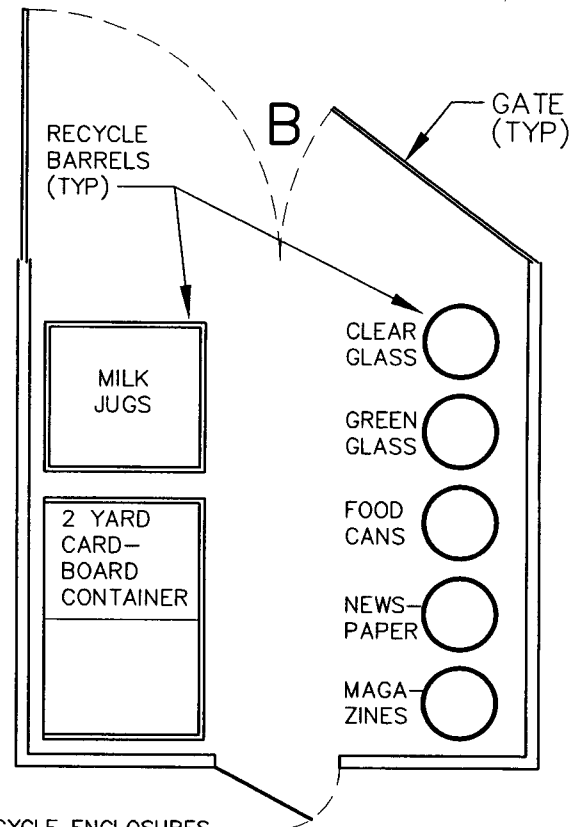
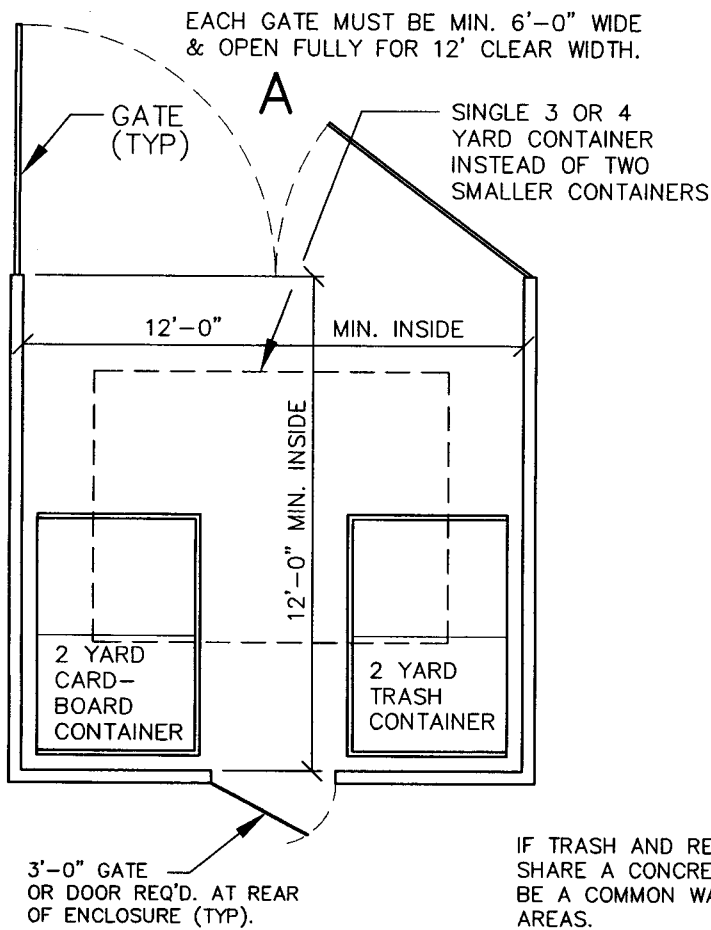


DOUBLE ACCESSIBLE PARKING SPACE

NOTES:

1. ONE ACCESSIBLE PARKING SPACE MUST BE DESIGNATED "VAN-ACCESSIBLE", THE OTHER SPACE CAN BE EITHER "VAN-ACCESSIBLE" OR STANDARD PARKING SPACE.
2. VAN-ACCESSIBLE OR WHEELCHAIR ONLY SPACES SHALL HAVE AN ADDITIONAL SIGN MOUNTED BELOW THE STANDARD PARKING SPACE PARKING SIGN.
3. VAN-ACCESSIBLE SPACE CAN BE USED BY ANY VEHICLE WITH A DMV DISABLED PERMIT.
4. MAXIMUM 2% CROSS SLOPE ALLOWED IN PARKING SPACE OR ACCESS AISLE.
5. POST MOUNTED SIGNS SHALL HAVE 7' (±3") CLEARANCE FROM SIGN BOTTOM TO GROUND.

LAST REVISION DATE: JAN 2010	
DOUBLE ACCESSIBLE PARKING SPACE	
(NTS)	
CARLTON, OR	DETAIL NO. 237



TRASH ENCLOSURE

RECYCLE ENCLOSURE

NOTES:

1. GATES:
 - (a) ALL GATES MUST ATTACH AT THE END OF OF THE WALLS TO PROVIDE A MINIMUM OF 12' CLEAR WORKING SPACE WHEN OPEN.
 - (b) TO SERVICE THE ENCLOSURE, THE GATES MUST BE ABLE TO BE PINNED IN MUST BE ABLE TO BE PINNED IN THE FULL OPEN POSITION.
 - (c) GATES MUST OPEN FROM OUTSIDE THE ENCLOSURE.
2. FOR 5 OR 6 YARD CONTAINERS THE ENCLOSURE DEPTH MUST BE 15'.
3. WHERE REQ'D. (I.E. RESTAURANTS), GREASE BARRELS MUST BE SEPARATE FROM TRASH AND RECYCLING ENCLOSURES.
4. ROOFS OR OVERHANGS SHALL HAVE 15' OF OVERHEAD CLEARANCE.
5. IF RECYCLING IS NOT INCLUDED, AREA (A) CAN PROVIDE SERVICE FOR TRASH AND CARDBOARD FOR CONTAINER SIZES OF 1 TO 2 YARDS. IF A 3 YARD OR LARGER TRASH CONTAINER IS NEEDED, AN ADDITIONAL 12' X 12' SPACE WILL BE NECESSARY FOR CARDBOARD CONTAINER SERVICE.
6. CONCRETE PADS REQUIRED FOR ALL ENCLOSURES. WALL, GATE & DOORS PER CITY STANDARDS.
7. A 1 YD. CONTAINER WILL HOLD APPROXIMATELY THE SAME AS 6 TRASH CANS (32 GAL SIZE). USE 6 TIMES THE CONTAINER SIZE IN YARDS TO ESTIMATE A CONTAINER CAPACITY. FOR EXAMPLE, A 3 YD. CONTAINER WILL HOLD APPROX. THE SAME AMOUNT AS 18 TRASH CANS (32 GAL SIZE).

LAST REVISION DATE:
DEC 2007

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TYPICAL TRASH AND RECYCLING ENCLOSURE

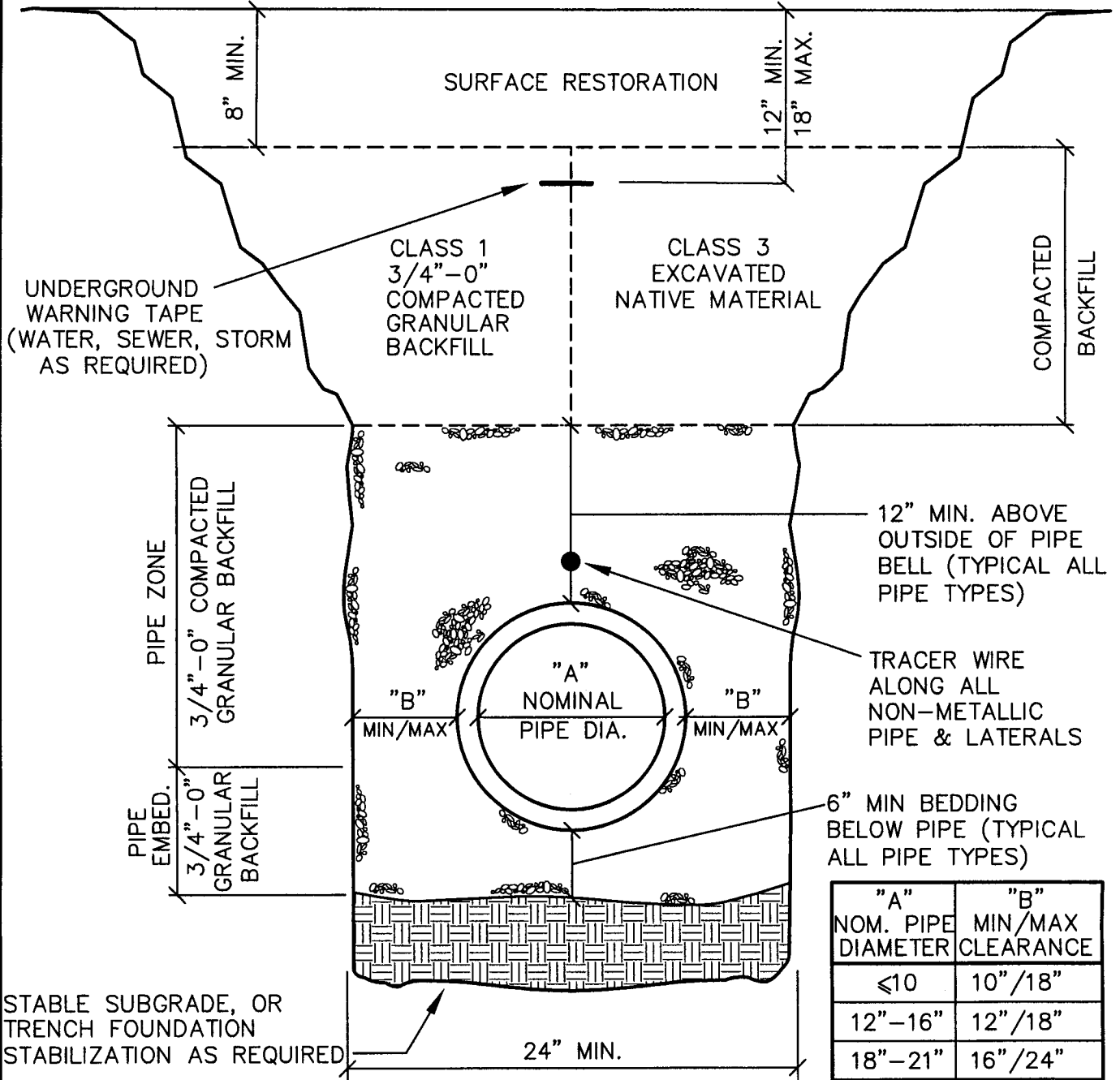
(NTS)

CARLTON, OR

DETAIL NO.

240

COMPACTION: CLASS 1 - 92% OPTIMUM PER AASHTO T-180
 CLASS 3 - 85% OPTIMUM PER AASHTO T-180



"A" NOM. PIPE DIAMETER	"B" MIN/MAX CLEARANCE
≤10	10"/18"
12"-16"	12"/18"
18"-21"	16"/24"
24"-30"	18"/30"
>30"	24"/36"

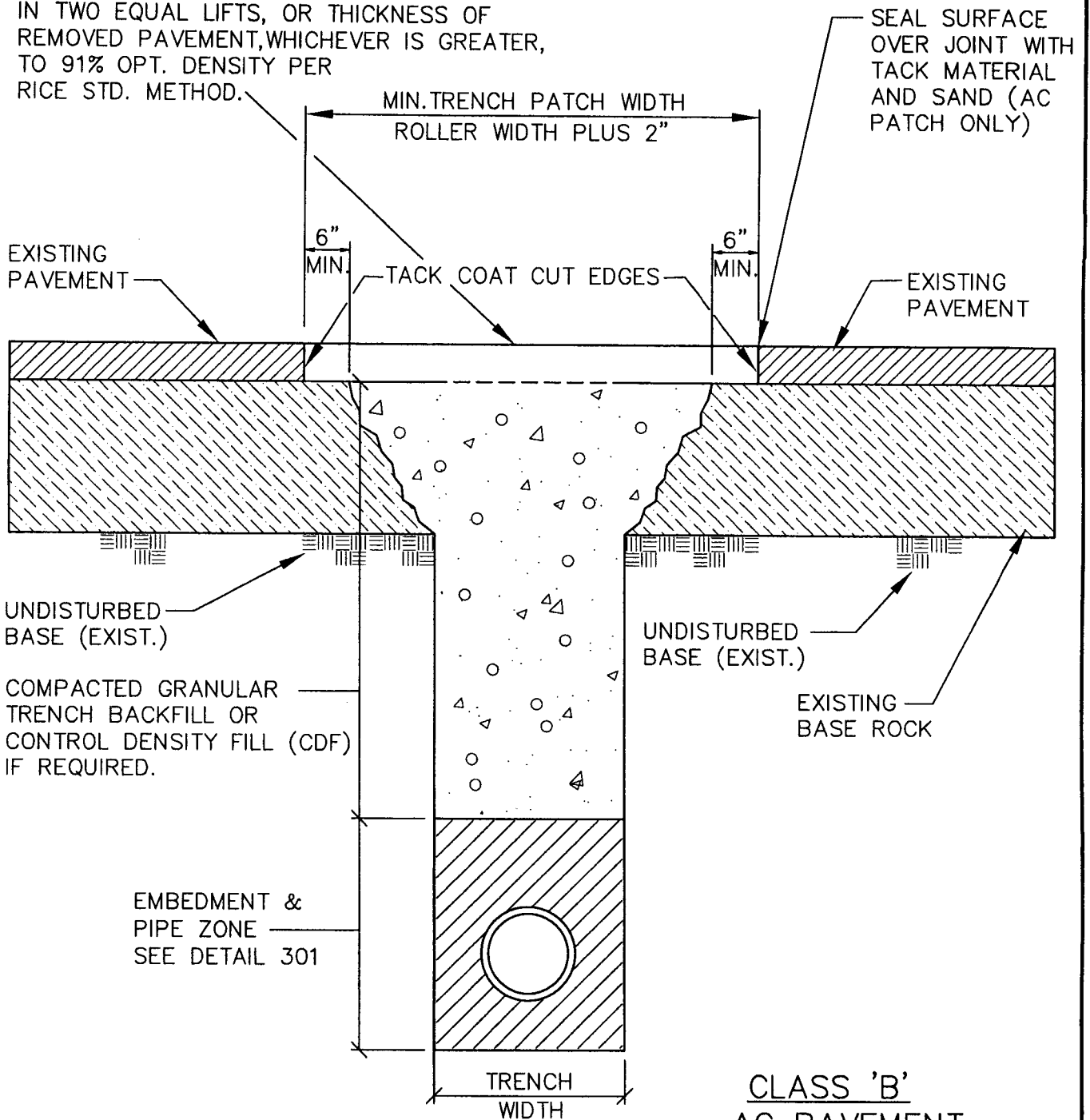
STABLE SUBGRADE, OR
 TRENCH FOUNDATION
 STABILIZATION AS REQUIRED

NOTES:

1. CLASS 1 REQ'D. UNDER ALL EXIST. OR FUTURE IMPROVED AREAS INCLUDING SIDEWALKS.
2. WHERE NEW PIPING IS IN SAME ALIGNMENT AS EXISTING PIPING, THE PIPE EMBEDMENT SHALL EXTEND TO A MIN. OF 6" BELOW THE NEW PIPING OR 6" BELOW EXISTING PIPING, WHICHEVER IS DEEPER.
3. FOR FLEXIBLE PIPE, BOTTOM OF TRENCH SHORING SHALL BE ABOVE PIPE SPRINGLINE PRIOR TO COMPACTING BACKFILL BELOW THE PIPE SPRINGLINE AND UNDER THE PIPE HAUNCHES.

LAST REVISION DATE: JAN 2010	
TRENCH BACKFILL, BEDDING, AND PIPE ZONE (NTS)	
CARLTON, OR	DETAIL NO. 301

PLACE 4" MIN. THICKNESS, CL.'C' A.C. IN TWO EQUAL LIFTS, OR THICKNESS OF REMOVED PAVEMENT,WHICHEVER IS GREATER, TO 91% OPT. DENSITY PER RICE STD. METHOD.



**CLASS 'B'
AC PAVEMENT**

NOTES:

1. ALL EXISTING AC OR PCC PAVEMENT SHALL BE SAWCUT PRIOR TO REPAVING.
2. PCC CONCRETE PAVEMENT SHALL BE REPLACED WITH 3300 PSI PCC TO A MINIMUM THICKNESS OF 6" OR TO THE THICKNESS OF REMOVED CONCRETE, WHICHEVER IS GREATER.
3. FOR PAVED DRIVEWAYS, (EXCEPT COMMERCIAL OR INDUSTRIAL), PAVEMENT THICKNESS MAY BE REDUCED TO 3" IN 2 LIFTS, AND OVERCUT MAY BE REDUCED TO 3" EACH SIDE.

LAST REVISION DATE: APR 2009	
MINOR OR PRIVATE STREET AND AC DRIVEWAY CUT SURFACE RESTORATION (NTS)	
CARLTON, OR	DETAIL NO. 302

PLACE 4" MIN. THICKNESS, CL.'C' A.C.
IN TWO LIFTS. COMPACT TO
91% OPTIMUM DENSITY PER
RICE STD. METHOD.

36" MIN. WIDTH
PETROMAT OR
EQUAL

SEAL SURFACE
OVER JOINT WITH
TACK MATERIAL
AND SAND.

GRIND BENCH INTO EXTG
AC PAVEMENT. SEE
NOTE 1 BELOW. 18"
MIN. FINISH WIDTH.

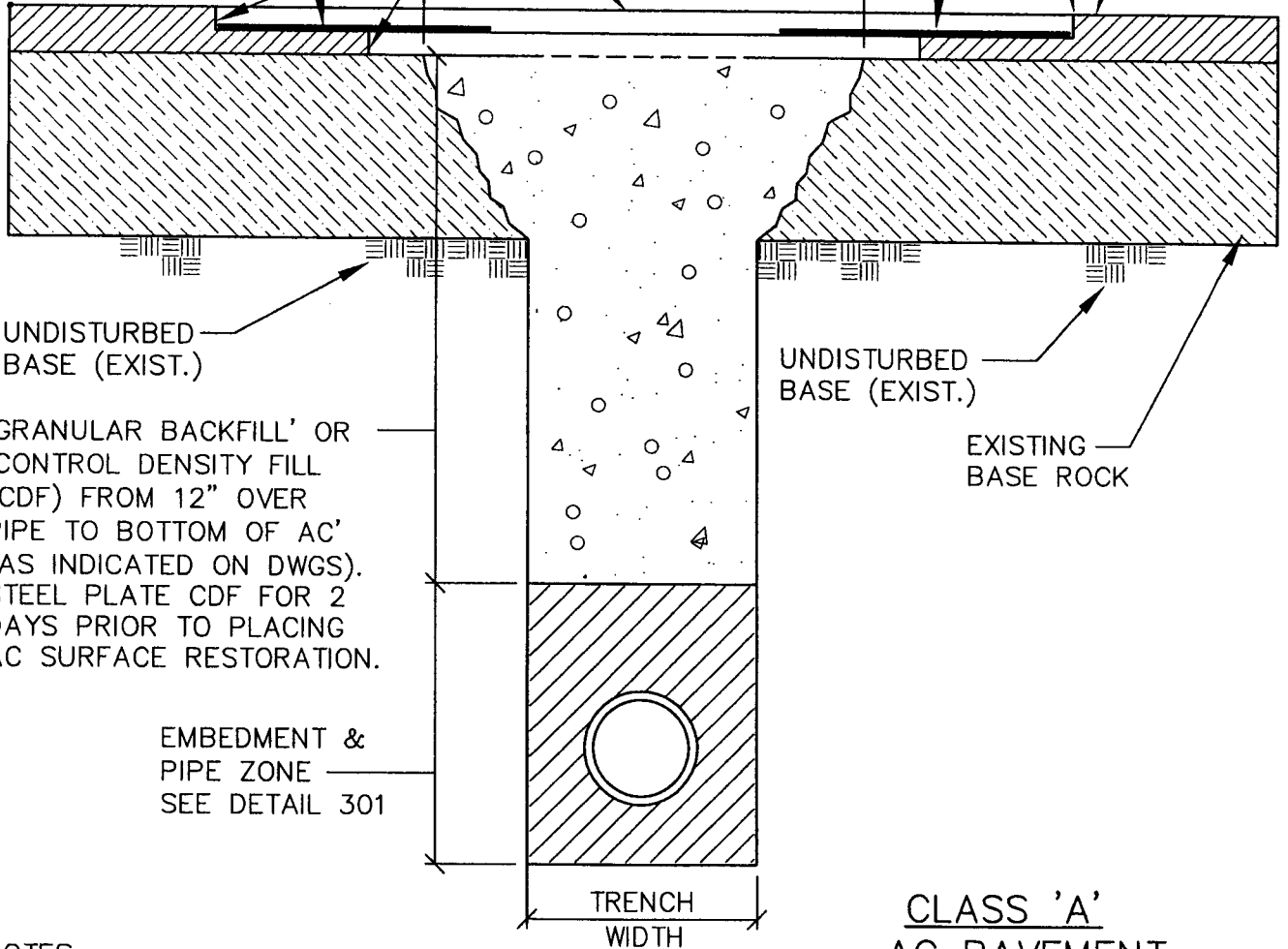
MIN. TRENCH PATCH WIDTH
ROLLER WIDTH PLUS 2"

6"
MIN.

TACK COAT CUT EDGES

6"
MIN.

EXISTING
PAVEMENT



UNDISTURBED
BASE (EXIST.)

'GRANULAR BACKFILL' OR
'CONTROL DENSITY FILL
(CDF) FROM 12" OVER
PIPE TO BOTTOM OF AC'
(AS INDICATED ON DWGS).
STEEL PLATE CDF FOR 2
DAYS PRIOR TO PLACING
AC SURFACE RESTORATION.

EMBEDMENT &
PIPE ZONE
SEE DETAIL 301

UNDISTURBED
BASE (EXIST.)

EXISTING
BASE ROCK

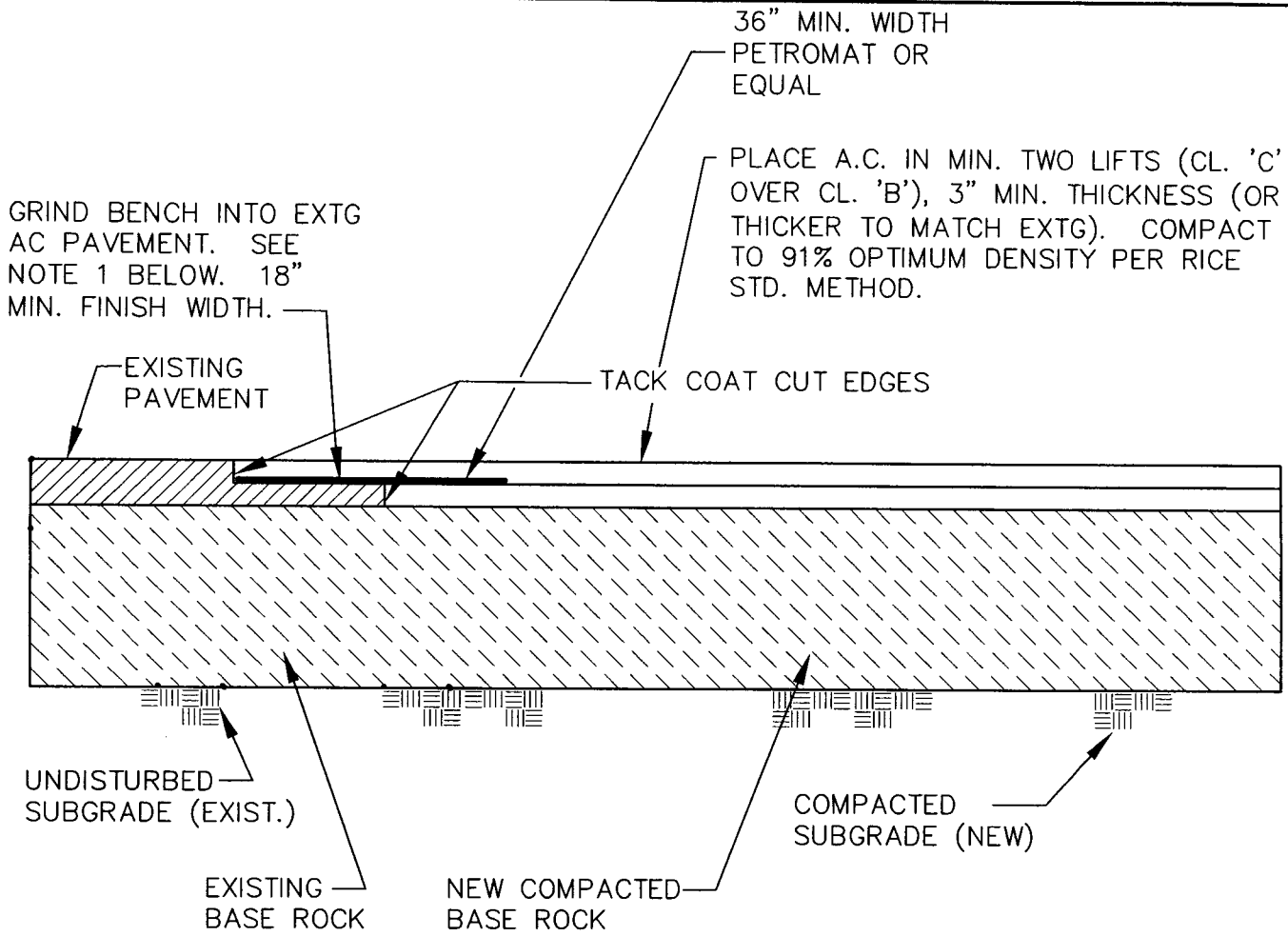
TRENCH
WIDTH

CLASS 'A'
AC PAVEMENT

NOTES:

1. FOLLOWING BACKFILL COMPACTION OR CDF INSTALLATION, GRIND 24" WIDE BENCH IN EXISTING AC ON BOTH SIDES & TRENCH END, 1-1/2" DEEP OR HALF THE DEPTH OF EXISTING AC, WHICHEVER IS GREATER.
2. FOLLOWING GRINDING, SAWCUT ALL TRENCH EDGES 6" BACK FROM TRENCH EDGE.
3. TACK COAT CUT EDGES AND INSTALL BASE LIFT OF AC TO LEVEL OF BENCH GRIND.
4. TACK COAT FOR FABRIC & INSTALL TOP LIFT OF AC. SAND SEAL ALL EDGES.

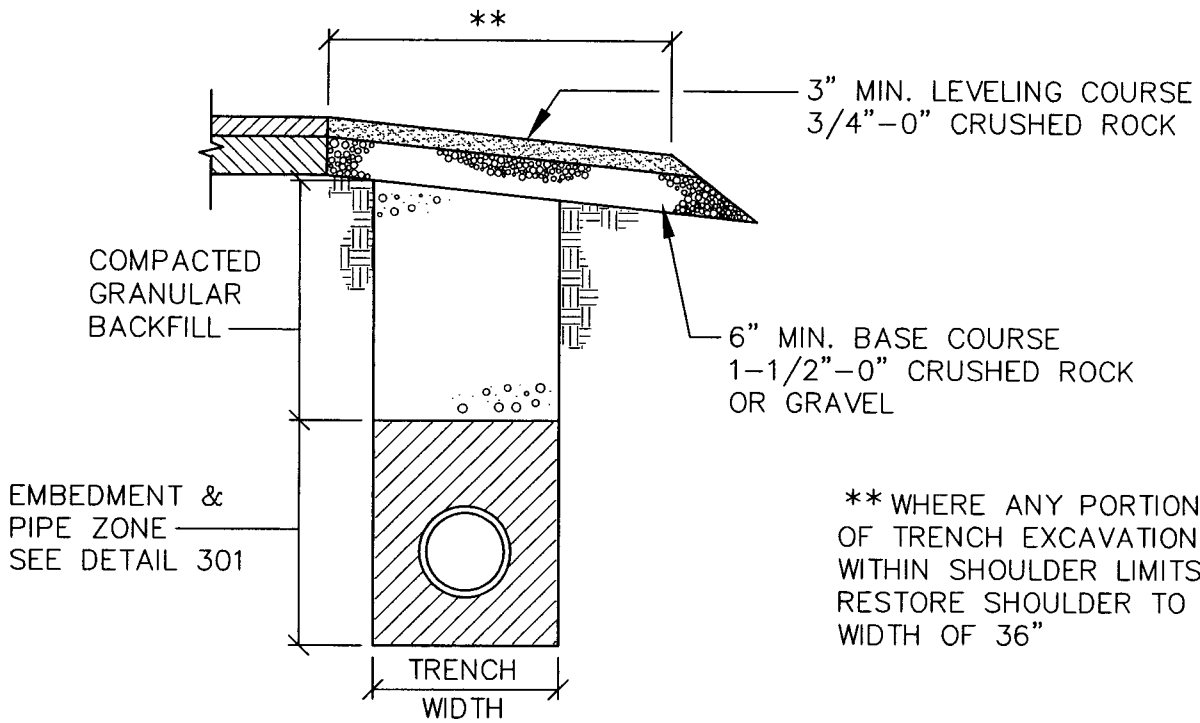
LAST REVISION DATE:	
APR 2009	
AC STREET CUT SURFACE RESTORATION W/BENCH GRIND (NTS)	
CARLTON, OR	DETAIL NO. 302A



NOTES:

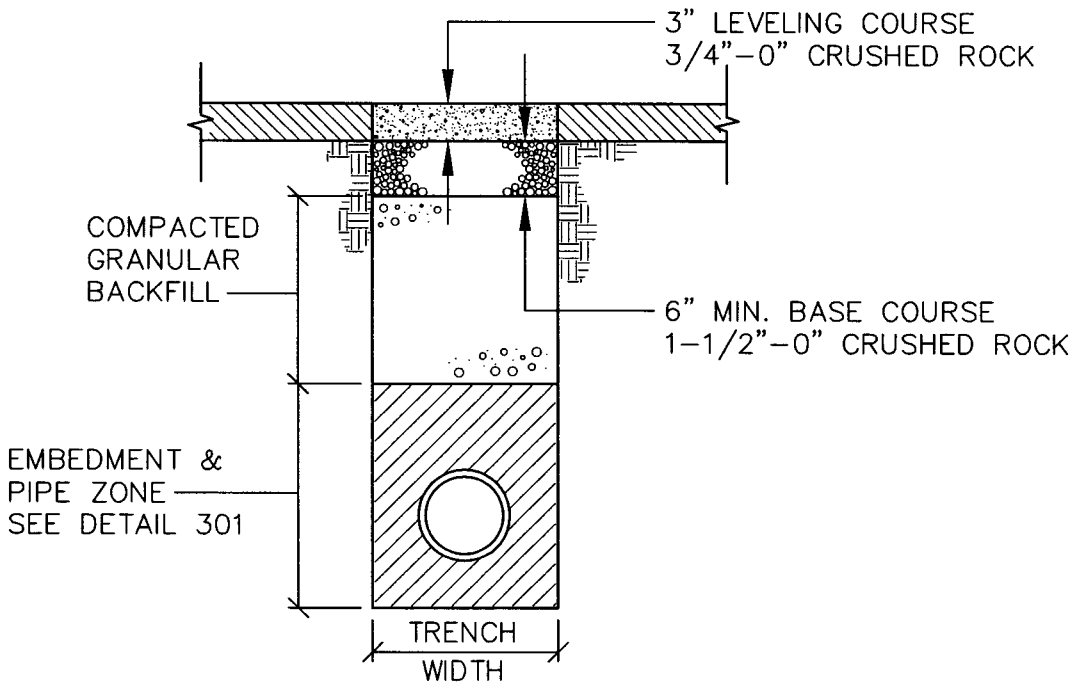
1. PRIOR TO SAWCUTTING, GRIND BENCH IN EXISTING AC LESSER OF 2" DEEP OR HALF THICKNESS OF EXISTING AC. BENCH TO EXTEND TO A POINT 18" MINIMUM BACK FROM SAWCUT LOCATION.
2. FOLLOWING GRINDING, SAWCUT PAVEMENT EDGES.
3. TACK COAT CUT EDGES AND INSTALL BASE LIFT OF AC TO LEVEL OF BENCH GRIND.
4. TACK COAT FOR PETROMAT FABRIC & INSTALL TOP LIFT OF AC. SAND SEAL ALL JOINTS.
5. SEE STANDARD STREET SECTION DETAILS FOR BASEROCK & AC THICKNESS, FABRIC, ETC.

LAST REVISION DATE: DEC 2007	
AC STREET CUT FOR STREET WIDENING	
(NTS)	
CARLTON, OR	DETAIL NO. 302B



** WHERE ANY PORTION OF TRENCH EXCAVATION FALLS WITHIN SHOULDER LIMITS, RESTORE SHOULDER TO MIN. WIDTH OF 36"

CLASS 'C'
GRAVEL SHOULDER

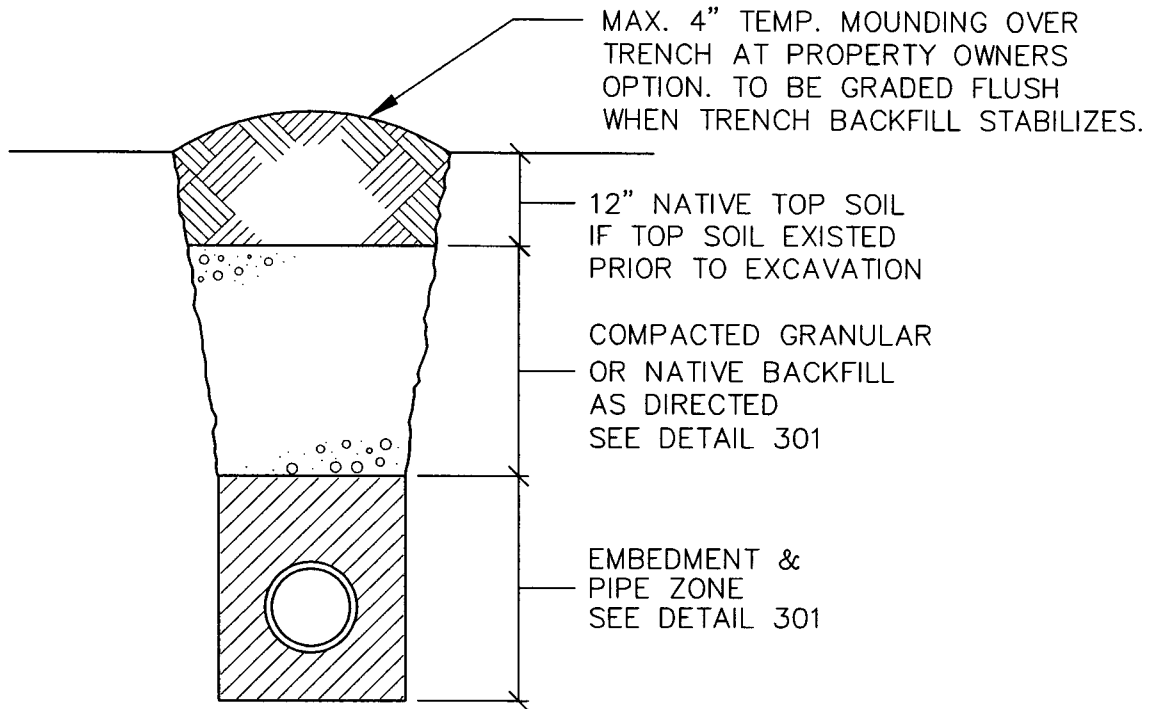


CLASS 'D'
GRAVEL STREET

NOTES:

1. COMPACTION STANDARD WILL BE 92% OPTIMUM PER AASHTO T-180

LAST REVISION DATE: DEC 2007	
GRAVEL SURFACE RESTORATION	
(NTS)	
CARLTON, OR	DETAIL NO. 303

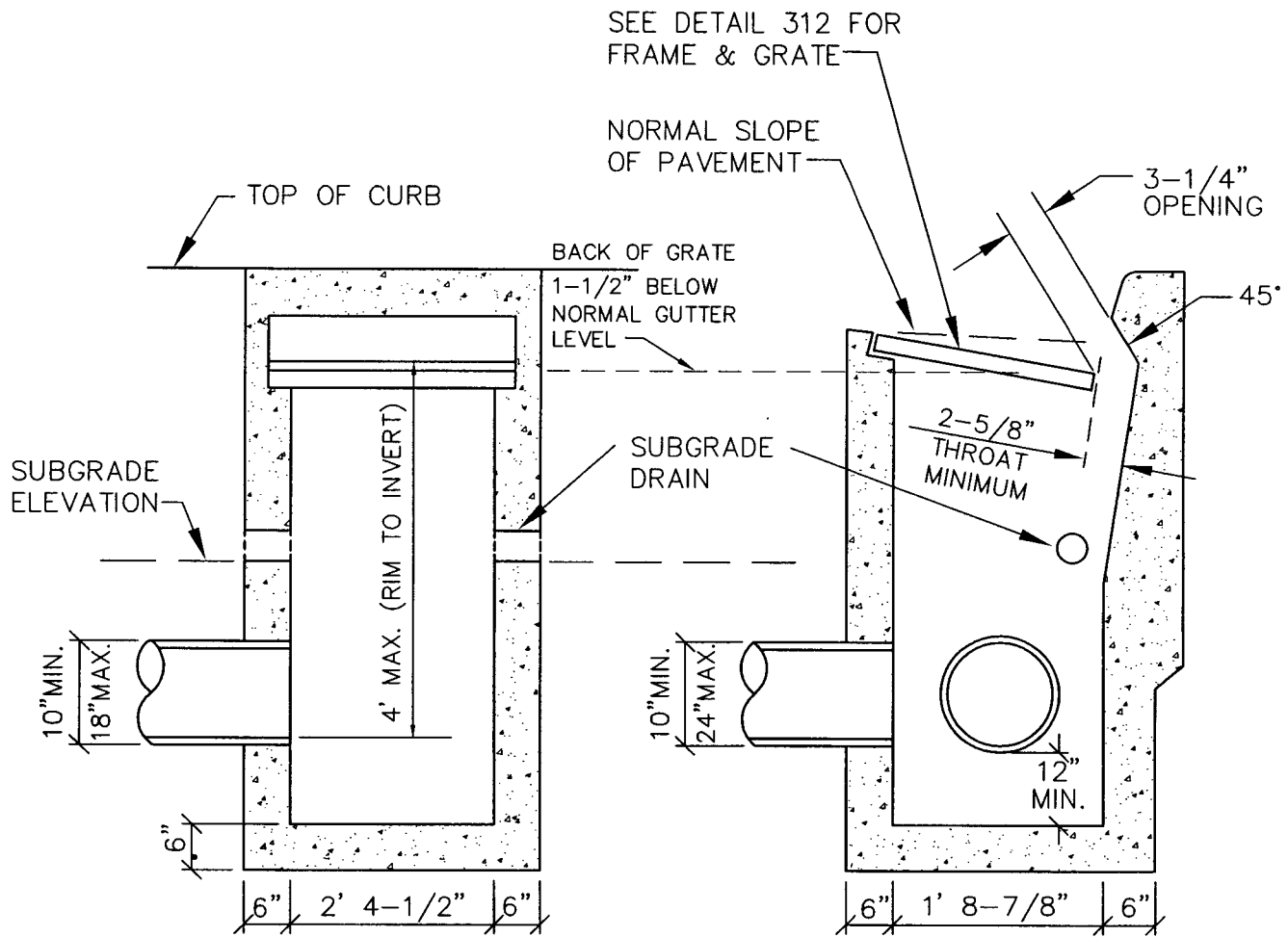


CLASS 'E'
UNIMPROVED & OPEN AREAS

NOTES:

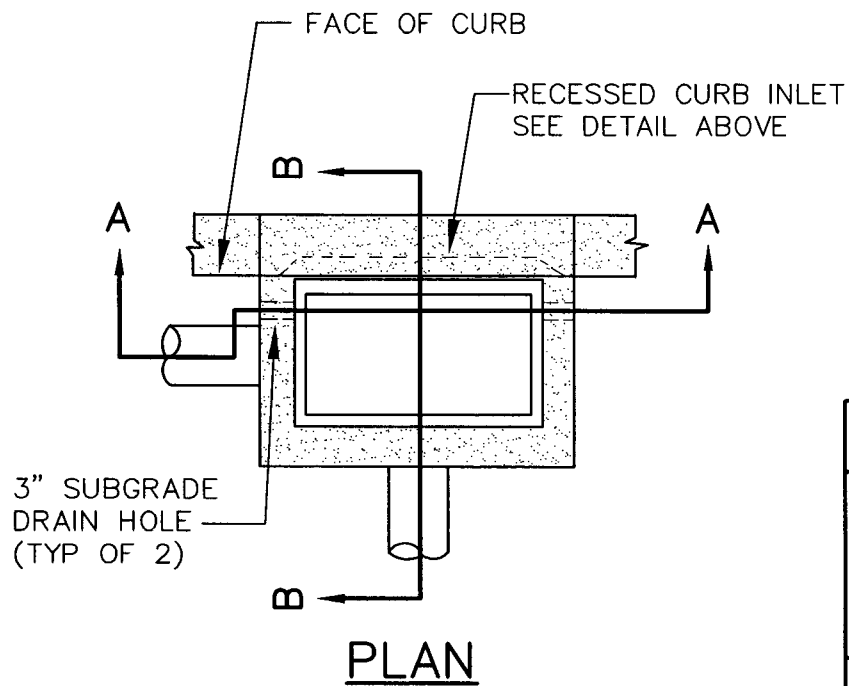
1. COMPACTION STANDARD WILL BE AASHTO T-180.

LAST REVISION DATE: DEC 2007	
NATIVE SURFACE RESTORATION	
(NTS)	
CARLTON, OR	DETAIL NO. 304



SECTION A-A

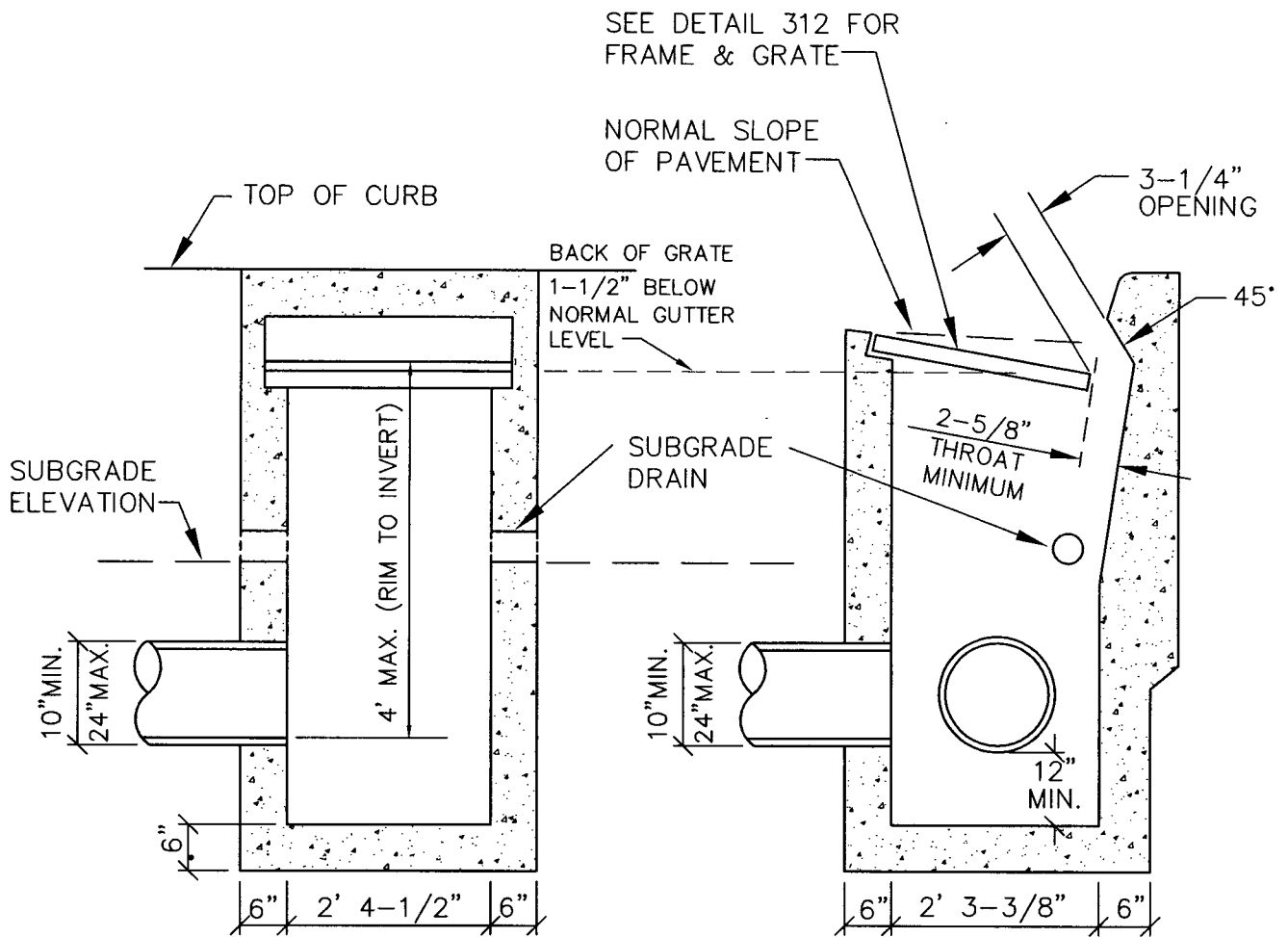
SECTION B-B



NOTES:

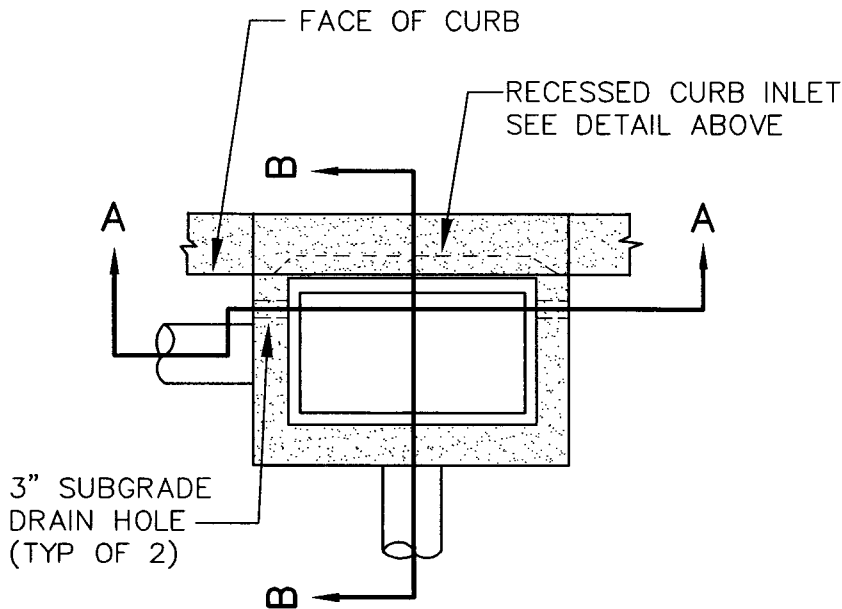
1. SEE CONSTRUCTION DRAWINGS FOR PIPE SIZE, LOCATION AND INVERT ELEVATION.
2. ALL CONCRETE TO BE 3300 PSI @ 28 DAYS.
3. MATCH EXISTING CURB UNLESS OTHERWISE NOTED.

LAST REVISION DATE: DEC 2007	COPYRIGHT 1996 WESTECH ENGINEERING, INC.
STANDARD SIDE-INLET GRATED CATCH BASIN	
(NTS)	
CARLTON, OR	DETAIL NO. 310



SECTION A-A

SECTION B-B

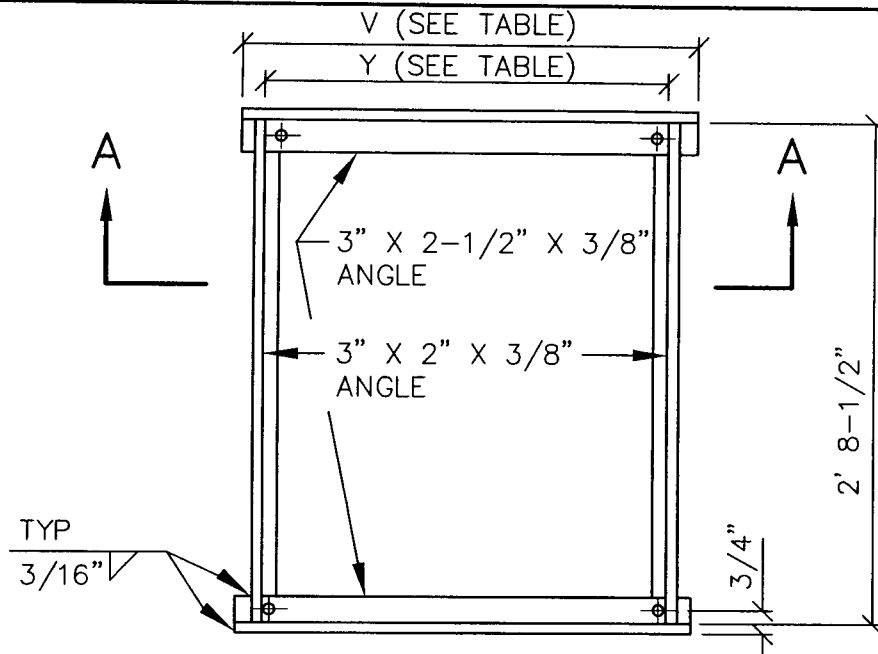


PLAN

NOTES:

1. SEE CONSTRUCTION DRAWINGS FOR PIPE SIZE, LOCATION AND INVERT ELEVATION.
2. ALL CONCRETE TO BE 3000 PSI @ 28 DAYS.
3. MATCH EXISTING CURB UNLESS OTHERWISE NOTED.

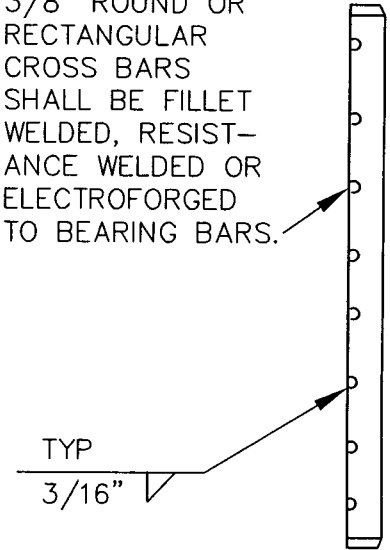
LAST REVISION DATE: DEC 2007	COPYRIGHT 1996 WESTECH ENGINEERING, INC.
OVERSIZE SIDE-INLET GRATED CATCH BASIN	
(NTS)	
CARLTON, OR	DETAIL NO. 311



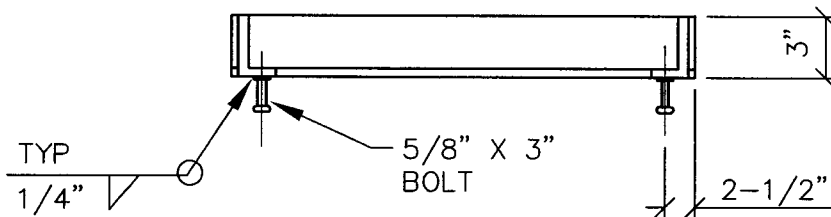
PLAN

NOTE:

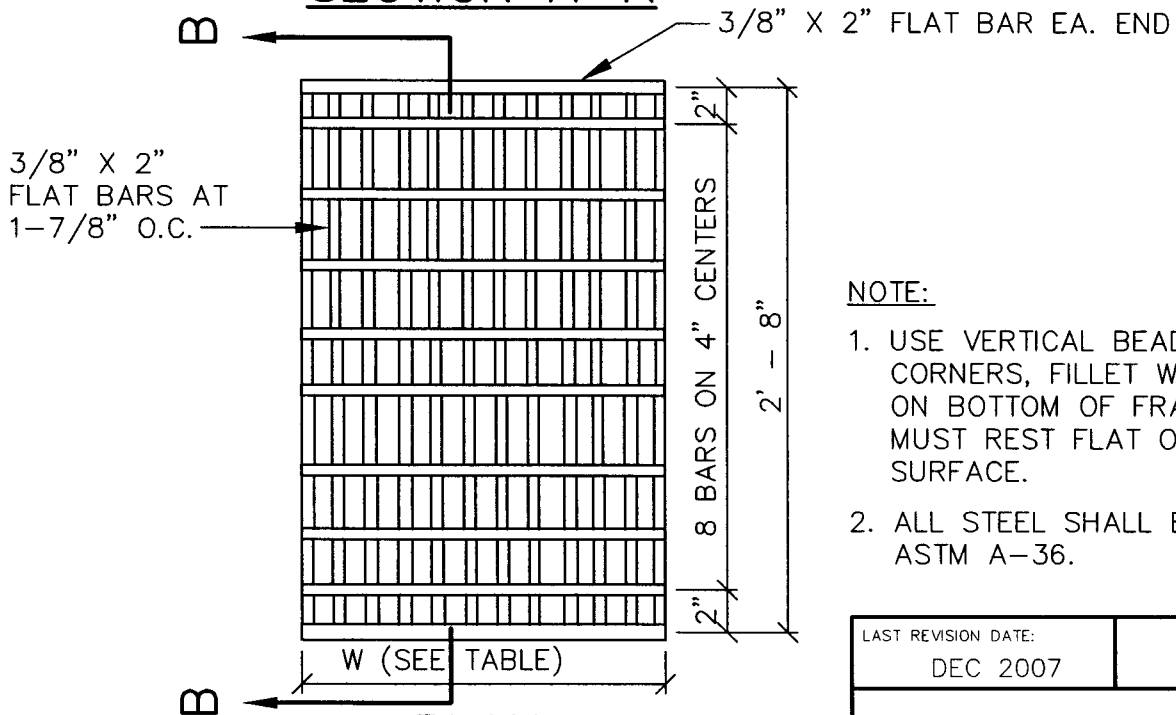
3/8" ROUND OR RECTANGULAR CROSS BARS SHALL BE FILLET WELDED, RESISTANCE WELDED OR ELECTROFORGED TO BEARING BARS.



SECTION B-B



SECTION A-A



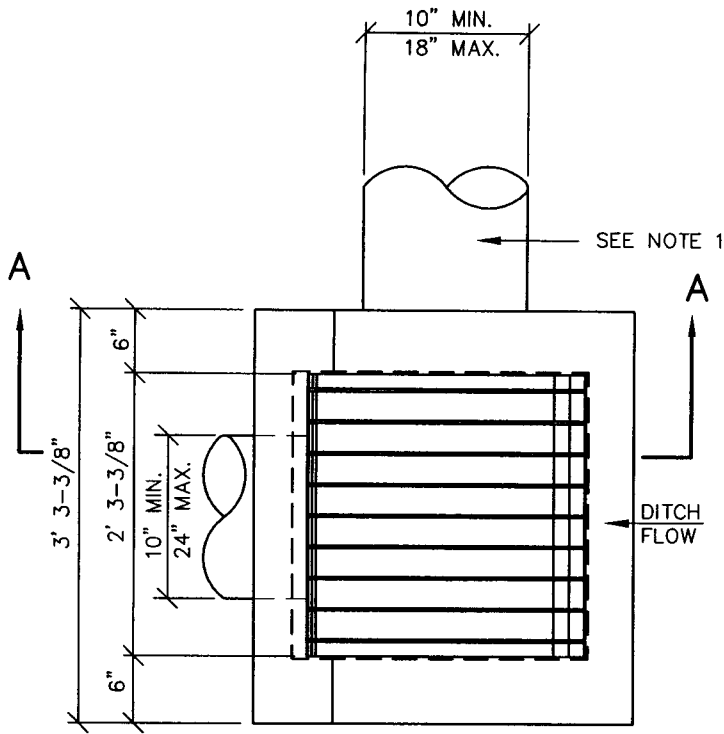
PLAN

NOTE:

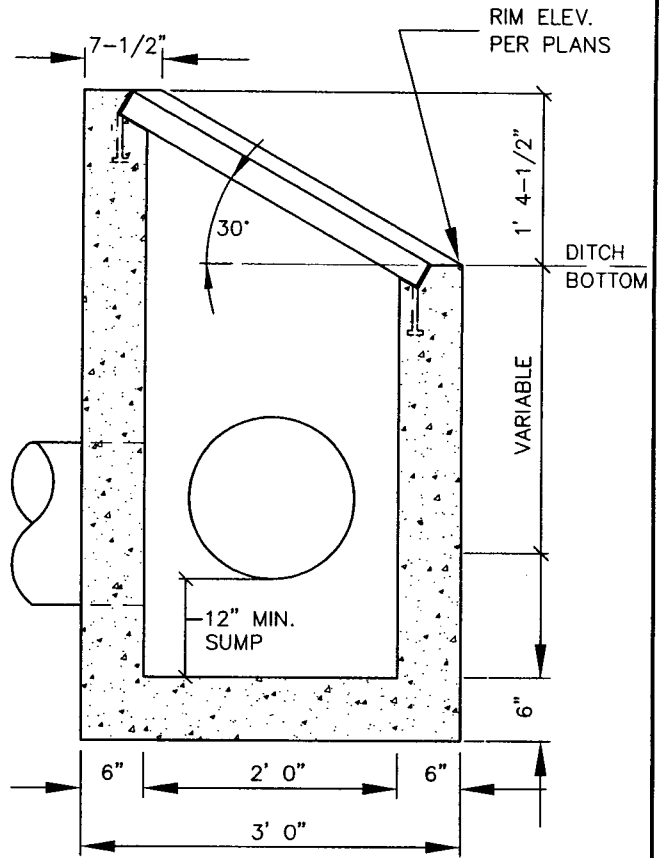
1. USE VERTICAL BEADS IN CORNERS, FILLET WELD JOINT ON BOTTOM OF FRAME. GRATE MUST REST FLAT ON FRAME SURFACE.
2. ALL STEEL SHALL BE ASTM A-36.

INLET TYPE	FRAME		GRATE		REMARKS
	V	Y	W	NO. OF BARS	
STANDARD	1' 10-3/4"	1' 9-3/8"	1'- 9"	12	1-GRATE
OVERSIZE	2' 4-3/4"	2' 3-3/8"	1' 1-1/2"	8	2-GRATES

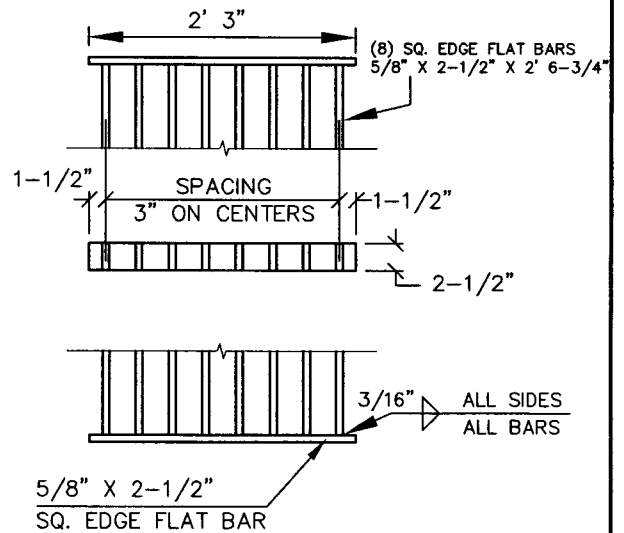
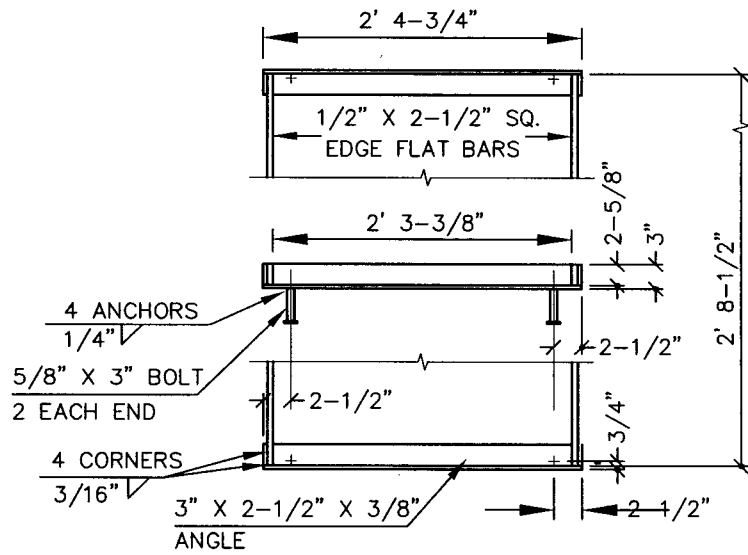
LAST REVISION DATE: DEC 2007	COPYRIGHT 1996 WESTECH ENGINEERING, INC.
CATCH BASIN GRATE DETAILS	
(NTS)	
CARLTON, OR	DETAIL NO. 312



PLAN



SECTION A - A



NOTES:

FRAME & GRATE

1. SEE CONSTRUCTION DRAWINGS FOR PIPE SIZE, LOCATION AND INVERT ELEVATION.
2. FRAME & GRATE SHALL BE ASTM A-36 STEEL, HOT-DIPPED GALV. AFTER CONSTRUCTION.
3. ALL CONCRETE TO BE 3000 PSI AT 28 DAYS.

LAST REVISION DATE:

DEC 2007

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WESTECH ENGINEERING, INC.

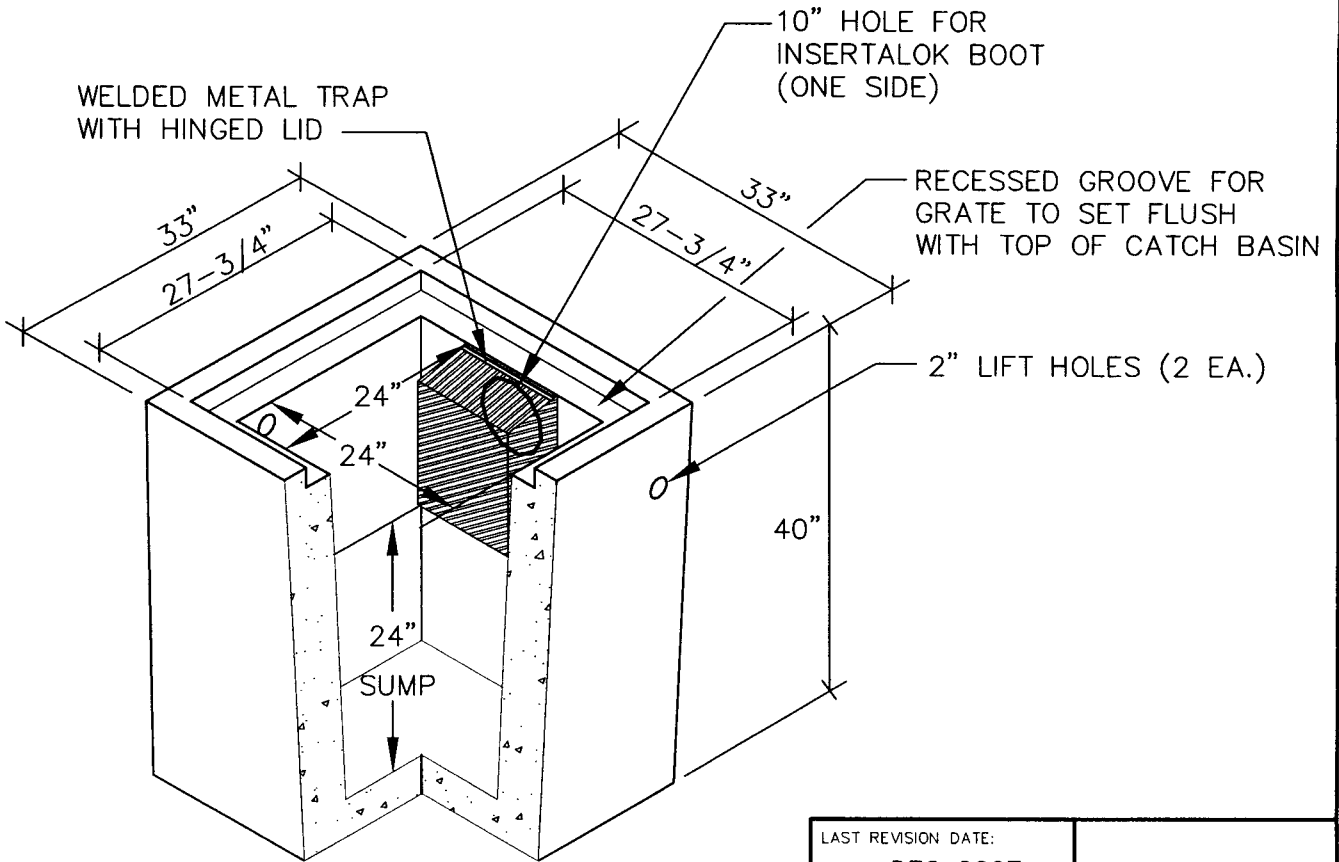
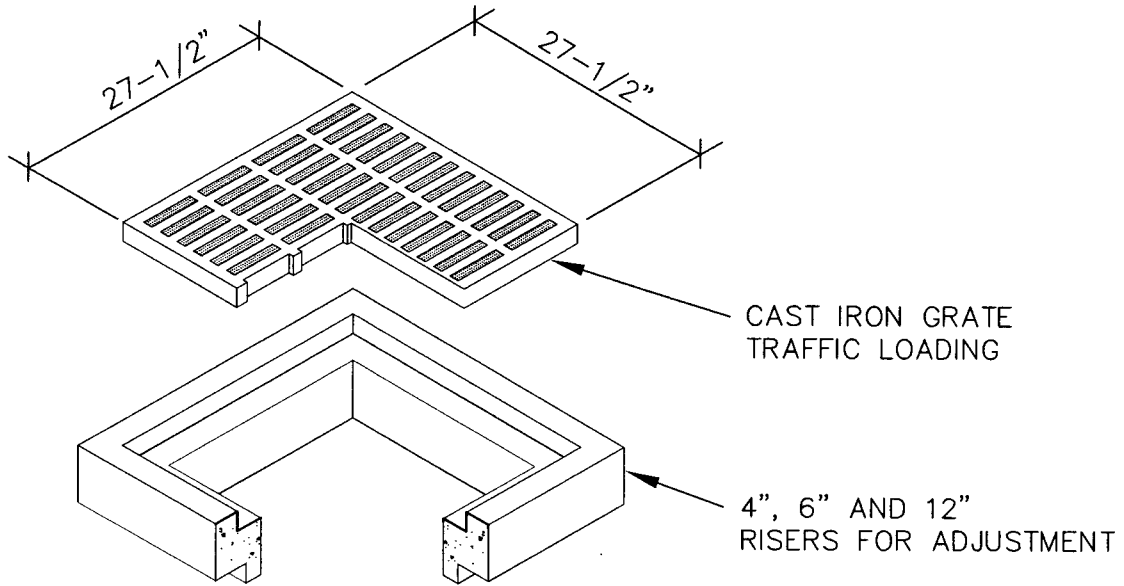
**TYPE 3 DITCH INLET
CATCH BASIN**

(NTS)

CARLTON, OR

DETAIL NO.

313

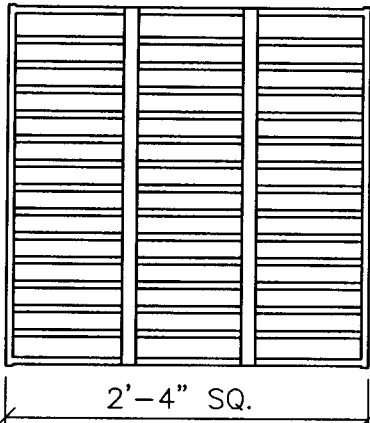


NOTES:

1. SEE CONSTRUCTION DRAWINGS FOR PIPE SIZE, LOCATION AND INVERT ELEVATION.
2. CONCRETE SHALL BE 3300 PSI @ 28 DAYS.
3. REBAR SHALL CONFORM TO ASTM A615 GRADE 60.
4. REBAR = #4 BARS @ 6" C.C.

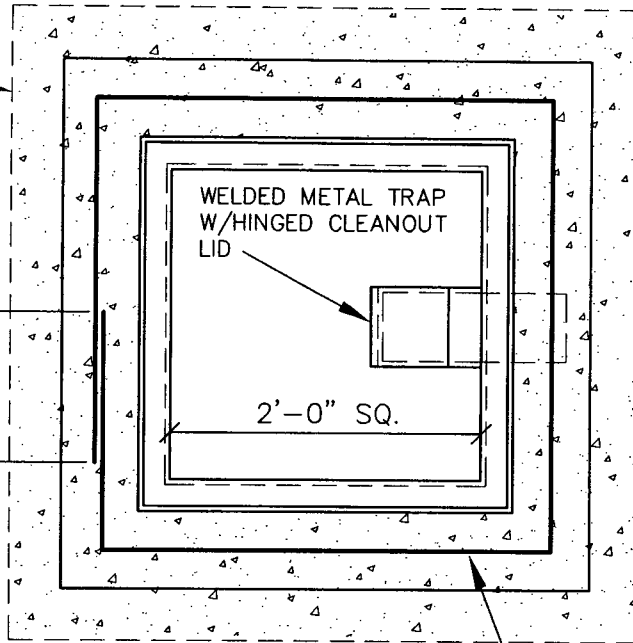
LAST REVISION DATE: DEC 2007	
PARKING LOT CATCH BASIN (PRECAST CONCRETE)	
(NTS)	
CARLTON, OR	DETAIL NO. 315

CAST-IN-PLACE
REINFORCED CONCRETE
SUPPORT COLLAR



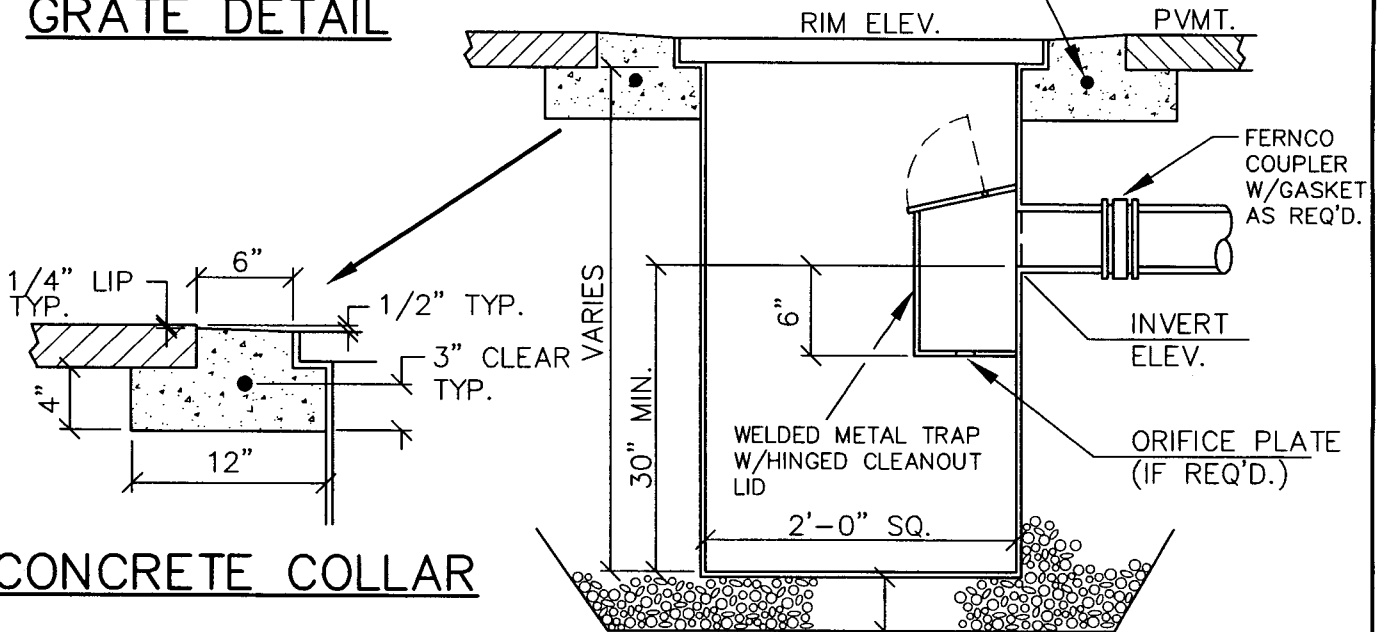
GRATE: WELDED STEEL DROP-IN
BAR GRATE (ASTM A36).
END BARS: 1/2" X 2"
CROSS BARS: 1/2" X 2" @ 2" O.C.
BIKE STRAPS: 1/8" X 1" (2 REQ'D)
16,000 LB. UNIFORM LOAD CAPACITY

GRATE DETAIL



PLAN VIEW

#4 REBAR
CONTINUOUS



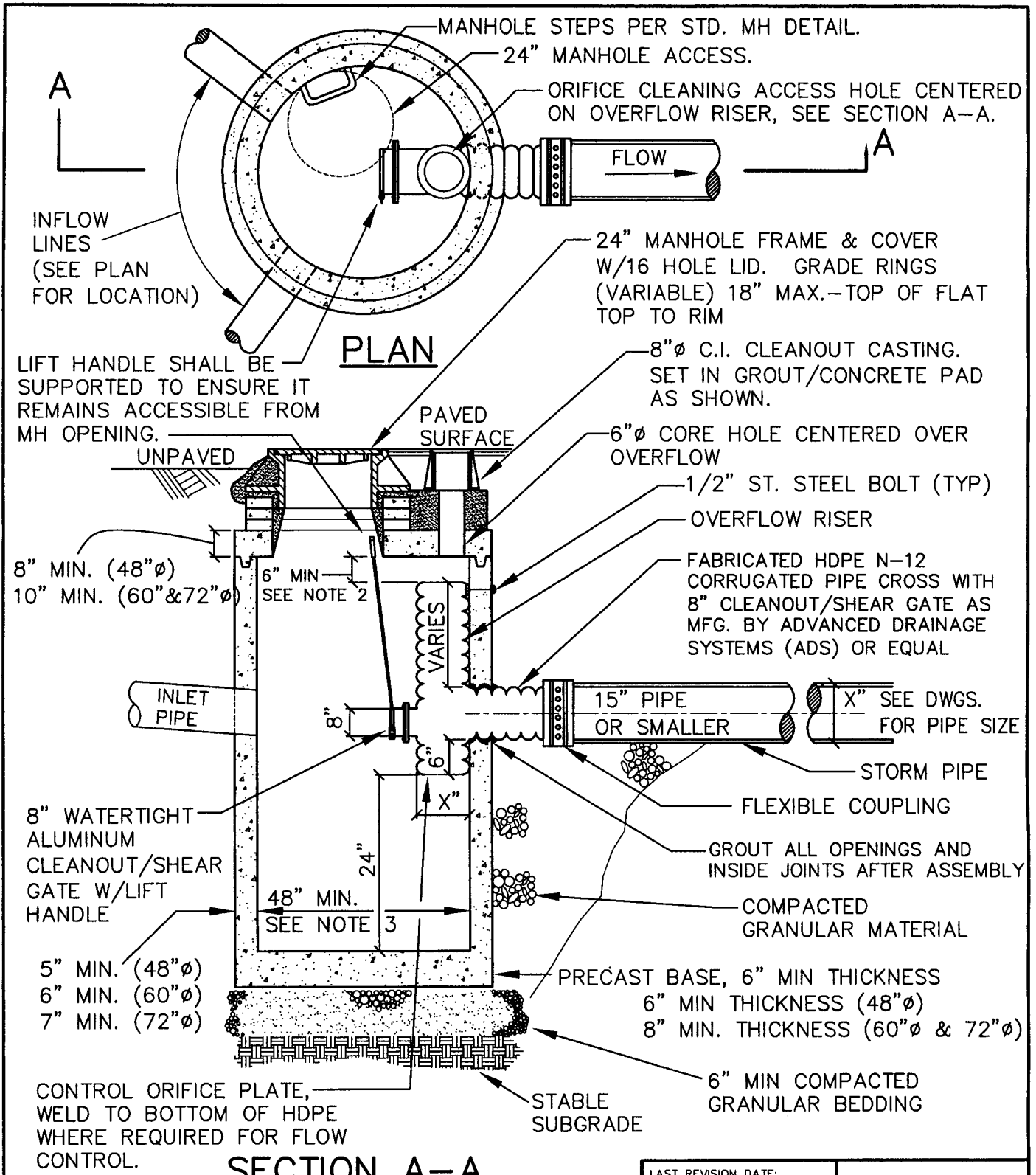
CONCRETE COLLAR

**CONSTRUCT BASIN OF WELDED
1/4" STEEL. COAT ALL SURFACES
WITH ASPHALTIC PAINT.**

NOTES:

1. SEE CONSTRUCTION DRAWINGS FOR PIPE SIZE, LOCATION AND INVERT ELEVATION.
2. OUTLET: SIZE AS REQ'D. FOR INDICATED PIPE SIZE.
3. FOR JUNCTION BOX, REPLACE GRATE WITH 3/4" STEEL PLATE. DRILL ONE, 1" LIFTING HOLE, CENTERED IN ONE END OF THE PLATE. WELD SHIMS TO RIM AS REQUIRED TO RAISE PLATE TO RIM ELEVATION.

LAST REVISION DATE: DEC 2007	
PARKING LOT CATCH BASIN (LYNCH STYLE) (NTS)	
CARLTON, OR	DETAIL NO. 316



LIFT HANDLE SHALL BE SUPPORTED TO ENSURE IT REMAINS ACCESSIBLE FROM MH OPENING.

PLAN

SECTION A-A

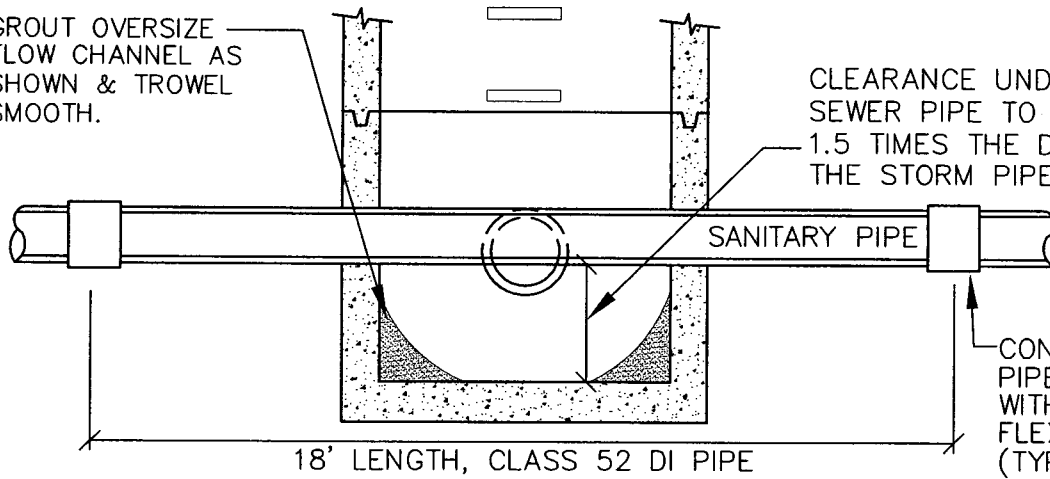
NOTES:

1. PRECAST SECTIONS SHALL CONFORM TO ASTM C-478.
2. DISTANCE FROM TOP OF OVERFLOW TO MH RIM SHALL BE BASED ON OVERFLOW CAPACITY CALC'S BY DESIGN ENGINEER (ASSUME ORIFICE CONTROL).
3. 60" MINIMUM DIA. MANHOLE REQUIRED FOR OUTLET PIPE LARGER THAN 15" OR INLET > 21".
4. ORIFICE CLEANING ACCESS TO BE 6" CORE HOLE THROUGH FLAT-TOP (CENTERED ON OVERFLOW) WITH CI CLEANOUT BOX GROUTED TO SLAB.

LAST REVISION DATE:	
MAR 2010	
POLLUTION/FLOW CONTROL MANHOLE W/OVERFLOW	
(NTS)	
CARLTON, OR	DETAIL NO. 320

GROUT OVERSIZE
FLOW CHANNEL AS
SHOWN & TROWEL
SMOOTH.

CLEARANCE UNDER SANITARY
SEWER PIPE TO BE A MINIMUM OF
1.5 TIMES THE DIAMETER OF
THE STORM PIPE



CONNECT DUCTILE IRON
PIPE TO SEWER PIPE
WITH APPROVED
FLEXIBLE COUPLING.
(TYP BOTH ENDS)

SECTION THRU SANITARY SEWER

MANHOLE FRAME
AND COVER

SET FRAME IN NON-SHRINK GROUT

PVMT.

UNPAVED

GRADE RINGS (VARIABLE)
18" MAX.—TOP OF FLAT TOP
TO RIM

30" MAX
TO STEP

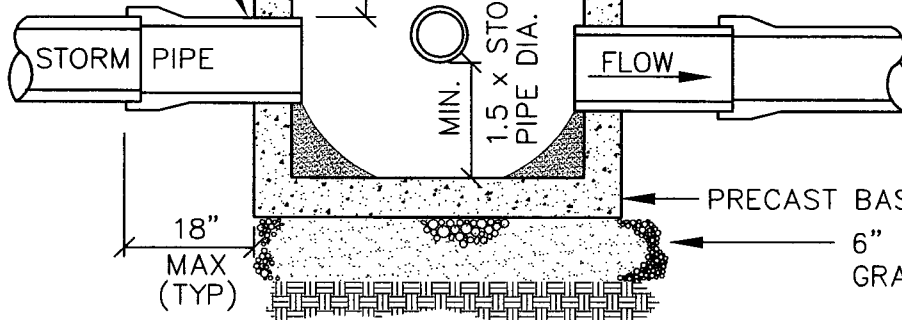
FLAT TOP SECTION, 8" MIN THICKNESS
5" MIN. THICK

12" TYP

INSIDE DIAMETER
SEE NOTES

ALL OPENINGS
CORED DRILLED.

SEE DRAWINGS FOR INVERT ELEVATIONS
AND PIPE ALIGNMENTS.



SECTION THRU STORM

STABLE
SUBGRADE

NOTES:

1. UNLESS OTHERWISE SHOWN ON DRAWINGS, USE 48" MANHOLE FOR SANITARY SEWER UP TO 12" DIA. & STORM DRAIN UP TO 18" DIAMETER.
2. PRECAST SECTIONS SHALL MEET OR EXCEED ASTM C-478. WATERTIGHT O-RING OR MASTIC KEYLOCK JOINTS REQUIRED.
3. STEPS TO BE FACTORY INSTALLED POLYPROPYLENE PLASTIC WITH GRADE 60 REINFORCING ROD.

LAST REVISION DATE:

DEC 2007

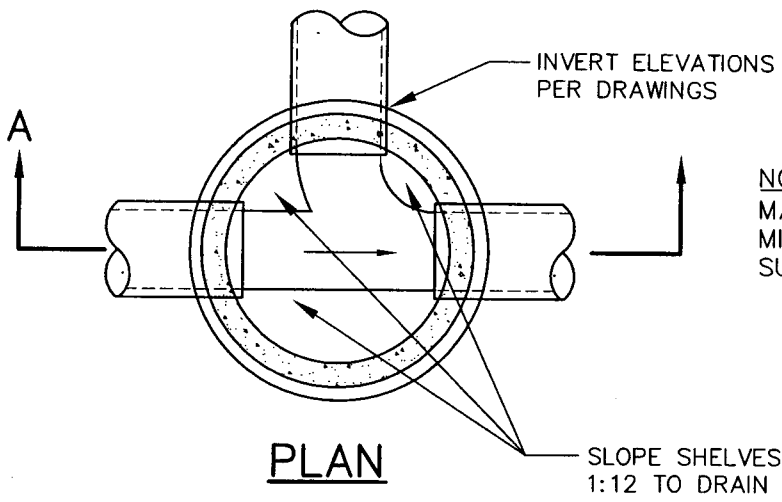
KUENZI MANHOLE

(NTS)

CARLTON, OR

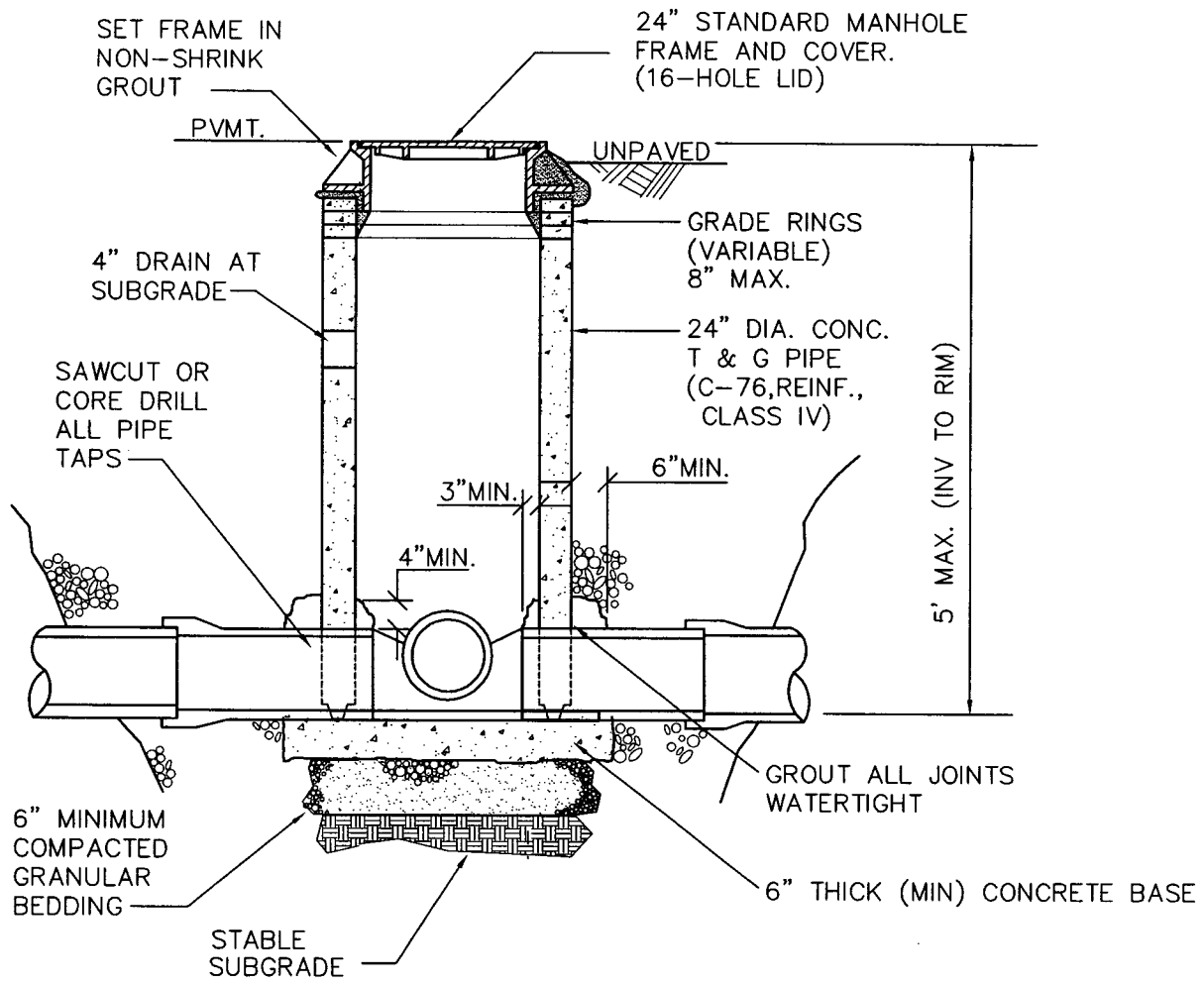
DETAIL NO.

330



NOTE: PER ORS 92.044(7),
MANHOLE MUST BE SET 1'
MINIMUM CLEAR FROM ANY
SURVEY MONUMENT

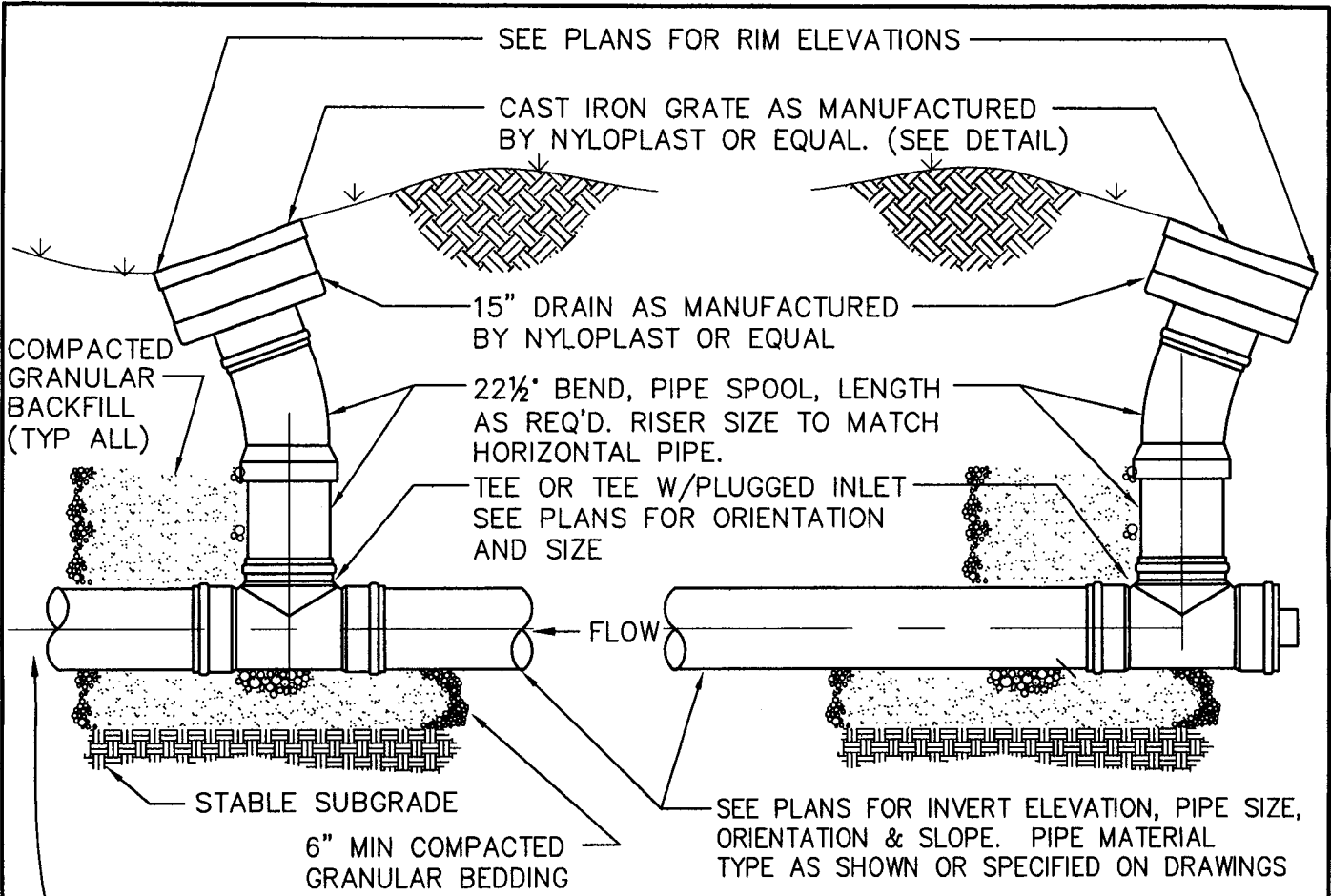
PLAN



SECTION A-A

NOTE:
1. MAXIMUM PIPE NUMBER & DIAMETERS AS FOLLOWS:
12" DIAMETER OR LESS - 4 MAXIMUM.
15" DIAMETER - 2 MAXIMUM.
ALL OTHER CONFIGURATIONS REQUIRE STANDARD MANHOLE.

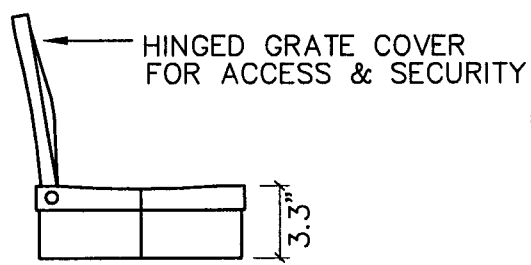
LAST REVISION DATE:	
MAR 2008	
24" DIA. STORM MANHOLE	
(NTS)	
CARLTON, OR	DETAIL NO. 350



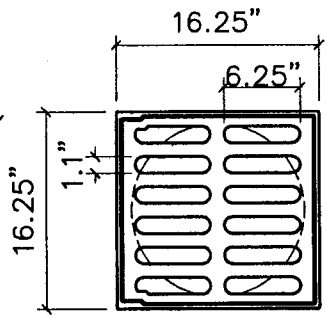
AREA DRAIN
NTS

CONNECT TO MAINLINE OR MANHOLE AS SHOWN ON DRAWINGS

SEE PLANS FOR INVERT ELEVATION, PIPE SIZE, ORIENTATION & SLOPE. PIPE MATERIAL TYPE AS SHOWN OR SPECIFIED ON DRAWINGS



HINGED GRATE COVER FOR ACCESS & SECURITY



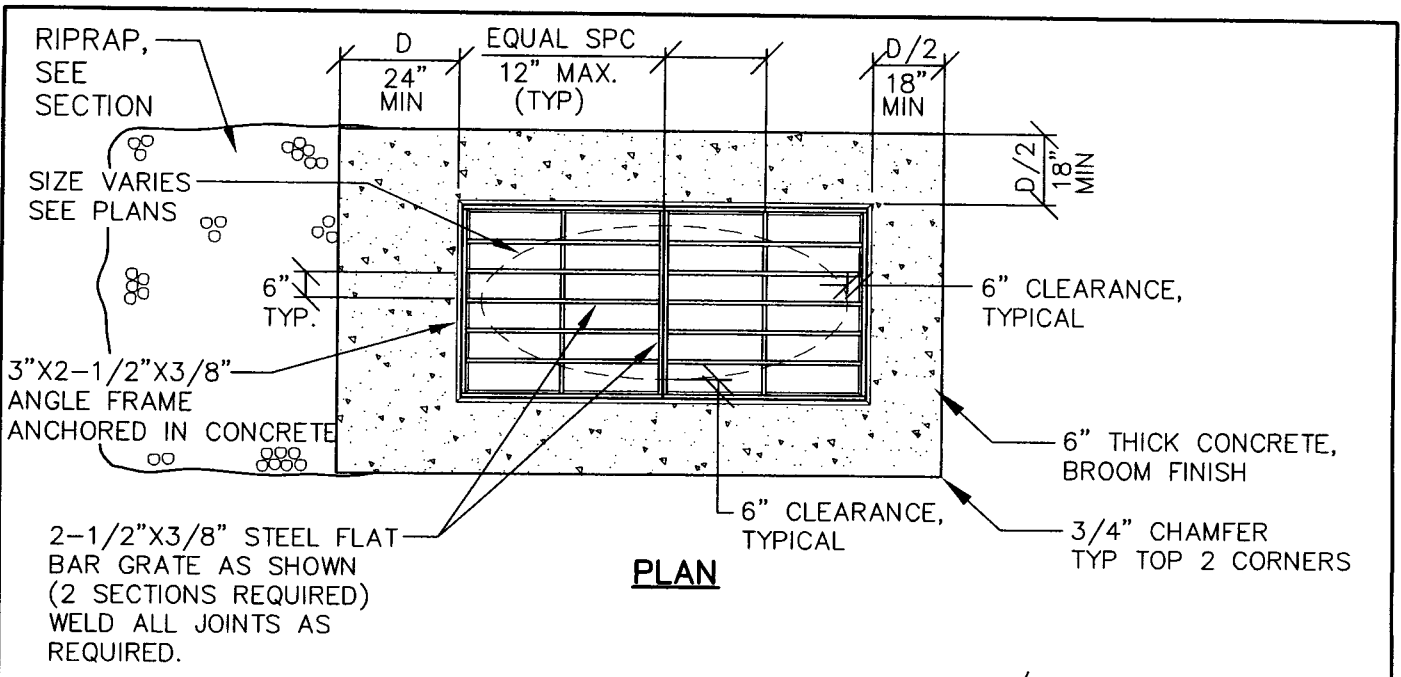
STANDARD

15" CAST IRON GRATE DETAIL
NTS

NOTES:

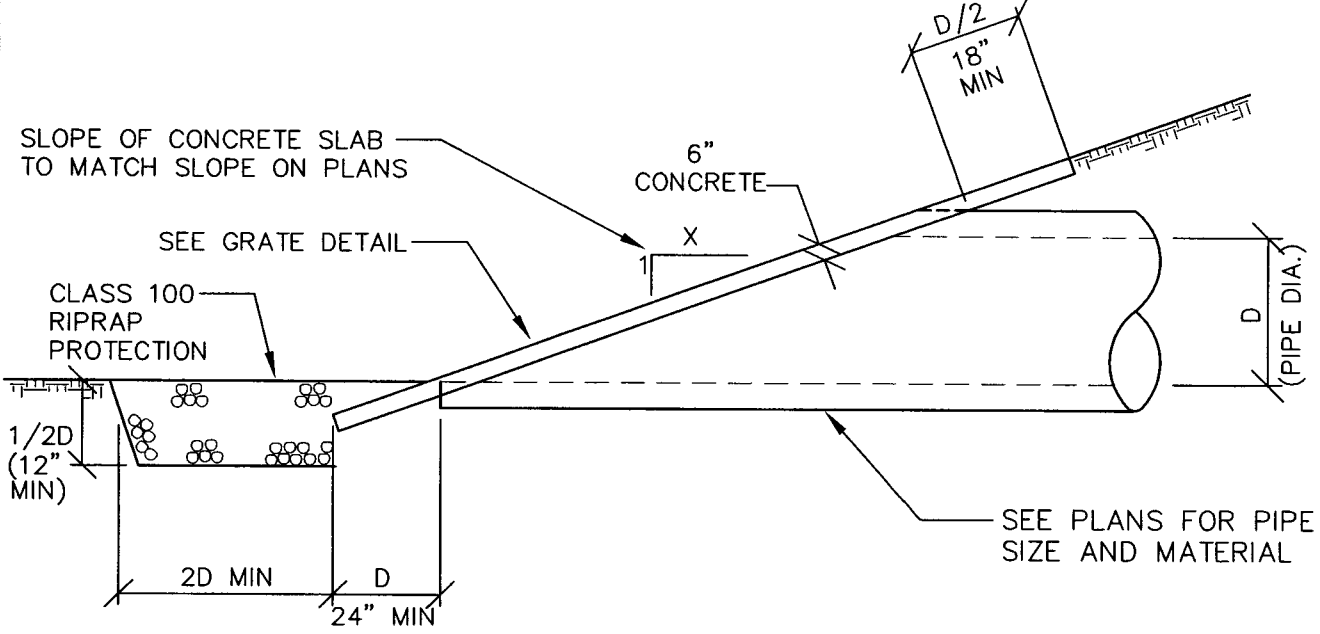
1. AREA DRAIN NOT FOR USE IN AREAS SUBJECT TO VEHICLE TRAFFIC.
2. USE WATERTIGHT GASKETED FITTINGS AND ADAPTORS FOR ALL PIPE CONNECTIONS.
3. ALL GRATES IN PEDESTRIAN AREAS SHALL CONFORM WITH ADA REQUIREMENTS, INCLUDING GRATE OPENING SIZE.

LAST REVISION DATE: JAN 2010	JO # STANDARD
PRIVATE AREA DRAIN, NON-TRAFFIC AREAS	
(NTS)	
CARLTON, OR	DETAIL NO. 355



PLAN

2-1/2"x3/8" STEEL FLAT BAR GRATE AS SHOWN (2 SECTIONS REQUIRED) WELD ALL JOINTS AS REQUIRED.



(NORMAL TO CL OF EMBANKMENT)

SECTION

NOTES:

1. SEE CONSTRUCTION DRAWINGS FOR PIPE SIZE, LOCATION AND INVERT ELEVATION.
2. FRAME AND GRATE SHALL BE ASTM A-36 STEEL, HOT DIP GALVANIZED AFTER CONSTRUCTION.
3. ALL CONCRETE TO BE 3300 PSI AT 28 DAYS.

LAST REVISION DATE: DEC 2007	
CONCRETE PIPE END CAP WITH GRATE (NTS)	
CARLTON, OR	DETAIL NO. 362

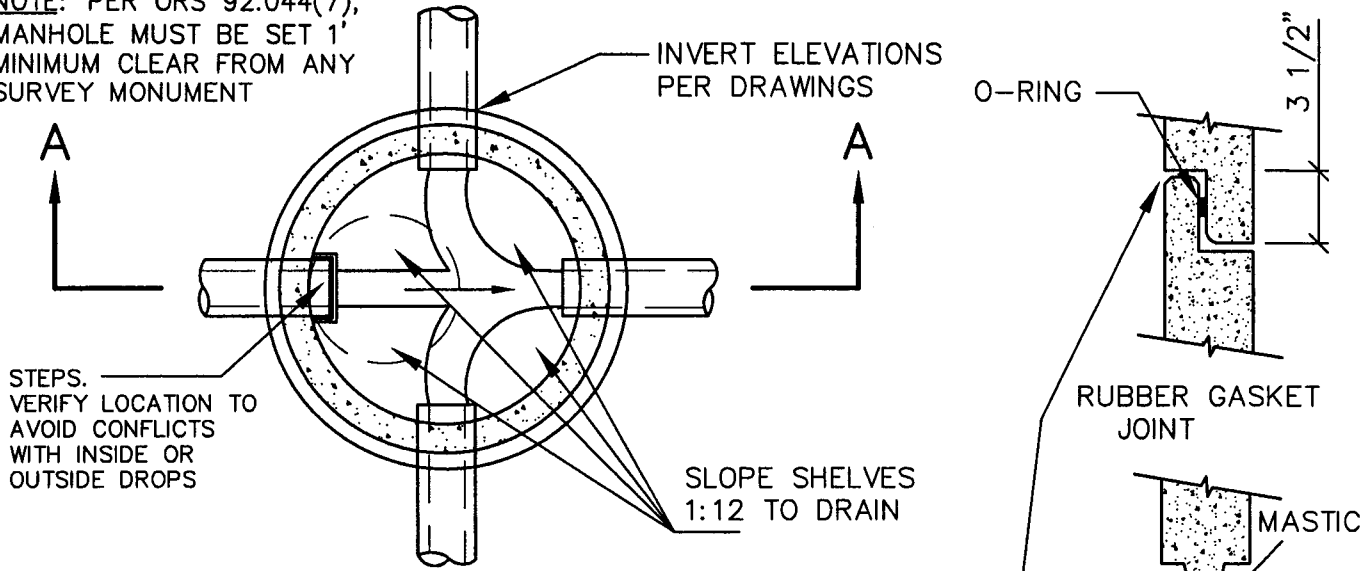
STORM SEWER MANDREL TEST REPORT

Project Location: (City)	Project Name:
Inspector: (Print)	Date: (Separate Report Required for Each Test Session)
Mandrel Diameters Verified? Yes / No	

Station (& Manhole #)		Size & Material	Length (ft)	Results	Backfill Compaction Completed?	Date Sewer Flushed & Cleaned	Comments
From	To						
				Pass / Fail	Yes / No		
				Pass / Fail	Yes / No		
				Pass / Fail	Yes / No		
				Pass / Fail	Yes / No		
				Pass / Fail	Yes / No		
				Pass / Fail	Yes / No		
				Pass / Fail	Yes / No		
				Pass / Fail	Yes / No		
				Pass / Fail	Yes / No		
				Pass / Fail	Yes / No		
				Pass / Fail	Yes / No		
				Pass / Fail	Yes / No		
				Pass / Fail	Yes / No		
				Pass / Fail	Yes / No		
				Pass / Fail	Yes / No		

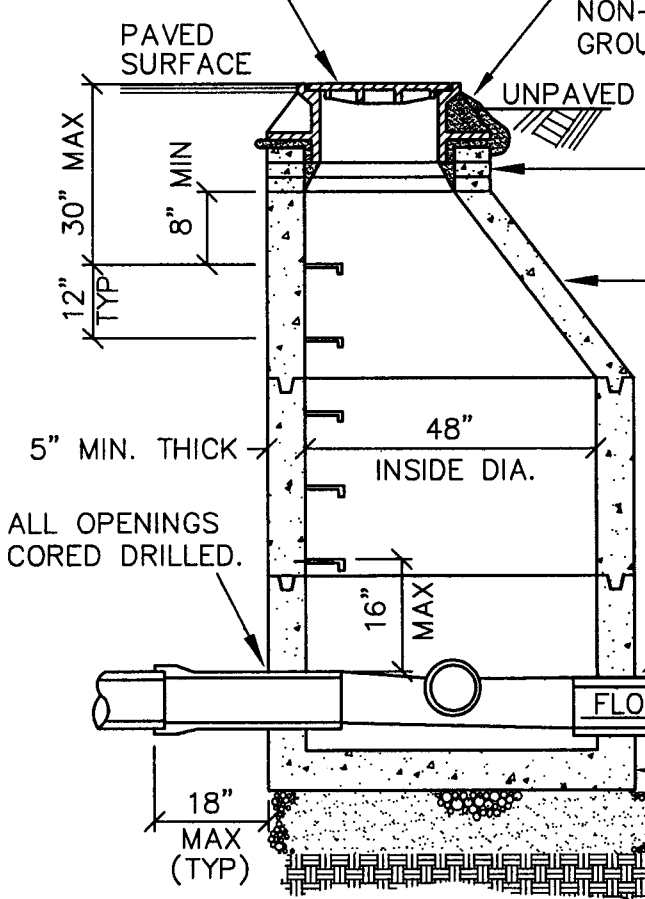
1. Mandrel testing shall be conducted on a manhole to manhole (or cleanout) basis and shall be done after the line has been completely flushed out with water.
2. Mandrel testing shall be conducted after trench backfill and compaction has been completed.
3. The mandrel diameter shall be 95% of the pipe initial inside diameter. The inspector shall verify the diameter of each mandrel used during each test session.

NOTE: PER ORS 92.044(7),
MANHOLE MUST BE SET 1'
MINIMUM CLEAR FROM ANY
SURVEY MONUMENT

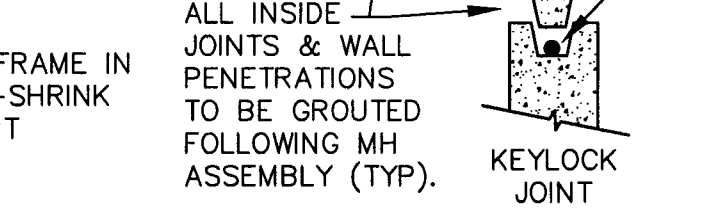


PLAN

MANHOLE FRAME AND COVER
PAVED SURFACE
UNPAVED



SECTION A-A



ALL INSIDE
JOINTS & WALL
PENETRATIONS
TO BE GROUTED
FOLLOWING MH
ASSEMBLY (TYP).

GRADE RINGS (VARIABLE)
18" MAX.-TOP OF CONE
TO RIM

SLOPE OF PRECAST ECCENTRIC CONE
SHALL FACE DOWN GRADE. LOCATE
STEPS ON UPSTREAM SIDE OF MANHOLE.

**FLAT TOP MH'S SHALL BE USED FOR ALL
MANHOLES LESS THAN 6' RIM TO INVERT**

ALL PIPE PENETRATIONS ON SANITARY
SEWER MANHOLES TO HAVE NEOPRENE
RUBBER BOOTS AS SPECIFIED.

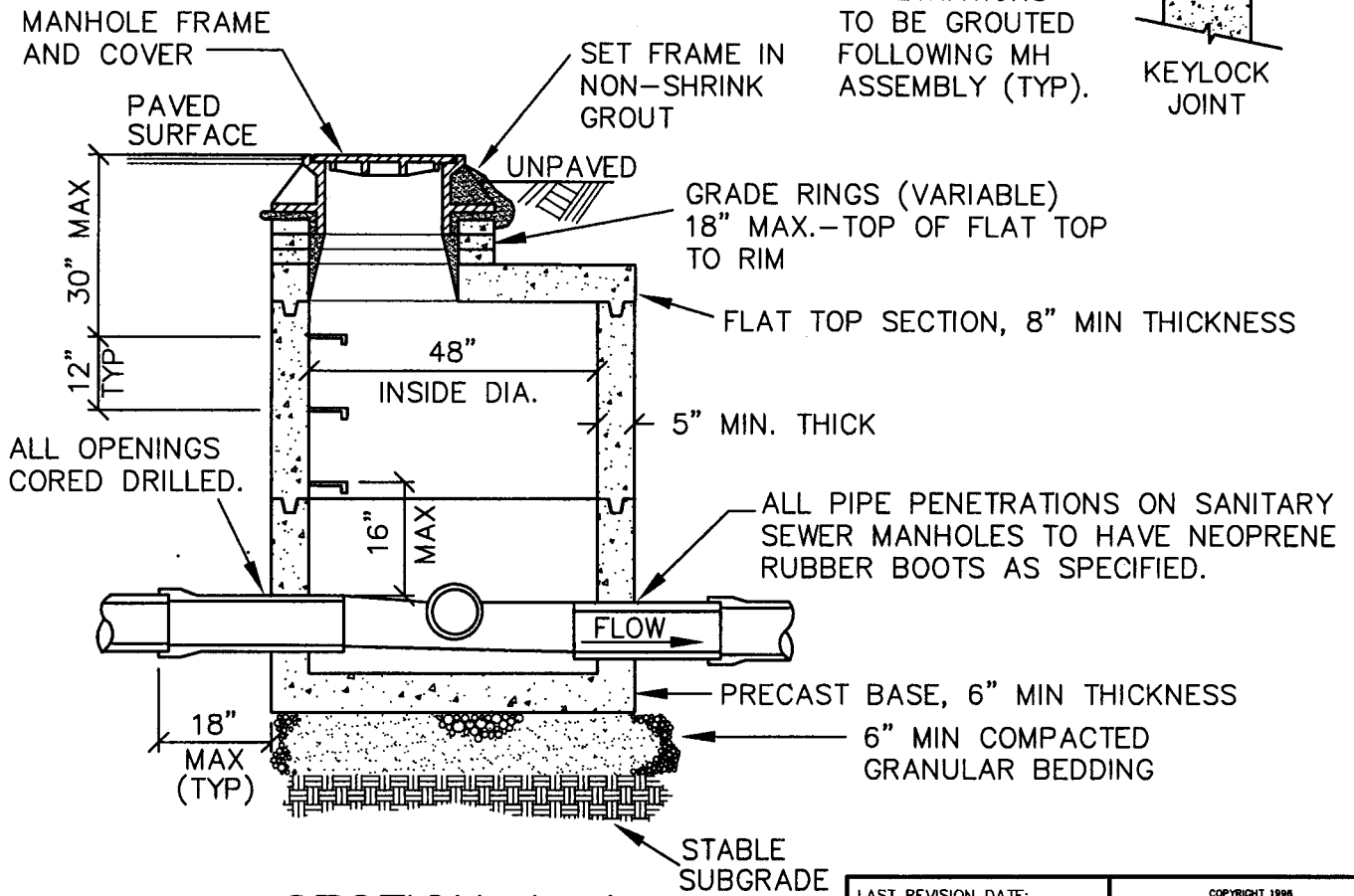
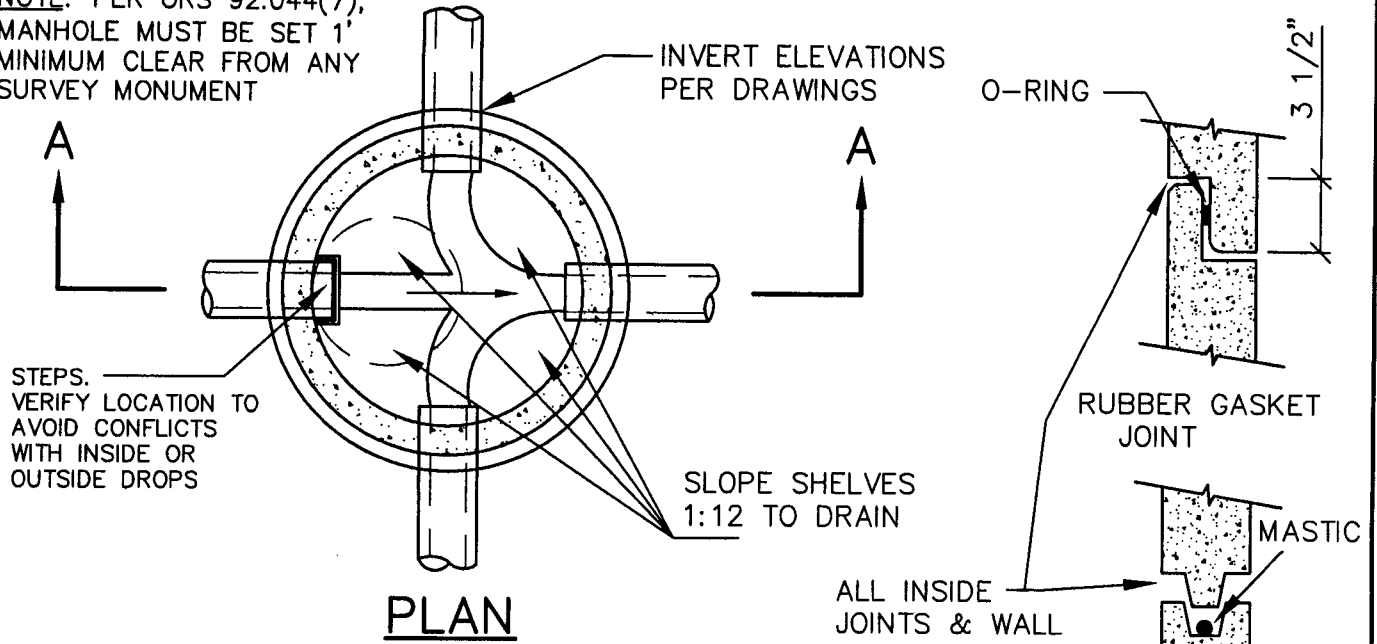
PRECAST BASE, 6" MIN THICKNESS
6" MIN COMPACTED
GRANULAR BEDDING

NOTES:

1. PRECAST SECTIONS SHALL MEET OR EXCEED
ASTM C-478.
2. WATERTIGHT O-RING OR MASTIC KEYLOCK
JOINTS REQUIRED.
3. STEPS TO BE FACTORY INSTALLED POLYPROPYLENE
PLASTIC WITH GRADE 60 REINFORCING ROD.

LAST REVISION DATE: JAN 2010	COPYRIGHT 1996 WESTECH ENGINEERING, INC.
STANDARD MANHOLE FOR 21" PIPE AND SMALLER	
(NTS)	
CARLTON, OR	DETAIL NO. 401

NOTE: PER ORS 92.044(7),
MANHOLE MUST BE SET 1'
MINIMUM CLEAR FROM ANY
SURVEY MONUMENT



NOTES:

1. PRECAST SECTIONS SHALL MEET OR EXCEED ASTM C-478.
2. WATERTIGHT O-RING OR MASTIC KEYLOCK JOINTS REQUIRED.
3. STEPS TO BE FACTORY INSTALLED POLYPROPYLENE PLASTIC WITH GRADE 60 REINFORCING ROD.

LAST REVISION DATE:
JAN 2010

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WESTECH ENGINEERING, INC.

**FLAT TOP MANHOLE
FOR 21" PIPE AND SMALLER**

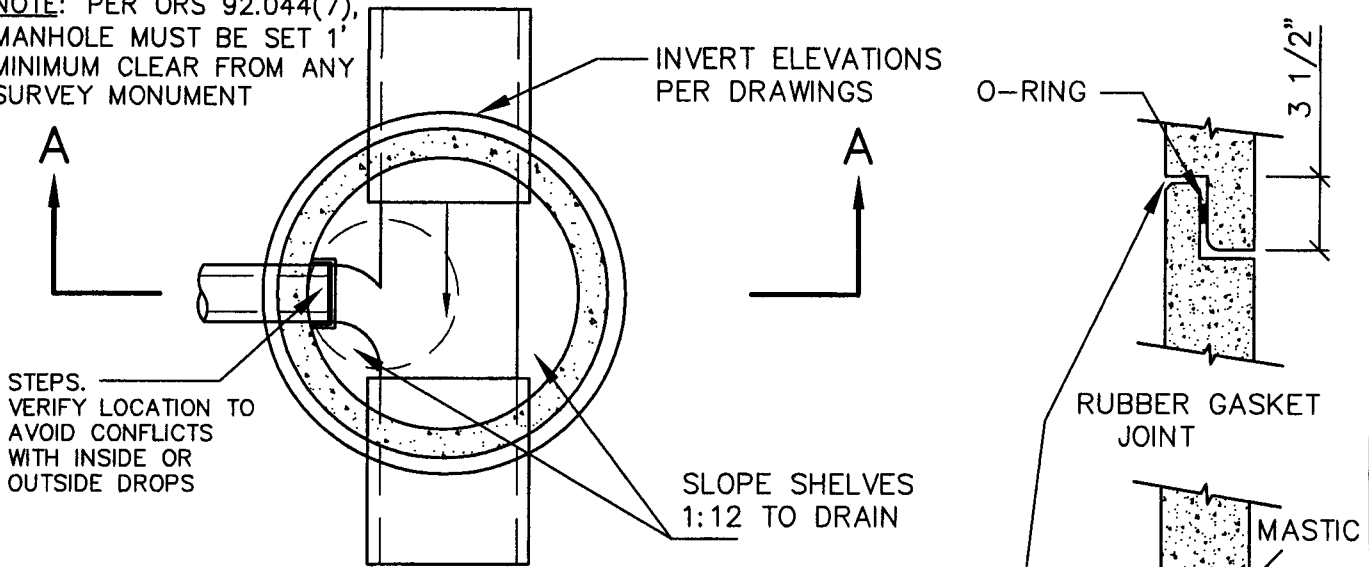
(NTS)

CARLTON, OR

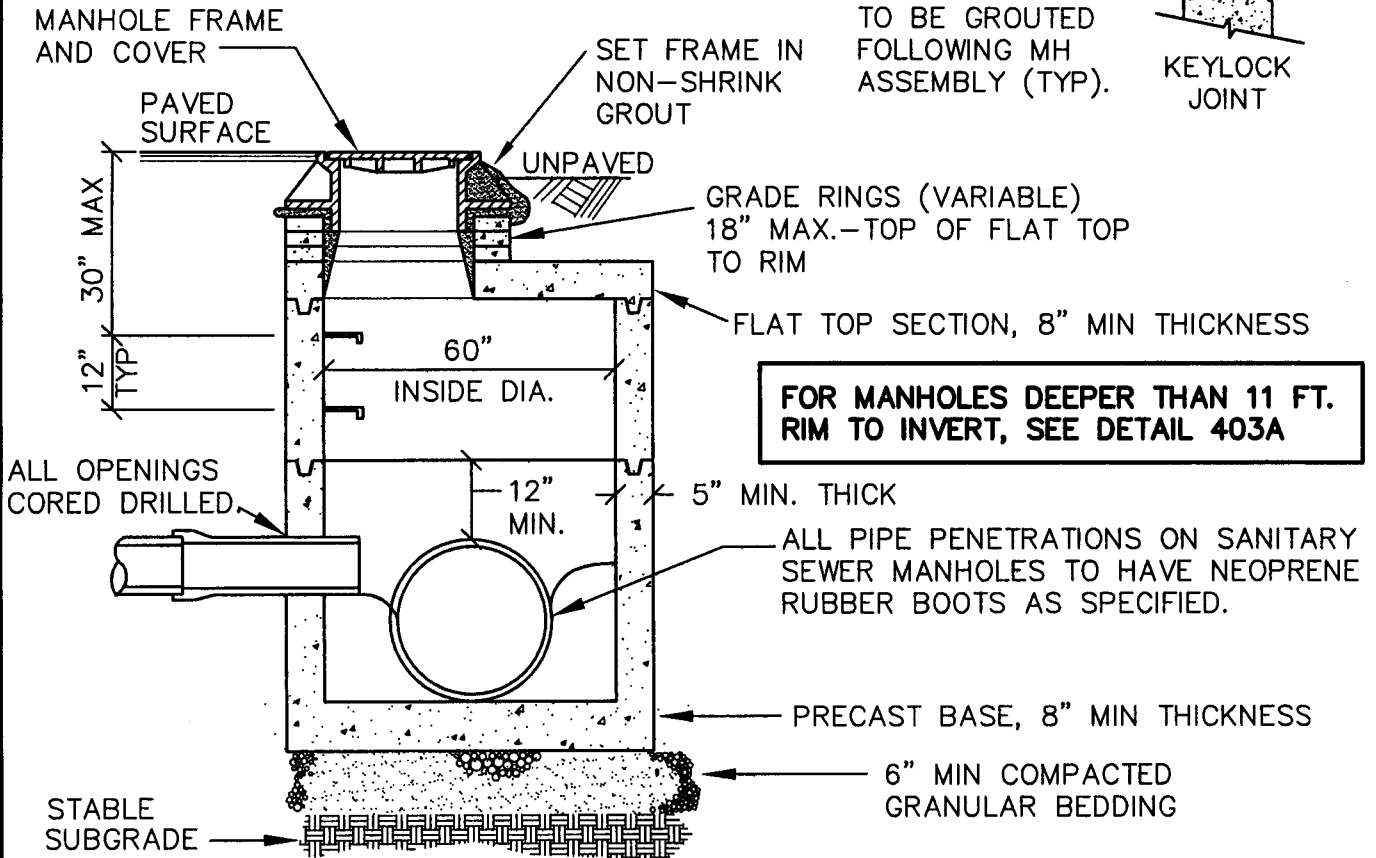
DETAIL NO.

402

NOTE: PER ORS 92.044(7),
MANHOLE MUST BE SET 1'
MINIMUM CLEAR FROM ANY
SURVEY MONUMENT



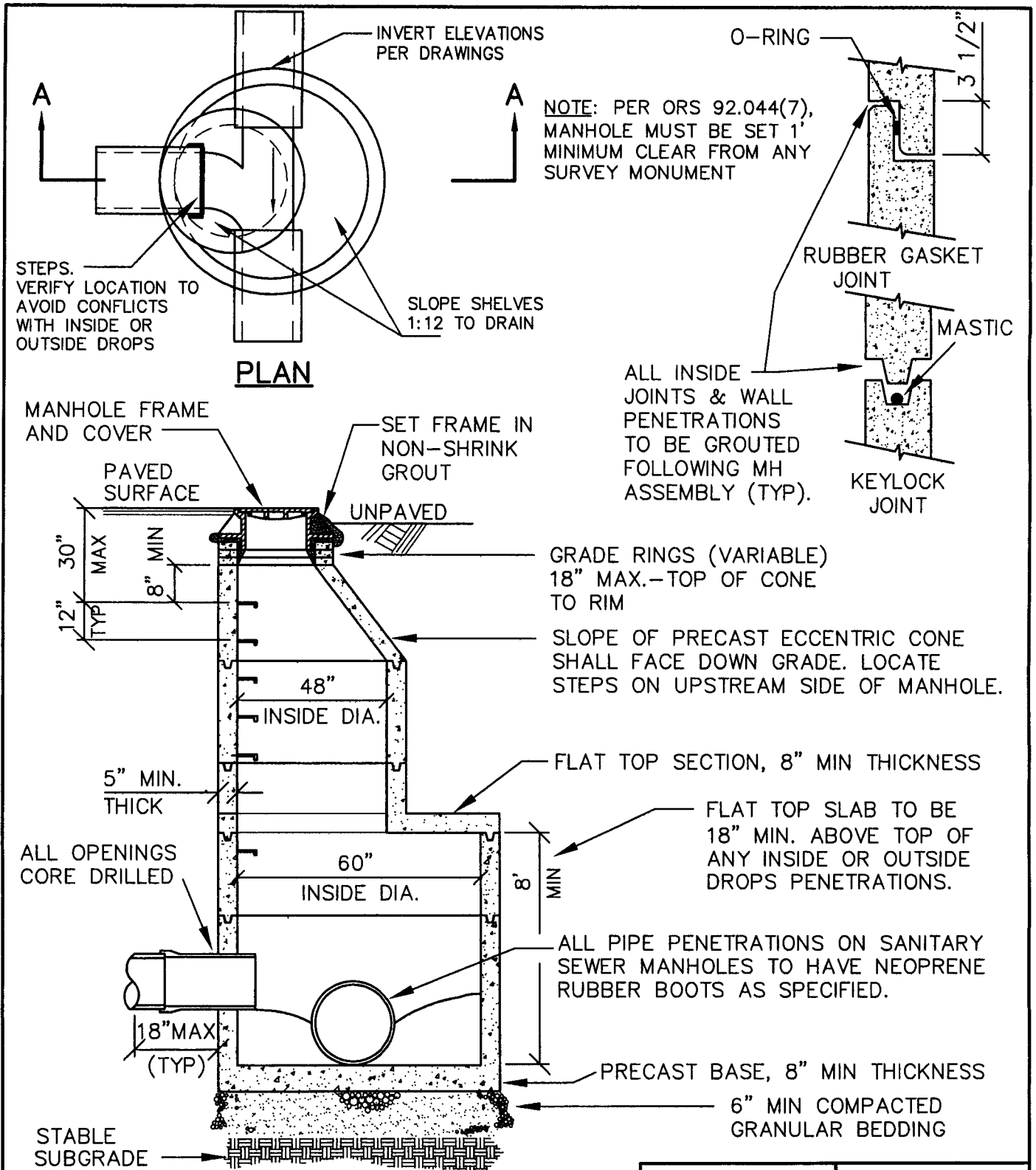
PLAN



SECTION A-A

- NOTES:**
1. PRECAST SECTIONS SHALL MEET OR EXCEED ASTM C-478.
 2. WATERTIGHT O-RING OR MASTIC KEYLOCK JOINTS REQUIRED.
 3. STEPS TO BE FACTORY INSTALLED POLYPROPYLENE PLASTIC WITH GRADE 60 REINFORCING ROD.

LAST REVISION DATE: JAN 2010	COPYRIGHT 1996 WESTECH ENGINEERING, INC.
MANHOLE FOR 24" AND 27" PIPE	
(NTS)	
CARLTON, OR	DETAIL NO. 403



NOTE: PER ORS 92.044(7),
MANHOLE MUST BE SET 1'
MINIMUM CLEAR FROM ANY
SURVEY MONUMENT

STEPS.
VERIFY LOCATION TO
AVOID CONFLICTS
WITH INSIDE OR
OUTSIDE DROPS

PLAN

MANHOLE FRAME
AND COVER

PAVED
SURFACE

SET FRAME IN
NON-SHRINK
GROUT

UNPAVED

30"
MAX
8"
MIN
12"
TYP

GRADE RINGS (VARIABLE)
18" MAX.-TOP OF CONE
TO RIM

SLOPE OF PRECAST ECCENTRIC CONE
SHALL FACE DOWN GRADE. LOCATE
STEPS ON UPSTREAM SIDE OF MANHOLE.

48"
INSIDE DIA.

FLAT TOP SECTION, 8" MIN THICKNESS

5" MIN.
THICK

FLAT TOP SLAB TO BE
18" MIN. ABOVE TOP OF
ANY INSIDE OR OUTSIDE
DROPS PENETRATIONS.

ALL OPENINGS
CORE DRILLED

60"
INSIDE DIA.

ALL PIPE PENETRATIONS ON SANITARY
SEWER MANHOLES TO HAVE NEOPRENE
RUBBER BOOTS AS SPECIFIED.

18" MAX
(TYP)

PRECAST BASE, 8" MIN THICKNESS

6" MIN COMPACTED
GRANULAR BEDDING

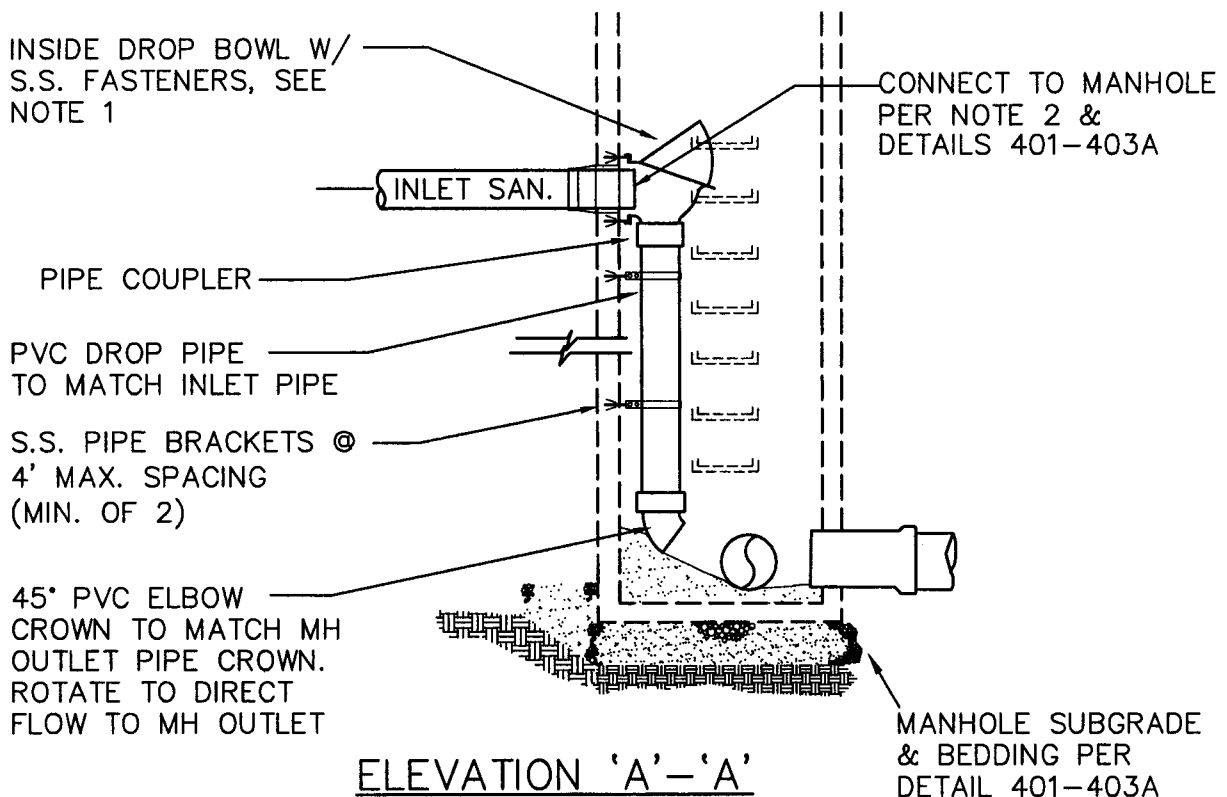
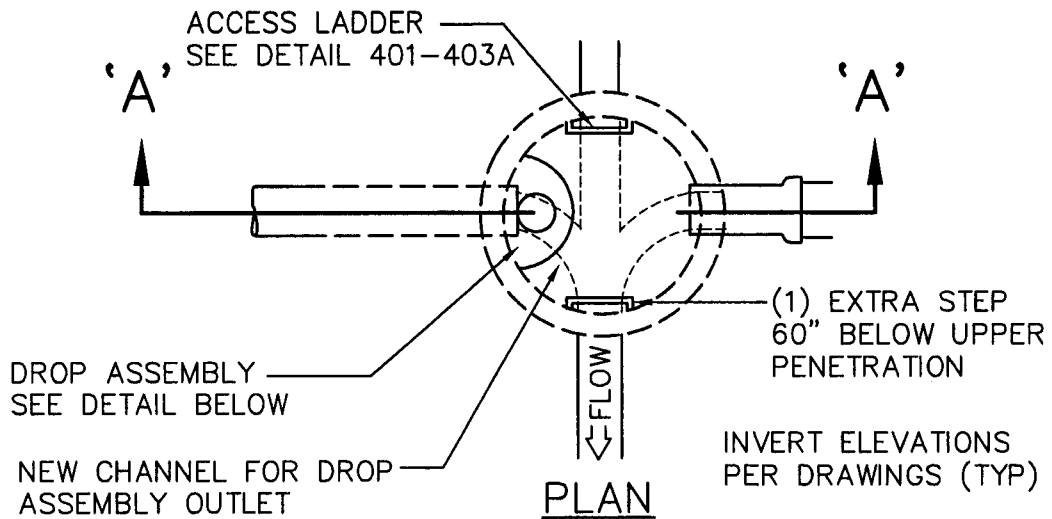
STABLE
SUBGRADE

SECTION A-A

NOTES:

1. PRECAST SECTIONS SHALL MEET OR EXCEED ASTM C-478.
2. WATERTIGHT O-RING OR MASTIC KEYLOCK JOINTS REQUIRED.
3. STEPS TO BE FACTORY INSTALLED POLYPROPYLENE PLASTIC WITH GRADE 60 REINFORCING ROD.

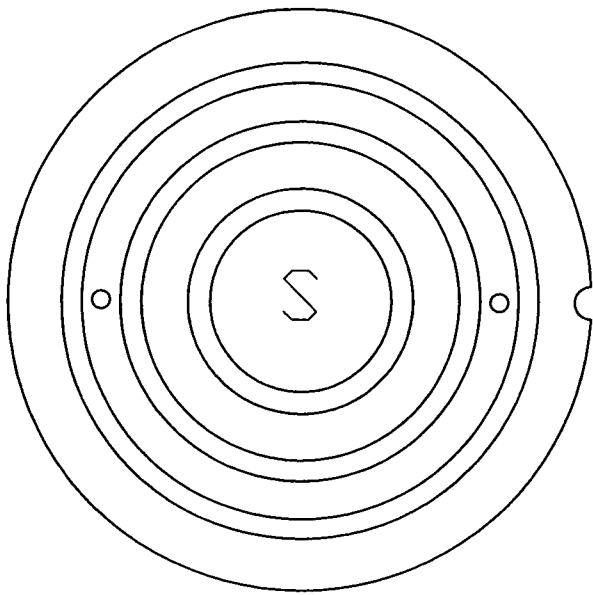
LAST REVISION DATE: JAN 2010	COPYRIGHT 1998 WESTECH ENGINEERING, INC.
DEEP MANHOLE FOR 24" AND 27" PIPE	
(NTS)	
CARLTON, OR	DETAIL NO. 403A



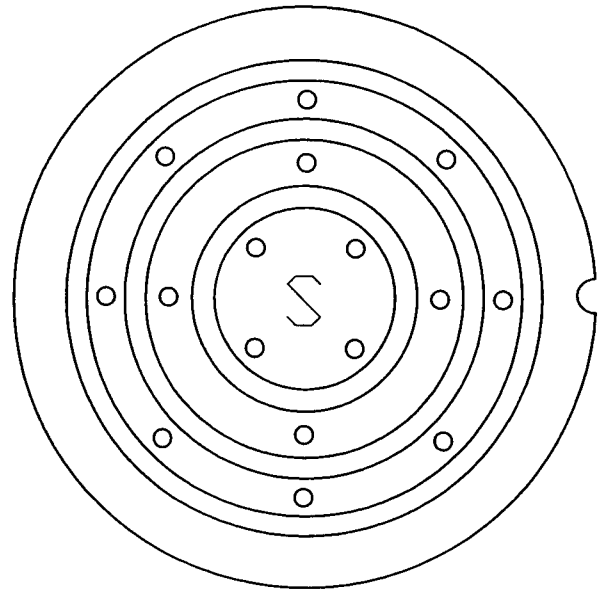
NOTES:

1. ALL INSIDE DROPS MUST BE APPROVED ON A CASE BY CASE BASIS BY THE PUBLIC WORKS SUPERINTENDANT. MINIMUM 60" DIAMETER MANHOLE REQUIRED FOR INSIDE DROPS UNLESS OTHERWISE APPROVED IN WRITING BY THE PUBLIC WORKS SUPERINTENDANT.
2. "RELINER" INSIDE DROP BOWL BY DURAN, INC. OR APPROVED EQUIVALENT. FOR INLET PIPES WITH SLOPES GREATER THAN 5%, PROVIDE BOWL WITH OPTIONAL HOOD.
3. ALL PIPE PENETRATIONS SHALL HAVE NEOPRENE RUBBER BOOTS. MANHOLE BASE, BARREL & TOP TO CONFORM WITH DETAILS 401-403A.

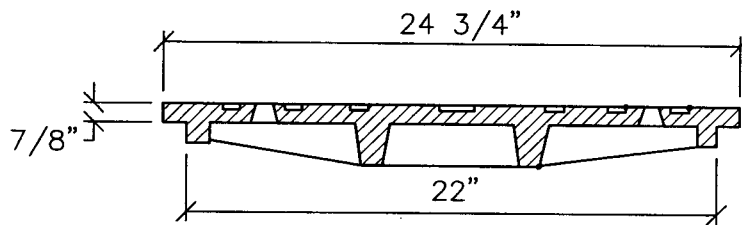
LAST REVISION DATE: JAN 2010	
INSIDE DROP CONNECTION FOR SANITARY SEWER MANHOLE	
(NTS)	
CARLTON, OR	DETAIL NO. 404



SANITARY



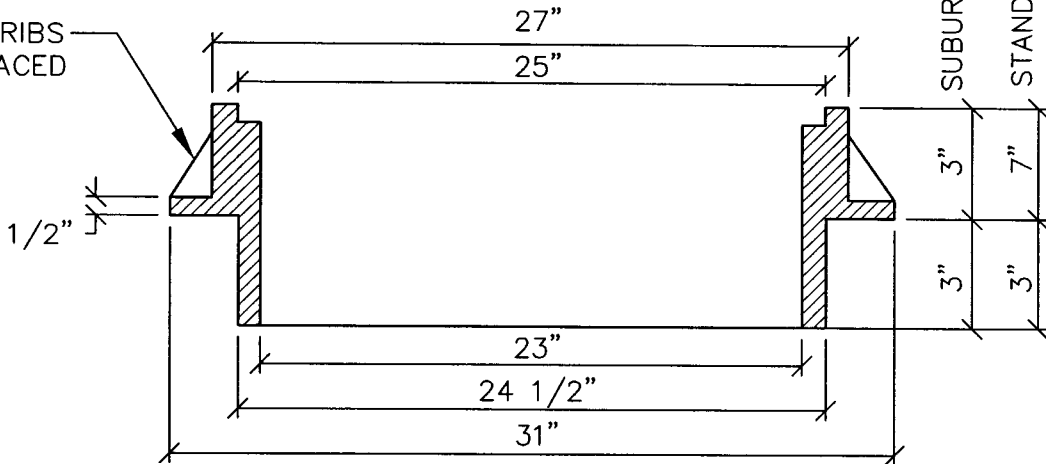
STORM



SUBURBAN FRAME

STANDARD FRAME

8 EA. -1/2" RIBS
EQUALLY SPACED



NOTES:

1. COVER AND FRAME SHALL BE GRAY CAST IRON
ASTM A-48, CLASS 30.
2. COVER AND FRAME TO BE MACHINED TO A TRUE
BEARING ALL AROUND.
3. NOTCH LID FOR LIFTING HOOK.

LAST REVISION DATE:
DEC 2007

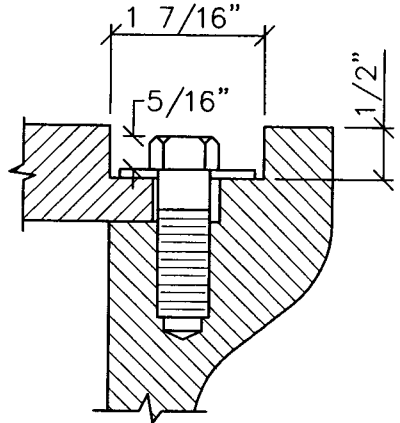
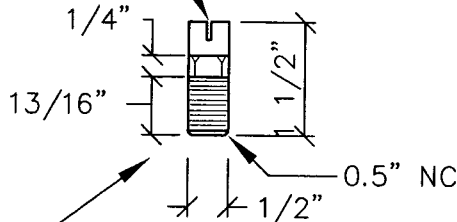
**MANHOLE FRAME AND COVER
(STANDARD AND SUBURBAN)**

(NTS)

CARLTON, OR

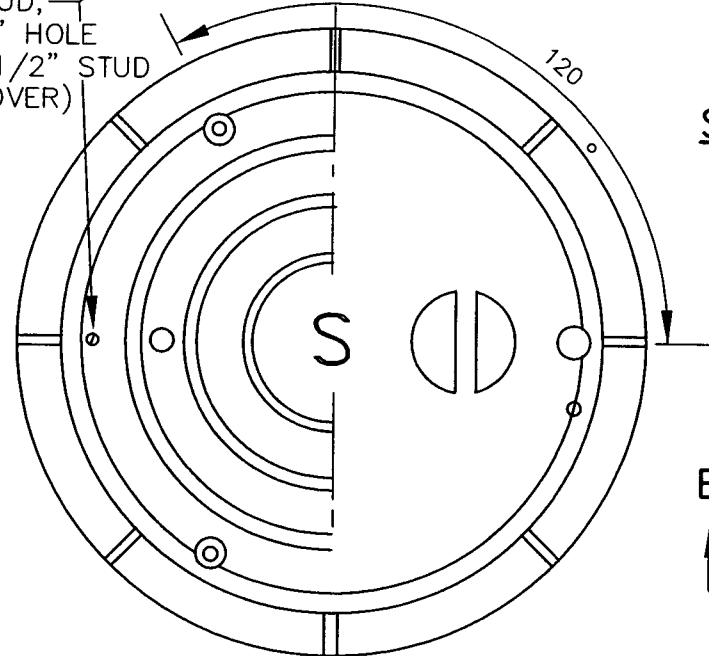
DETAIL NO.
405

SLOT FOR SCREWDRIVER

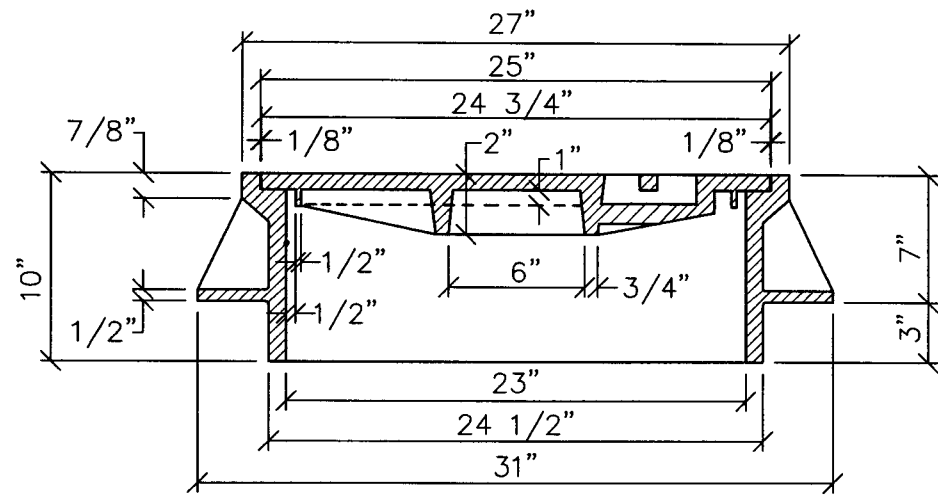
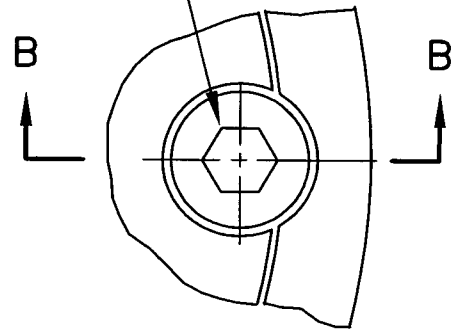


SECTION B-B

LOCATING STUD,
DRILL 25/64" HOLE
& TAP FOR 1/2" STUD
(ONE PER COVER)



1/2"-13NCx1"
STAINLESS STEEL
HEX HEAD
CAP SCREW

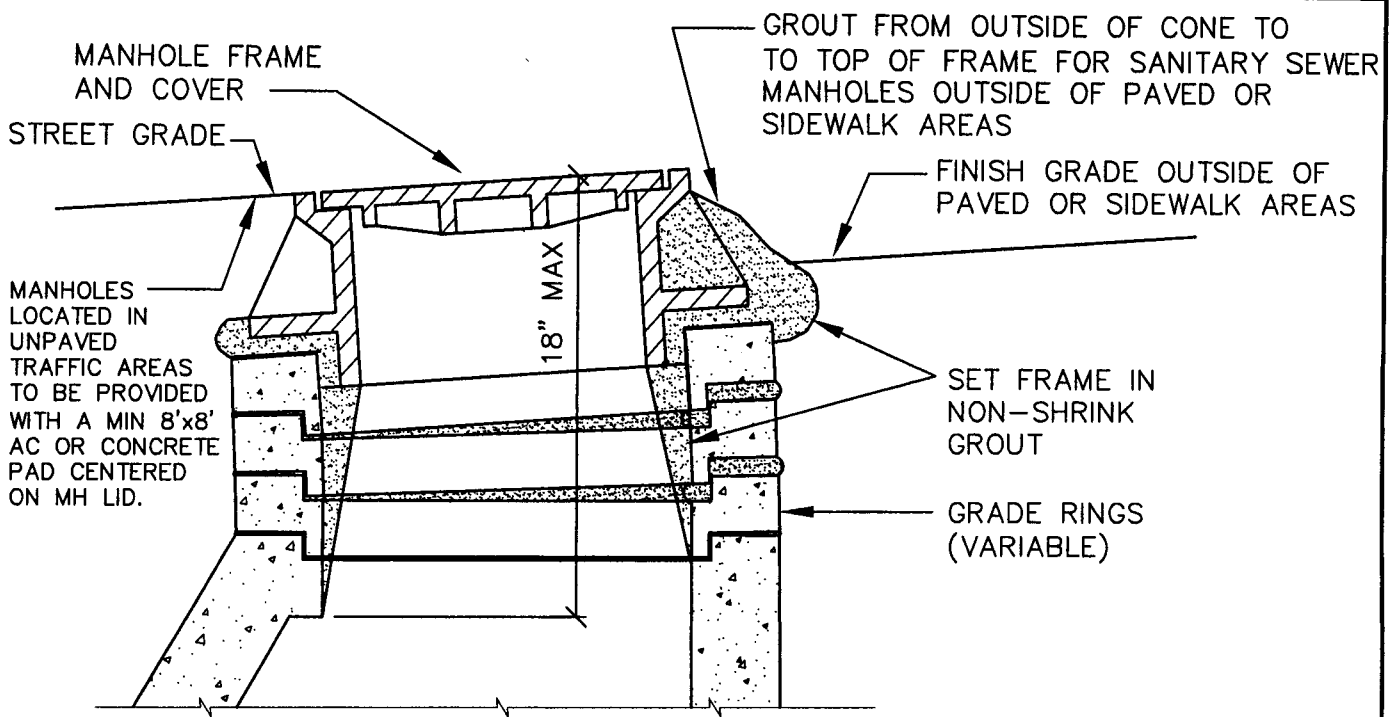


SECTION A-A

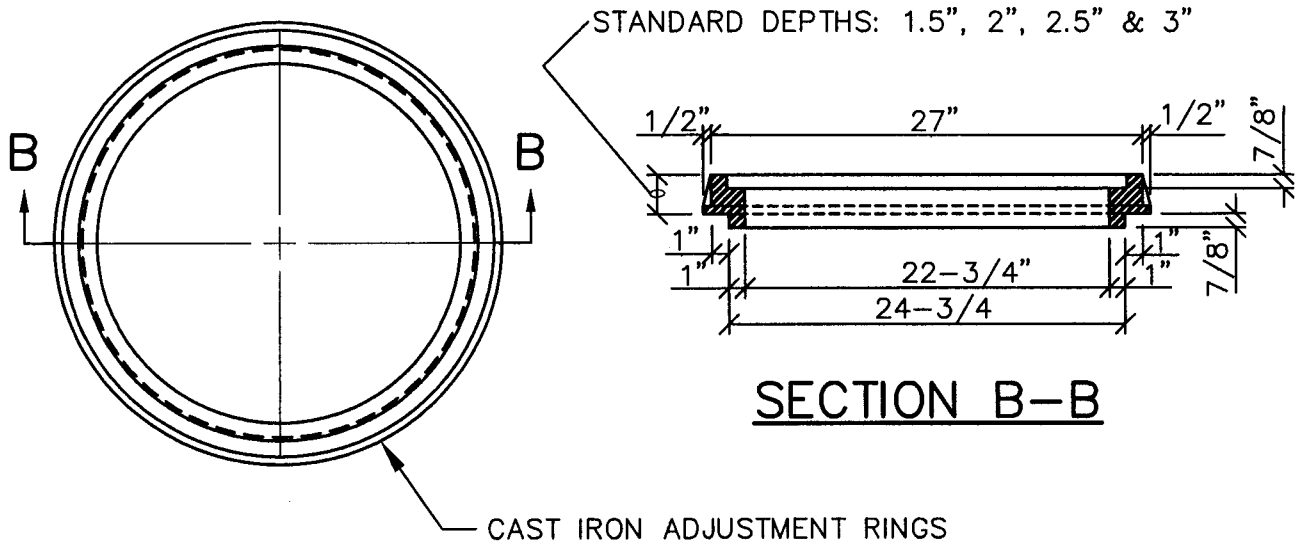
NOTES:

1. COVER AND FRAME TO BE MACHINED TO A TRUE BEARING ALL AROUND.
2. MATERIAL SHALL BE OF GRAY CAST IRON ASTM A-48, CLASS 30.

LAST REVISION DATE: DEC 2007	
LOCKDOWN MANHOLE FRAME AND COVER	
(NTS)	
CARLTON, OR	DETAIL NO. 406



TYPICAL MANHOLE GRADE ADJUSTMENT



SECTION B-B

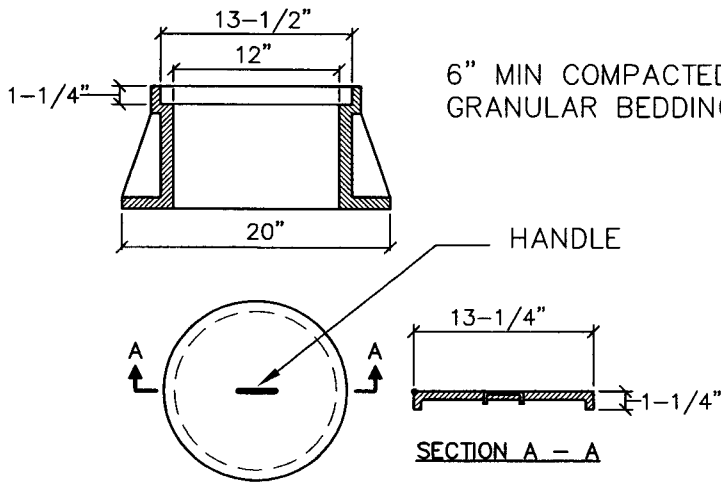
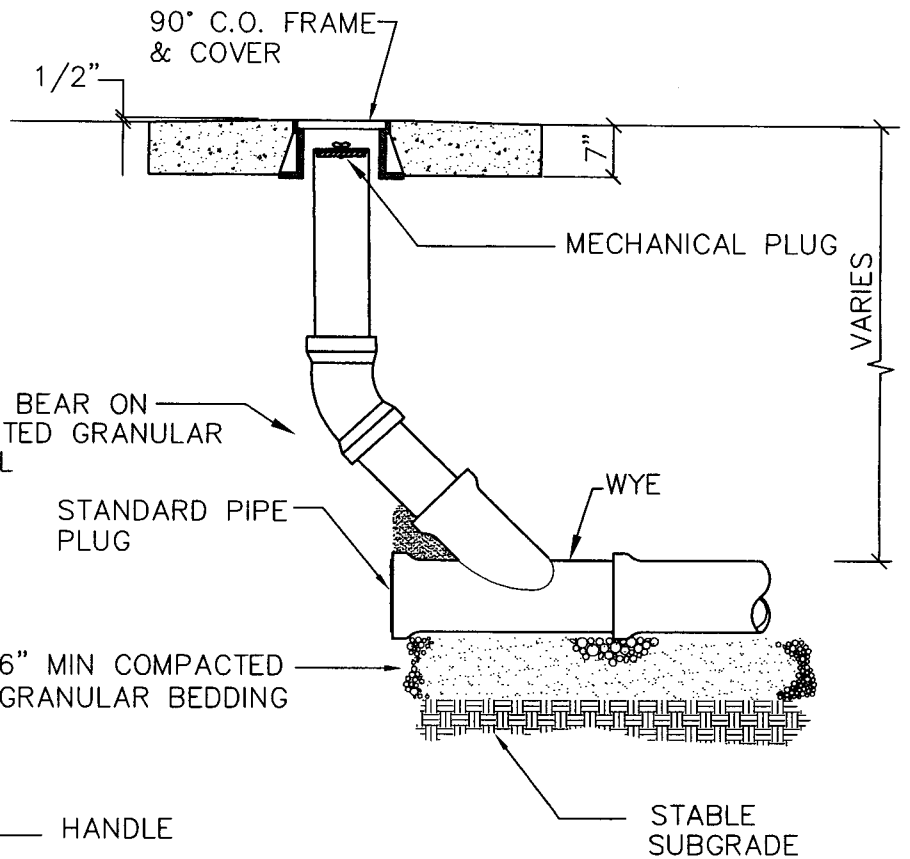
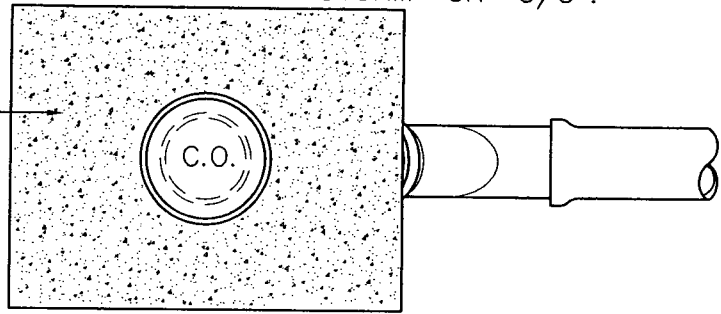
MANHOLE ADJUSTMENT RINGS FOR RESURFACING ONLY

- NOTES:
1. CAST IRON ADJUSTMENT RINGS ALLOWED ONLY WITH OVERLAYS AND NOT ON NEW MANHOLES. MAXIMUM 1 ADJUSTMENT RING PER MANHOLE.
 2. SANITARY SEWER – 2 HOLE LIDS
STORM DRAINS – 16 HOLE LIDS

LAST REVISION DATE: JAN 2010	COPYRIGHT 1996 WESTECH ENGINEERING, INC.
MANHOLE RIM ADJUSTMENT DETAILS	
(NTS)	
CARLTON, OR	DETAIL NO. 407

CLEANOUT COVERS: ALL SEWER CLEANOUT LIDS TO READ "SEWER"
 ALL STORM CLEANOUT LIDS TO READ "STORM" OR "C/O".

24" SQUARE CONCRETE PAD
 OR AC PAVEMENT OUTSIDE
 OF PAVED AREAS. SLOPE
 AWAY FROM CLEANOUT.



CLEANOUT FRAME & COVER

NOTES:

1. USE INLAND FOUNDRY MODEL 240 FRAME & COVER IN ALL AREAS.
2. COVER AND FRAME SHALL BE GRAY CAST IRON ASTM A-48, CLASS 30.
3. COVER AND FRAME TO BE MACHINED TO A TRUE BEARING ALL AROUND.

LAST REVISION DATE: MAY 2009	COPYRIGHT 1996 WESTECH ENGINEERING, INC.
MAINLINE CLEANOUT	
(NTS)	
CARLTON, OR	DETAIL NO. 411

NOTE: NO VERTICAL OR HORIZONTAL BENDS GREATER THAN 22-1/2° WITHIN RIGHT-OF-WAY OR PUBLIC UTILITY EASEMENT.

NOTE: PER ORS 92.044(7), SERVICE LINES MUST BE SET 1' MINIMUM CLEAR FROM ANY SURVEY MONUMENT

STAMP 2" TALL "S" OR "D" IN TOP OF CURB & GUTTER PAN AT POINT OF CROSSING.

PRESSURE TREATED 2" X 4" WIRED TO INVERT AND EXTENDING ABOVE FINISH GRADE. STAKE SHALL BE CONTINUOUS AND REMAIN VERTICAL AFTER BACKFILLING. END SHALL BE PAINTED & LABELED (WHITE FOR SEWER) (GREEN FOR STORM), AND LABELED WITH DEPTH TO PIPE. EXTEND TONING WIRE TO SURFACE.

PROPERTY LINE
CLEANOUT
SEE DETAIL 416

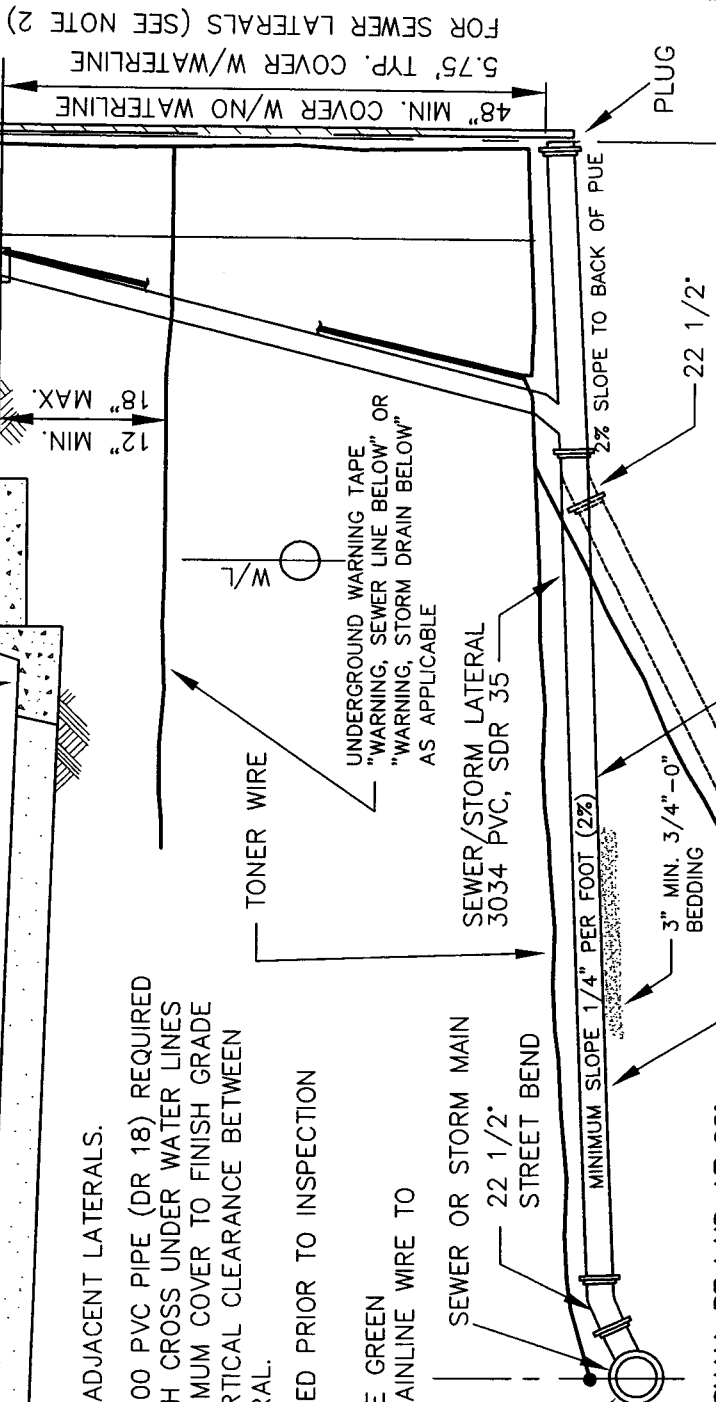
EASEMENT LINE
OR
P/L PUE

NOTES:

1. MIN. 18" SEPARATION BETWEEN ADJACENT LATERALS.
2. ONE FULL LENGTH (18') OF C-900 PVC PIPE (DR 18) REQUIRED FOR ALL SEWER LATERALS WHICH CROSS UNDER WATER LINES WITH LESS THAN 5.75 FEET MINIMUM COVER TO FINISH GRADE OR LESS THAN 18" MINIMUM VERTICAL CLEARANCE BETWEEN WATER LINE AND SERVICE LATERAL.
3. SERVICE SHALL NOT BE BACKFILLED PRIOR TO INSPECTION BY PUBLIC WORKS.
4. INSTALL A CONTINUOUS 12 GAUGE GREEN INSULATED TRACER WIRE FROM MAINLINE WIRE TO END OF LATERAL.

TYPICAL, SHALLOW MAINS

TYPICAL, DEEP MAINS



LATERALS SHALL BE LAID AT 90° TO RIGHT-OF-WAY OR EASEMENT LINE UNLESS OTHERWISE SHOWN ON PLANS.

SPLICE WITH WATERPROOF WIRE NUT (TYP ALL SERVICES)

3" MIN. 3/4"-0" BEDDING

MINIMUM SLOPE 1/4" PER FOOT (2%)

3" MIN. 3/4"-0" BEDDING

22 1/2° MAXIMUM BEND

MAINTAIN STRAIGHT GRADE FROM MAINLINE TO CLEANOUT (TYP)

MANUFACTURED TEE-WYES FOR 8-INCH MAINLINES & STANDARD TEES FOR 10-INCH & LARGER MAINLINES.

SEE DETAIL 418 OR 419 FOR CONNECTION TO EXISTING MAINLINES.

LAST REVISION DATE:
MAR 2008

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SEWER AND STORM SERVICE LATERALS

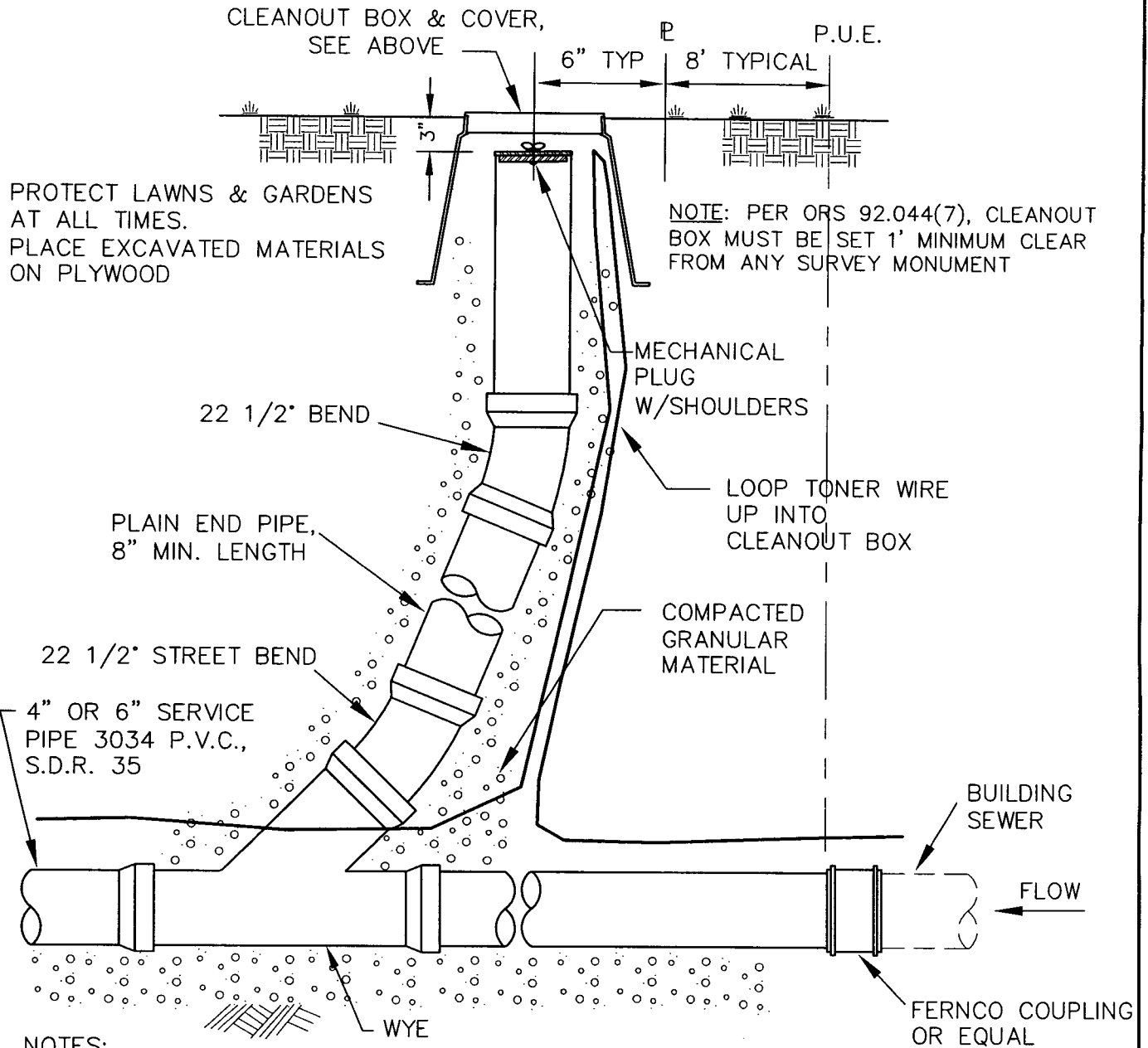
(NTS)

CARLTON, OR

DETAIL NO.
415

CLEANOUT COVERS: ALL SEWER CLEANOUT LIDS TO READ "SEWER"
 ALL STORM CLEANOUT LIDS TO READ "STORM" OR "C/O".

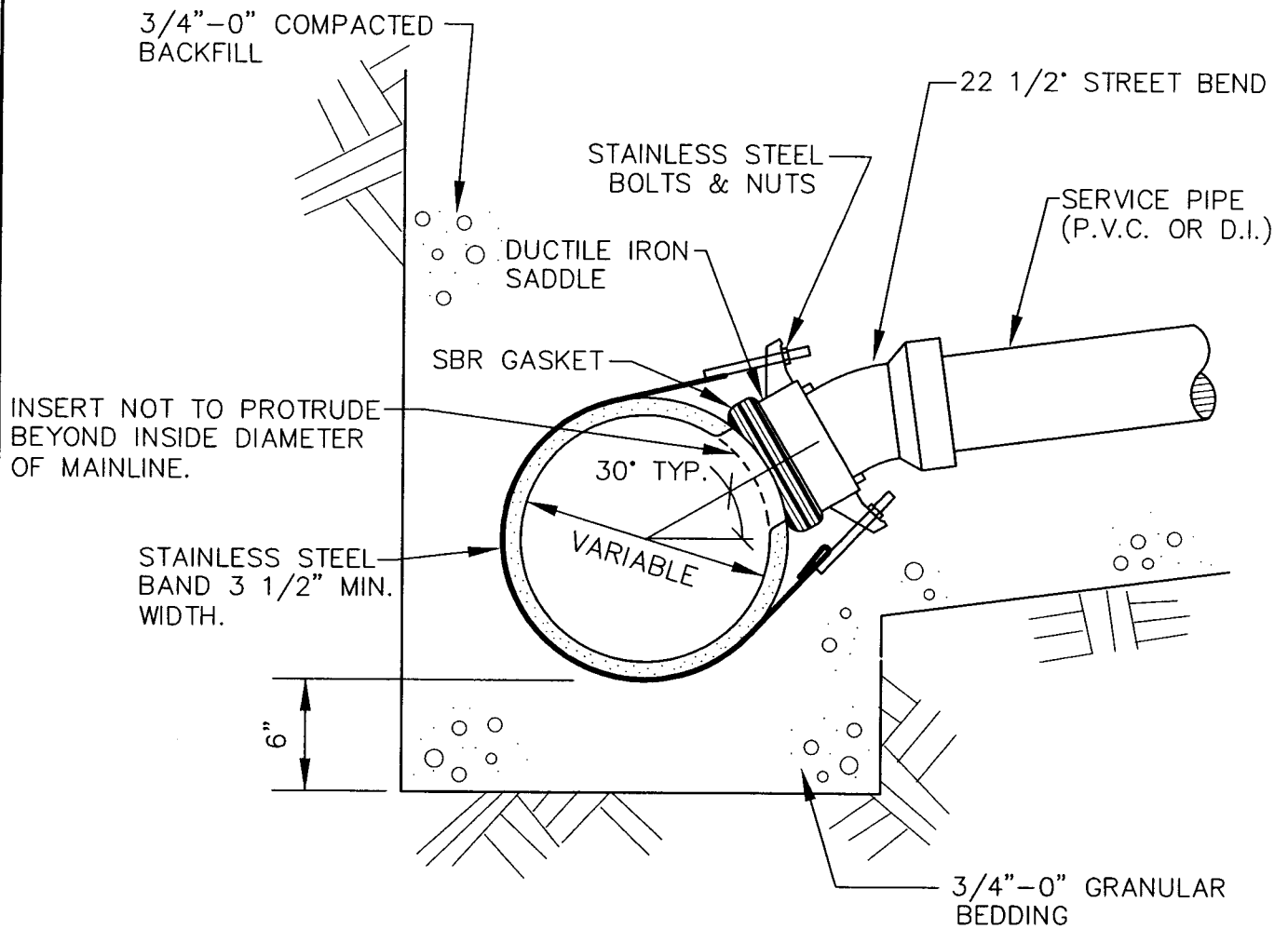
1. NON-TRAFFIC AREAS:
 CARSON MODEL 910 T-COVER OR EQUAL (GREEN FOR SEWER, GREY FOR STORM).
2. TRAFFIC AREAS, INCLUDING DRIVEWAYS:
 8" X 4" CAST IRON FRAME & COVER, OLYMPIC M1007 OR EQUAL.



NOTES:

1. CLEANOUT RISER SHALL BE SAME SIZE AND MATERIAL AS LATERAL PIPE.
2. PROVIDE CASTING FOR CLEANOUTS LOCATED IN DRIVEWAYS.
3. CLEANOUT PIPE SHALL BE LEFT A MINIMUM OF 18" ABOVE EXISTING GRADE UNTIL ALL CURBING IS INSTALLED AND ALL PRIVATE UTILITY TRENCHES ARE BACKFILLED. CLEANOUTS SHALL THEN BE SET FLUSH WITH FINISH GRADE.

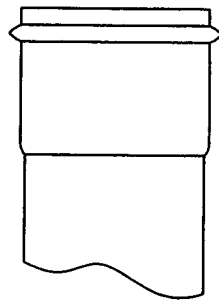
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STANDARD SERVICE LATERAL CLEANOUT	
(NTS)	
CARLTON, OR	DETAIL NO. 416



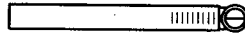
NOTES:

1. SERVICE SADDLES ALLOWED ON EXISTING SEWER MAINS ONLY. MANUFACTURED TEE-WYE FITTINGS SHALL BE USED ON ALL NEW MAINLINES.
2. THE TAP SHALL NOT BE MADE EXCEPT IN THE PRESENCE OF A CITY INSPECTOR; NOR SHALL ANY CONNECTION BE MADE WITHOUT CITY APPROVAL.
3. SERVICE SADDLES SHALL BE ROMAC STYLE "CB" OR EQUAL W/ VIRGIN SBR GASKET PER ASTM D2000 MBA 710.
4. HOLE IN MAIN SHALL BE CORE DRILLED.
5. ϕ OF TAP SHALL BE ABOVE SPRINGLINE.

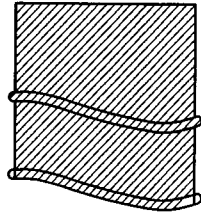
LAST REVISION DATE: DEC 2007	COPYRIGHT 1996 WESTECH ENGINEERING, INC.
SEVICE SADDLE CONNECTION TO EXISTING SEWERS	
(NTS)	
CARLTON, OR	DETAIL NO. 418



← PVC HUB (ASTM D-3034 SDR 35). DRIVE INTO CENTER OF RUBBER SLEEVE AFTER SLEEVE IS PLACED IN HOLE.

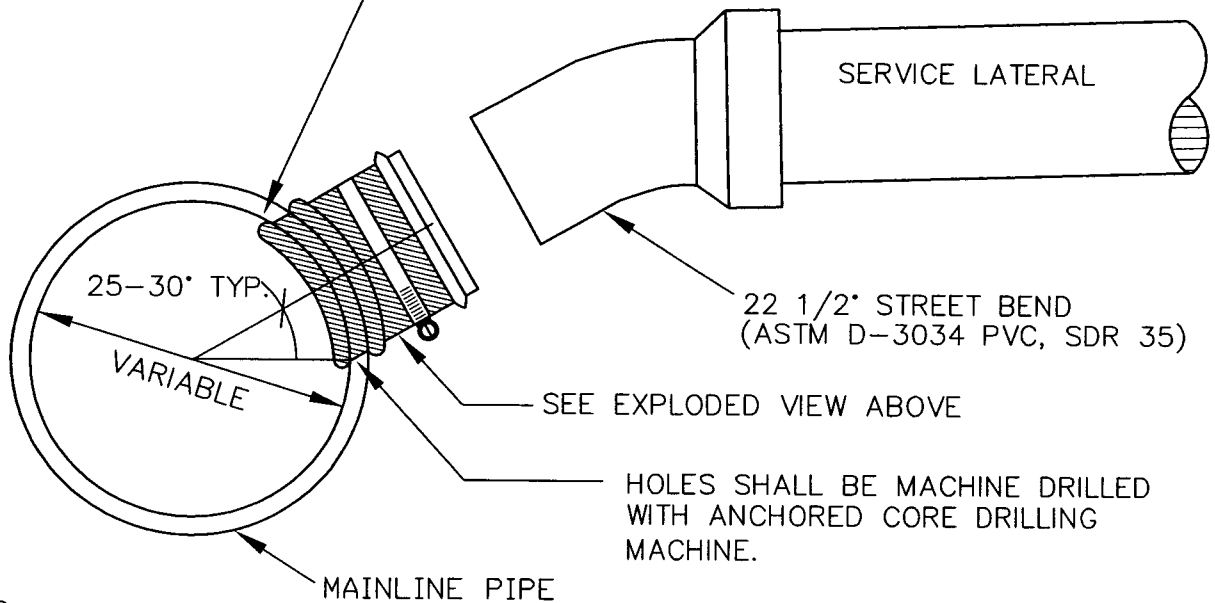


← STAINLESS STEEL BAND (9/16" SERIES 300) TO SECURE UPPER HALF OF RUBBER SLEEVE TO THE PVC HUB.



← MOLDED RUBBER SLEEVE (ASTM C-443) INCLUDES MOLDED SEGMENT(S) THAT HOLD THE SLEEVE IN PLACE.

PVC HUB NOT TO PROTRUDE BEYOND INSIDE DIAMETER OF RUBBER SLEEVE.



NOTES:

1. INSERTA-TEES TO BE USED ON EXISTING PVC OR DUCTILE IRON SEWER MAINS, AND MAY BE USED ON EXISTING CONCRETE OR AC PIPES W/PUBLIC WORKS APPROVAL. MANUFACTURED TEE-WYE FITTINGS SHALL BE USED ON ALL NEW MAINLINES.
2. THE TAP SHALL NOT BE MADE EXCEPT IN THE PRESENCE OF A CITY INSPECTOR; NOR SHALL ANY CONNECTION BE MADE WITHOUT CITY APPROVAL.
3. ϕ OF TAP SHALL BE ABOVE SPRINGLINE.

LAST REVISION DATE: DEC 2007	
INSERTA-TEE CONNECTION TO EXISTING SEWER	
(NTS)	
CARLTON, OR	DETAIL NO. 419

MANHOLE VACUUM TEST REPORT

Project Location: (City)			Project Name:				
Inspector: (Print)			Date: (Separate Report Required for Each Test Session)				
Testing Company: (Name & Phone #)							
Manhole No.	Manhole Diameter (inch)	Manhole Depth (ft)	Surface Restoration Complete?	Time Required ³ (sec)	Time to Drop from 10" Hg to 9" Hg (sec)	Results	Comments
			Yes / No			Pass / Fail	
			Yes / No			Pass / Fail	
			Yes / No			Pass / Fail	
			Yes / No			Pass / Fail	
			Yes / No			Pass / Fail	
			Yes / No			Pass / Fail	
			Yes / No			Pass / Fail	
			Yes / No			Pass / Fail	

1. All adjacent surface restoration shall be completed prior to conducting manhole acceptance tests, including finish paving and final adjustments to grade. Any test conducted prior to completion of surface restoration shall be considered informal, and will not count for acceptance.
2. The vacuum test head seal shall be inflated in accordance with the manufacturer's recommendations, but in all cases the grade rings and casting shall be included in the test. A vacuum of 10-inches of mercury shall be drawn and the vacuum pump shut off. With the valves closed, the time shall be measured for the vacuum to drop to 9-inches.
3. The manhole shall pass if the time for the vacuum reading to drop to 9-inches meets or exceeds the values indicated on the following table. Times for deeper depths as required by the City Engineer.

REQUIRED MANHOLE VACUUM TEST TIMES			
Manhole Depth (feet)	Required Time (sec)		
	48-inch diameter	60-inch diameter	72-inch diameter
8	20	26	33
10	25	33	41
12	30	39	49
14	35	46	57
18	40	52	65
20	45	59	73
22	50	65	81

SANITARY SEWER AIR TEST REPORT

Project Location:				Project Name:						
Inspector: (Print)				Date: (Separate Report Required for Each Test Session)						
TV Inspection Required? Yes / No				Mandrel Testing Completed? Date Completed or Scheduled:						
Station (& Manhole #)		Main/ Lateral	Size & Material	Total Length (ft)	C ¹	K ¹	Test Time (Seconds) for Pressure Drop Shown (psi)			Comments
							From	To	Required ²	
		Main								Pass / Fail
		Laterals								
		Totals								
		Main								Pass / Fail
		Laterals								
		Totals								
		Main								Pass / Fail
		Laterals								
		Totals								
		Main								Pass / Fail
		Laterals								
		Totals								

¹ For C and K values, see table and formulas on reverse side.

² For total C ≤ 1.0, test time (seconds) required = 2 times K
 For total C > 1.0, test time (seconds) required = 2 times (K/C)

TEST PROCEDURE

1. Add air slowly to the portion of the pipe installation under test until the internal air pressure is raised to 4.0 psig (or higher pressure as required to address groundwater). Increase the test pressure by 0.433 psi for each foot of average ground water depth over the exterior crown of the pipe under test, with the maximum test pressure not to exceed 9.0 psi.
2. Add air slowly until the internal air pressure is raised to 4.0 psig (or higher pressure as required due to groundwater).
3. After required test pressure is reached, allow 2-minutes minimum for air temperature to stabilize, adding only the amount of air required to maintain pressure.
4. After the temperature stabilization period, disconnect the air supply.
5. Record the time required for the internal air pressure to drop from 3.5 psi (or higher as required due to groundwater backpressure) to 2.5 psi (or higher as required due to groundwater backpressure). If this time exceeds the required time (or if there is less than 1.0 psi pressure drop), the test is successful.

ACCEPTANCE: The tested sewer section shall be considered acceptable if the pressure drop during the test time is less than 1.0 psi from the starting pressure.

SEWER AIR TEST C AND K VALUES

Pipe Size (inch)	C-Value ¹ per foot length	K-Value ² per foot length
4	0.00155	0.176
6	0.00233	0.396
8	0.00311	0.704
10	0.00388	1.100
12	0.00466	1.584
15	0.00582	2.475
18	0.00699	3.564
21	0.00815	4.851

¹ C = 0.0003882dL Where d = diameter (inches)
² K = 0.011d²L L = Length (ft)

Example:

Air Test a system consisting of two mainline segments as follows:

Segment 1: 395 feet of 8-inch mainline, 100 feet of 4-inch laterals, and 35 feet of 6 inch laterals.

Segment 2: 200 feet of 8-inch mainline, 30 feet of 4-inch laterals, and 20 feet of 6 inch laterals.

Station (& Manhole #)		Main/Lateral	Size & Material	Total Length (ft)	C ¹	K ¹	Test Time (Seconds) for Pressure Drop Shown (psi)			Comments	
From	To						Required ²	4.0 - 3.5	3.5 - 2.5		
0+00 MH A1	3+95 MH A2	Main	8" PVC	395	1.2285	278.1	310/1.46= 212			Pass / Fail	
		Laterals	4" PVC 6" PVC	100 35	0.155 0.01855	17.6 13.86					212*2= 414 sec
		Totals			1.465	309.54					
3+95 MH A2	5+95 MH A3	Main	8" PVC	200	0.622	140.8	2*154= 308 sec			Pass / Fail	
		Laterals	4" PVC 6" PVC	20 30	0.0465 0.0466	5.28 7.92					
		Totals			0.715	154.0					

Note: For total C ≤ 1.0, test time (seconds) required = 2 times K
 For total C > 1.0, test time (seconds) required = 2 times (K/C)

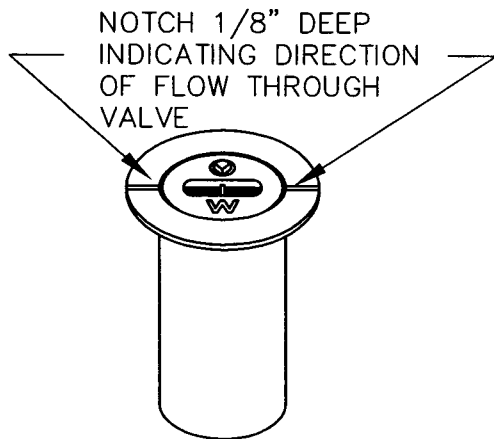
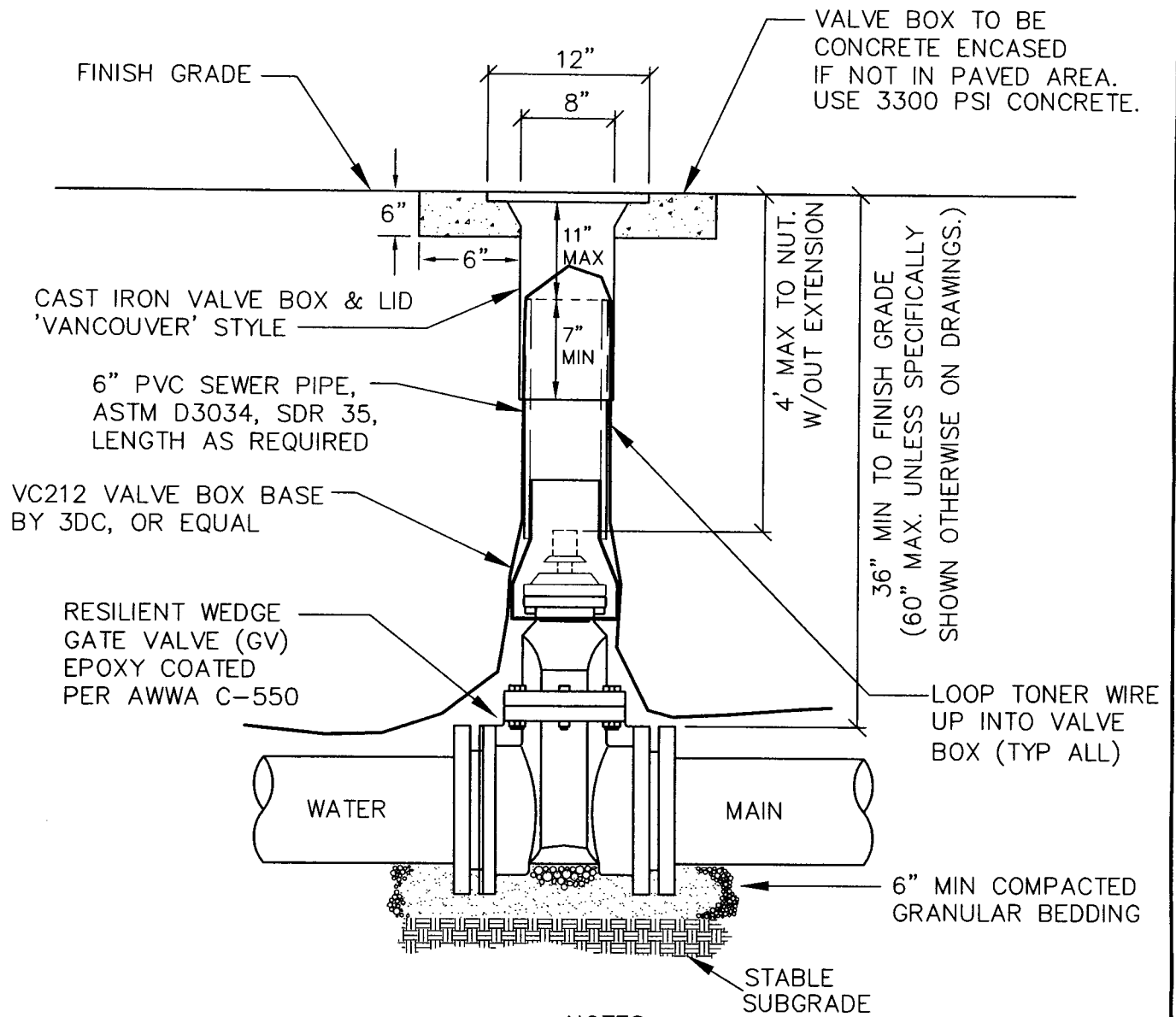
The tested sewer section shall be considered acceptable when tested as described herein if the section under test does not loose air at a rate greater than 0.0015 cfm per square foot of internal sewer surface.

SANITARY SEWER MANDREL TEST REPORT

Project Location: (City)	Project Name:
Inspector: (Print)	Date: (Separate Report Required for Each Test Session)
Mandrel Diameters Verified? Yes / No	

Station (& Manhole #)		Size & Material	Length (ft)	Results	Backfill Compaction Completed?	Date Sewer Flushed & Cleaned	Comments
From	To						
				Pass / Fail	Yes / No		
				Pass / Fail	Yes / No		
				Pass / Fail	Yes / No		
				Pass / Fail	Yes / No		
				Pass / Fail	Yes / No		
				Pass / Fail	Yes / No		
				Pass / Fail	Yes / No		
				Pass / Fail	Yes / No		
				Pass / Fail	Yes / No		
				Pass / Fail	Yes / No		
				Pass / Fail	Yes / No		
				Pass / Fail	Yes / No		
				Pass / Fail	Yes / No		
				Pass / Fail	Yes / No		
				Pass / Fail	Yes / No		

1. Mandrel testing shall be conducted on a manhole to manhole (or cleanout) basis and shall be done after the line has been completely flushed out with water.
2. Mandrel testing shall be conducted after trench backfill and compaction has been completed.
3. The mandrel diameter shall be 95% of the pipe initial inside diameter. The inspector shall verify the diameter of each mandrel used during each test session.

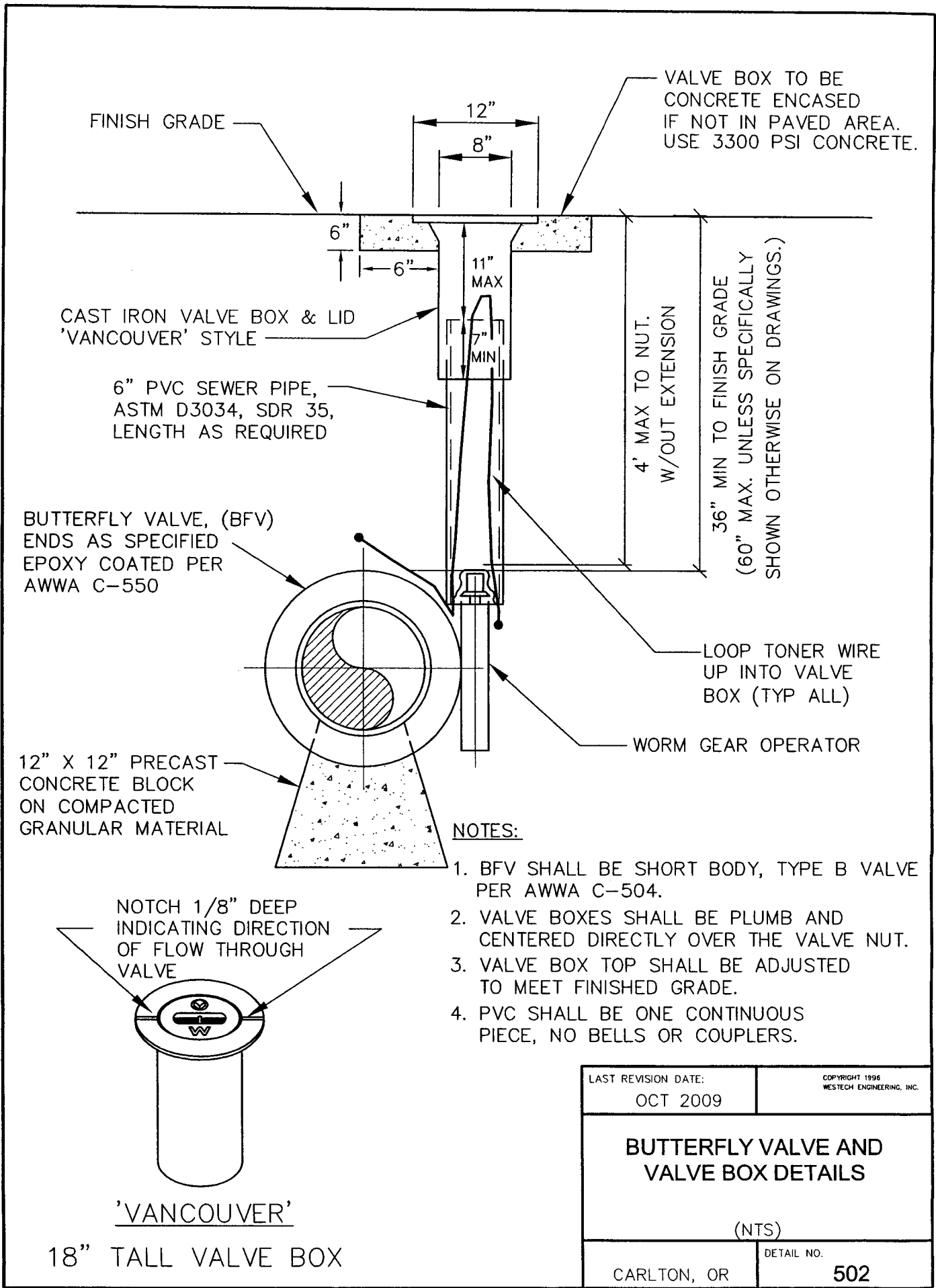


'VANCOUVER'
18" TALL VALVE BOX

NOTES:

1. GV SHALL CONFORM TO AWWA C-509.
2. VALVE BOXES SHALL BE PLUMB AND CENTERED DIRECTLY OVER THE VALVE NUT.
3. VALVE BOX TOP SHALL BE ADJUSTED TO MEET FINISHED GRADE.
4. PVC SHALL BE ONE CONTINUOUS PIECE, NO BELLS OR COUPLERS.

LAST REVISION DATE: MAR 2008	COPYRIGHT 1996 WESTECH ENGINEERING, INC.
GATE VALVE AND VALVE BOX DETAIL	
(NTS)	
CARLTON, OR	DETAIL NO. 501



NOTE: HYDRANT COLOR TO BE FACTORY RED

(2) 2½" HOSE NOZZLES (NST). REMOVE CHAINS FROM CAPS

4 1/2" PUMPER NOZZLE W/NST THREAD, INSTALL 4-INCH STORZ ADAPTER (FSAF/MF) WITH SNAP-TITE STORZ BLIND CAP & CABLE.

NOTE: PER ORS 92.044(7), FIRE HYDRANT MUST BE SET 1' MINIMUM CLEAR FROM ANY SURVEY MONUMENT

6" MIN. FROM P/L S/W

2" MIN
6" MAX

12" MIN. BEHIND S/W
24" MIN. BEHIND CURB

STD. VALVE BOX (VANCOUVER STYLE) W/VC212 VB BASE

3' TYP
2%
S/W

DRAIN ROCK SEE NOTE 3

THRUST BLOCK, BEARING TO BE EQUIV. TO SIZING FOR TEE, SEE NOTE 5 & 8.

36" MIN.
FIELD-LOK TYPE GASKETS REQUIRED ON ANY PUSH-ON JOINTS

LOOP TONER WIRE UP INTO VALVE BOX (TYP ALL)

6" FL X MJ RESILIENT WEDGE GATE VALVE

VARIES

6" D.I.

12" X 12" CONC. BLOCK

RETAINER GLANDS (TYP ALL JOINTS)
6" FLG X MJ SHOE

UNDISTURBED EARTH (TYP)

MAINLINE TEE 6" FLANGED SIDE OUTLET

NOTES:

1. HYDRANTS TO BE MUELLER SUPER CENTURION 250, MODEL A-423 W/FULL SIZE (5¼") FOOT VALVE.
2. **ALL FITTINGS IN CONTACT WITH CONCRETE SHALL BE WRAPPED IN PLASTIC.** HYDRANT DRAIN HOLES TO REMAIN OPEN TO DRAIN ROCK AND OPERATIONAL.
3. 1-1/2" TO 3/4" CLEAN DRAIN ROCK SHALL BE PLACED A MIN. OF 6" ABOVE DRAIN OUTLET.
4. WHERE PLANTER STRIP EXISTS, HYDRANT SHALL BE PLACED SO FRONT PORT IS A MIN. OF 24" BEHIND FACE OF CURB.
5. THRUST BLOCK AT STANDARD 6" FIRE HYDRANT TEE SHALL HAVE MIN. 3.7 SQ. FT. BEARING AREA.
6. ALL HYDRANTS SHALL BE SET PLUMB.
7. FOR HYDRANT LEADS LONGER THAN 30', AN ADDITIONAL GATE VALVE SHALL BE PROVIDED WITHIN 3 FT. OF THE HYDRANT.
8. RETAINER GLANDS MAY BE USED IN LEIU OF HYDRANT THRUST BLOCK ON NEAR SIDE HYDRANTS.
9. PAINT CURB RED 10 FEET EACH WAY FROM HYDRANT & INSTALL REFLECTIVE BLUE TRAFFIC MARKER @ STREET CENTERLINE.

LAST REVISION DATE:
JUNE 2010

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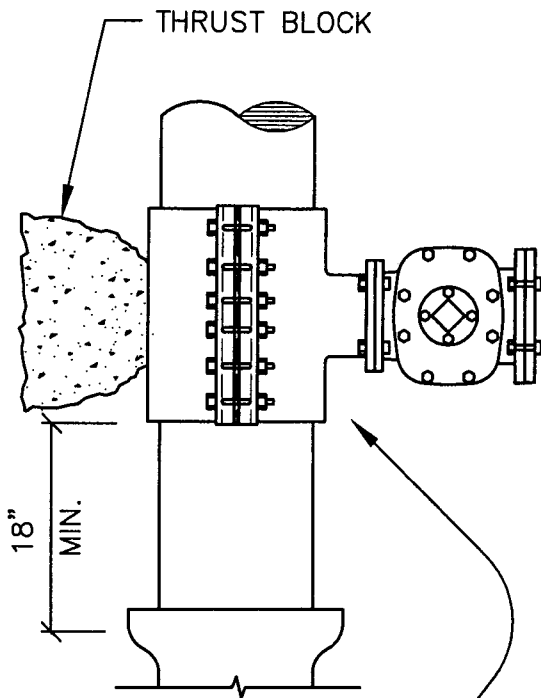
**STANDARD
FIRE HYDRANT ASSEMBLY**

(NTS)

CARLTON, OR

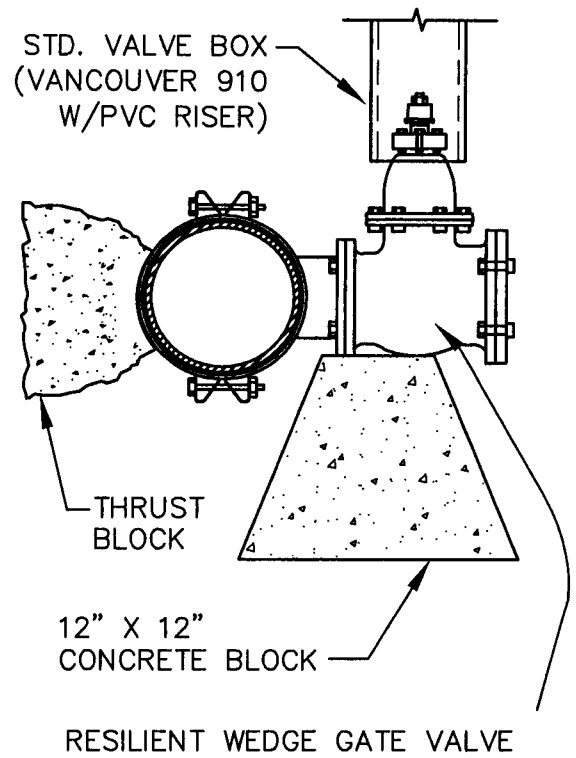
DETAIL NO.

503



ROMAC SST/SSTIII, MUELLER H304,
JCM MODEL 432 OR APPROVED EQUAL

TOP VIEW



RESILIENT WEDGE GATE VALVE

SIDE VIEW

NOTES:

1. WATER MAIN SHALL BE CLEANED & SPRAYED WITH CHLORINE SOLUTION IN TAP AREA BEFORE ATTACHING SLEEVE.
2. TAPPING SLEEVE SHALL BE ALL STAINLESS STEEL WITH FULL PERIMETER GASKET.
3. TAPPING VALVE SHALL BE EPOXY COATED PER AWWA C-550.
4. SLEEVE AND VALVE SHALL BE PRESSURE TESTED BEFORE MAKING TAP. PRESSURE TEST AND TAP SHALL BE MADE IN THE PRESENCE OF AN AUTHORIZED CITY REPRESENTATIVE.
5. APPROVED TAPPING MACHINE SHALL BE USED TO MAKE TAP.
6. 3/4" GRANULAR BACKFILL SHALL BE PLACED AND COMPACTED TO 95% OF MAXIMUM DENSITY AS DETERMINED BY AASHTO T-180.
7. THRUST BLOCKING REQUIREMENTS SHALL BE DETERMINED BY THE ENGINEER.
8. TAP SHALL BE MADE NO CLOSER THAN 18" FROM THE NEAREST JOINT.
- 9. SLEEVE AND VALVE SHALL BE WRAPPED WITH 8 MIL PLASTIC PRIOR TO CONCRETE PLACEMENT.**
10. CONCRETE BLOCK(S) SHALL COMPLETELY SUPPORT TAPPING TEE AND VALVE.
11. CONTRACTOR SHALL COORDINATE ALL TAPS WITH CITY AND PERFORM ALL TAPS WITH PUBLIC WORKS STAFF PRESENT.
12. ALL TAPPING EQUIPMENT (AND ANY TOOL COMING IN CONTACT WITH THE PIPE THROUGH THE TAPPING SLEEVE) SHALL BE CHLORINE DISINFECTED WITH A 300 MG/L CHLORINE SOLUTION.

LAST REVISION DATE:
JAN 2010

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TAPPING TEE
AND VALVE

(NTS)

CARLTON, OR

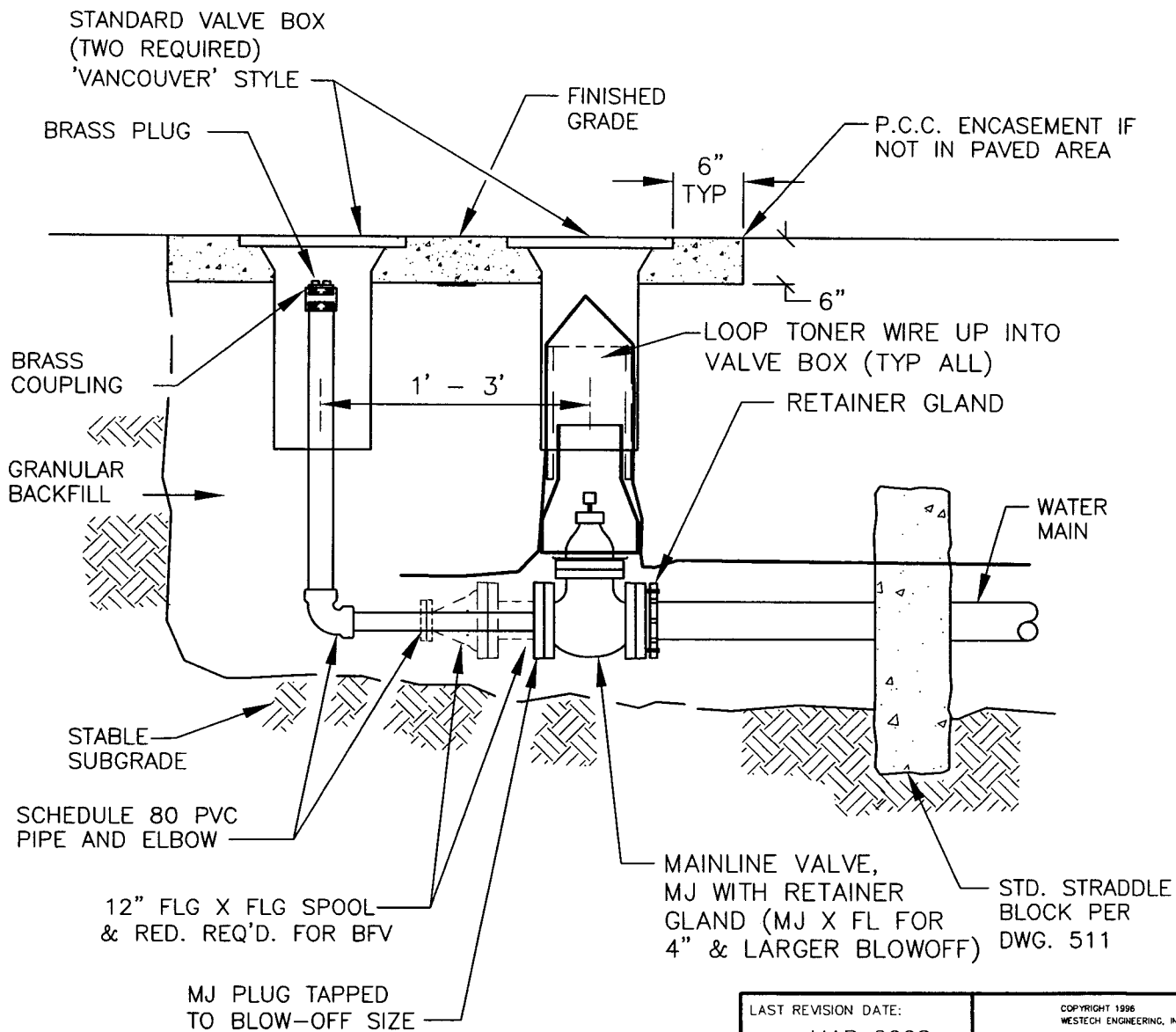
DETAIL NO.

505

BLOW-OFF SIZES REQUIRED (ASSUMES 40 PSI RESIDUAL PRESS.)	
MAIN SIZE	BLOW-OFF SIZE
6" - 8"	2"
10" - 12"	4"
>12"	BY ENGR.

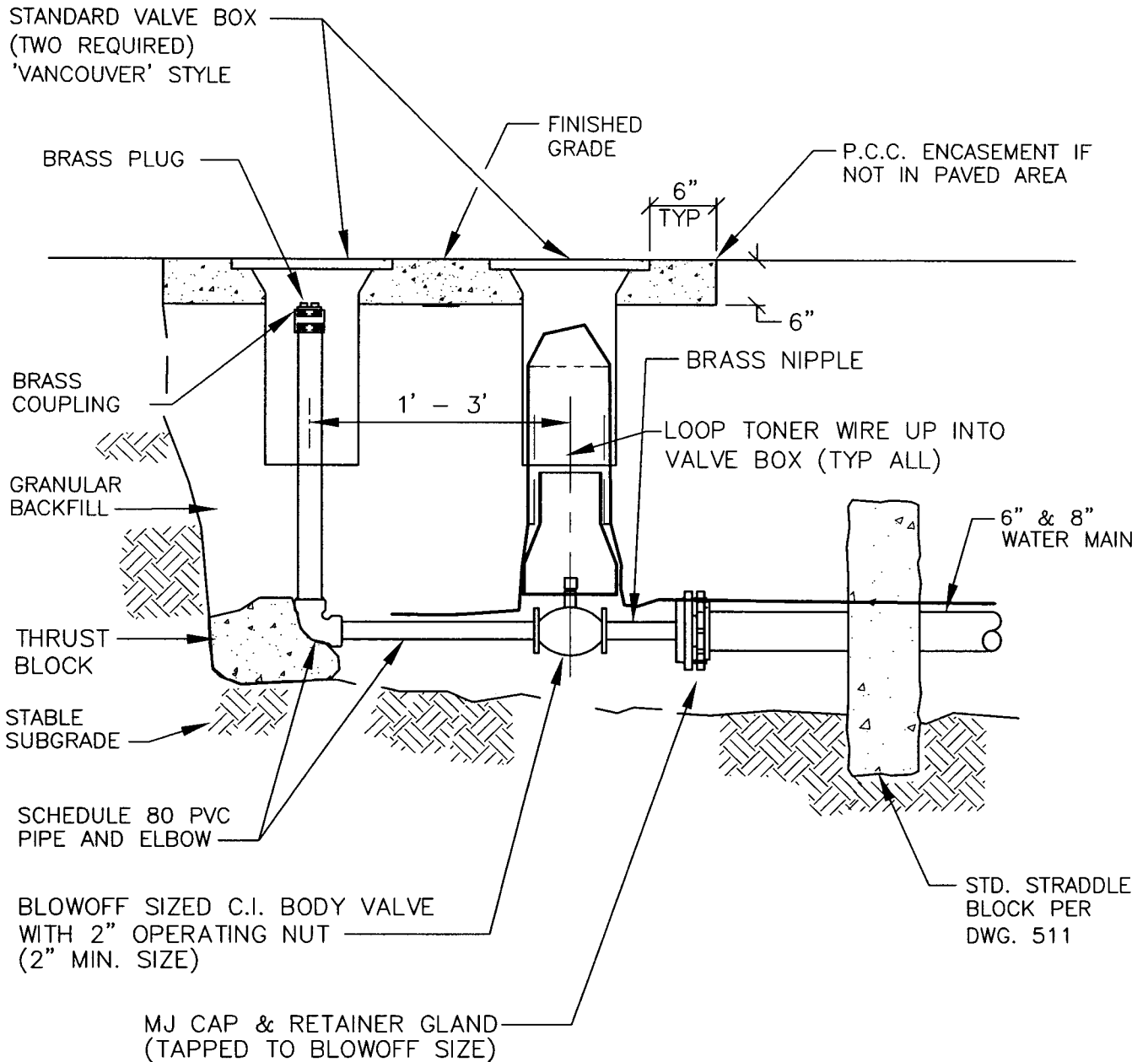
NOTES:

1. BACKFILL WITH GRANULAR BACKFILL.
2. REQUIRED ON ALL LINES WHICH MAY BE EXTENDED IN FUTURE OR AS DIRECTED BY CITY ENGINEER.
3. ALL CONCRETE TO BE 3300 PSI @ 28 DAYS.
4. FLANGED DUCTILE IRON PIPE AND FITTINGS MAY BE REQUIRED FOR 4" & LARGER BLOWOFFS.



NOTE: PER ORS 92.044(7), VALVE BOXES MUST BE SET 1' MINIMUM CLEAR FROM ANY SURVEY MONUMENT

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MAINLINE BLOWOFF ASSEMBLY	
(NTS)	
CARLTON, OR	DETAIL NO. 506

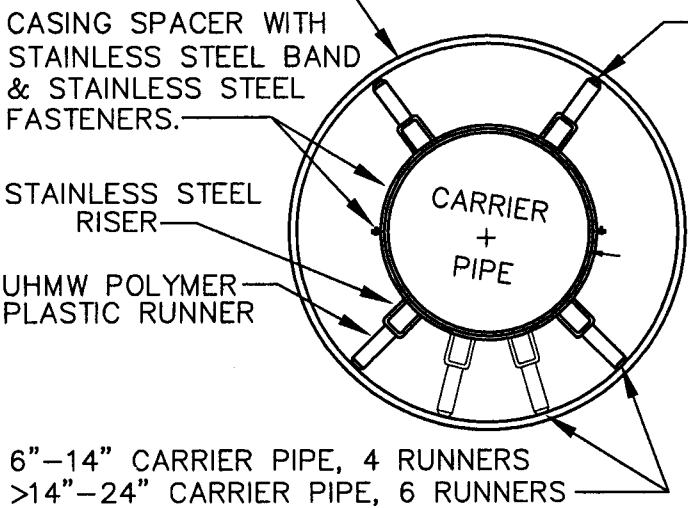
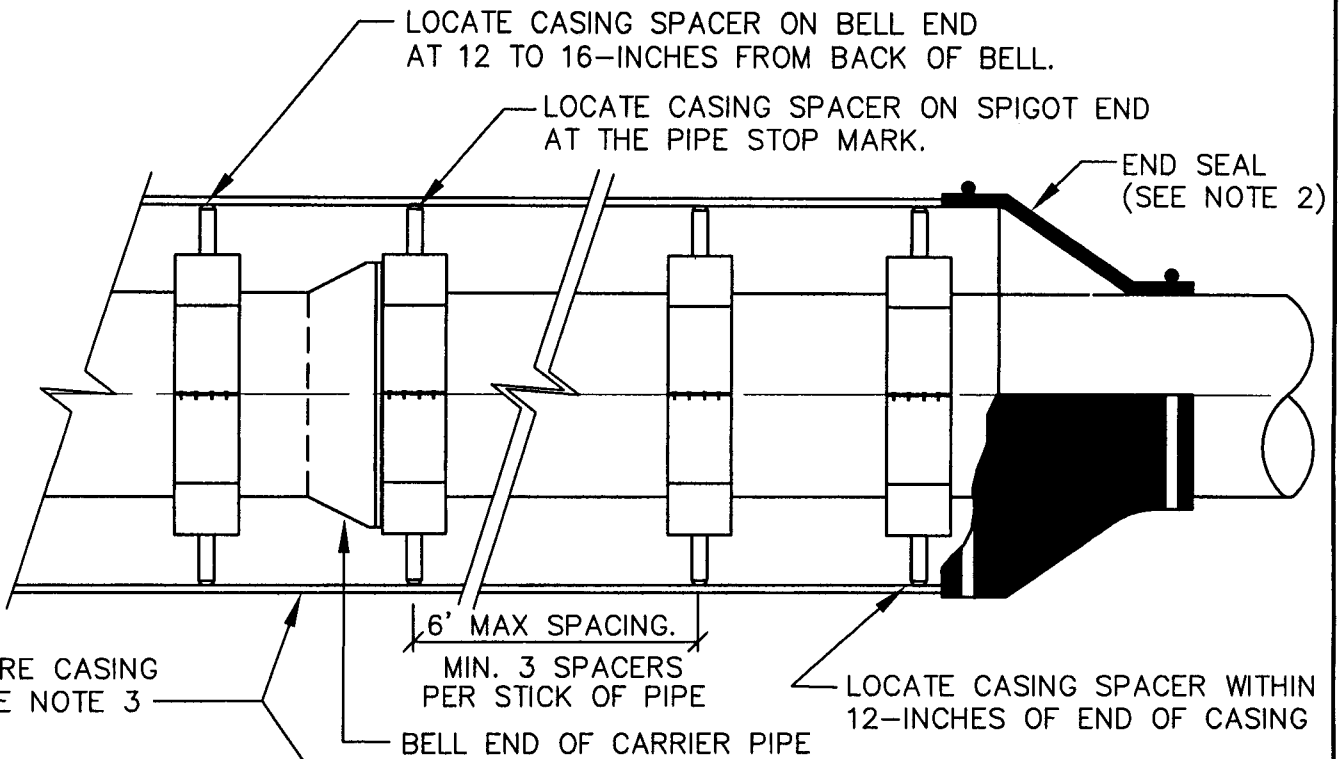


NOTE: PER ORS 92.044(7), VALVE BOXES MUST BE SET 1' MINIMUM CLEAR FROM ANY SURVEY MONUMENT

NOTES:

1. BACKFILL WITH GRANULAR BACKFILL.
2. ALLOWED ONLY ON PERMANENT DEAD END LINES IN CUL-DE-SACS WHICH CANNOT BE EXTENDED IN THE FUTURE.
3. ALL CONCRETE TO BE 3300 PSI @ 28 DAYS.
4. 2" BLOWOFF SIZE ASSUMES 40 PSI RESIDUAL PRESSURE.

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STANDARD BLOWOFF WITH PLUGGED END	
(NTS)	
CARLTON, OR	DETAIL NO. 507



6"-14" CARRIER PIPE, 4 RUNNERS
 >14"-24" CARRIER PIPE, 6 RUNNERS

CARRIER PIPE DIAMETER	MIN. DIA. CASING (*1, *2)	MIN CASING WALL THICKNESS (INCH)
6"	12"	0.25 (1/4)
8"	14"	0.25 (1/4)
10"	16"	0.312 (5/16)
12"	18"	0.375 (3/8)

*1: CASING SIZE LISTED IS FOR PRESSURE PIPE. LARGER DIA CASING REQ'D FOR GRAVITY PIPE.
 *2: SEE PWDS 5.8.m FOR GRAVITY PIPE CASING SIZE REQUIREMENTS OR LARGER CASING SIZES.

NOTES:

- CASING SPACERS - APS MODEL SSI, CALPICO M-SS SERIES OR APPROVED EQUIV. 4"-18" CARRIER PIPE, USE 8" WIDE BAND. >18" CARRIER PIPE, USE 12" WIDE BAND.
- SEAL BOTH ENDS OF BORE CASING WITH END SEALS. WITHOUT SAND FILL, USE APS MODEL AZ OR APPROVED EQUIV. FASTEN TO CASING AND CARRIER PIPE WITH ST. STEEL BANDS. WITH SAND FILL, USE GROUT END CAPS (PLUG VENT TUBES AFTER SAND FILL).
- CASING SHALL BE WELDED SMOOTH STEEL PIPE CONFORMING TO ASTM A-53, GRADE B OR APPROVED EQUIVALENT ($F_y = 35,000$ psi).
- CARRIER PIPE DIAMETER & MATERIAL AS PER DWGS.
- FOR GRAVITY SEWER OR STORM CARRIER PIPES, THE CASING ANNULAR SPACE SHALL BE COMPLETELY FILLED WITH SAND TO PREVENT FLOATATION OF CARRIER PIPE BY GROUNDWATER.
- CARRIER PIPE SHALL BE COMPLETELY FILLED WITH WATER PRIOR TO INSTALLING OR BLOWING SAND.
- INCREASE CASING DIA AS REQ'D TO ALLOW TRIMMING OF CASING SPACERS ON GRADE CRITICAL BORES

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BORE CASING, CARRIER PIPE AND CASING SPACER DETAIL (NTS)	
CARLTON, OR	DETAIL NO. 508

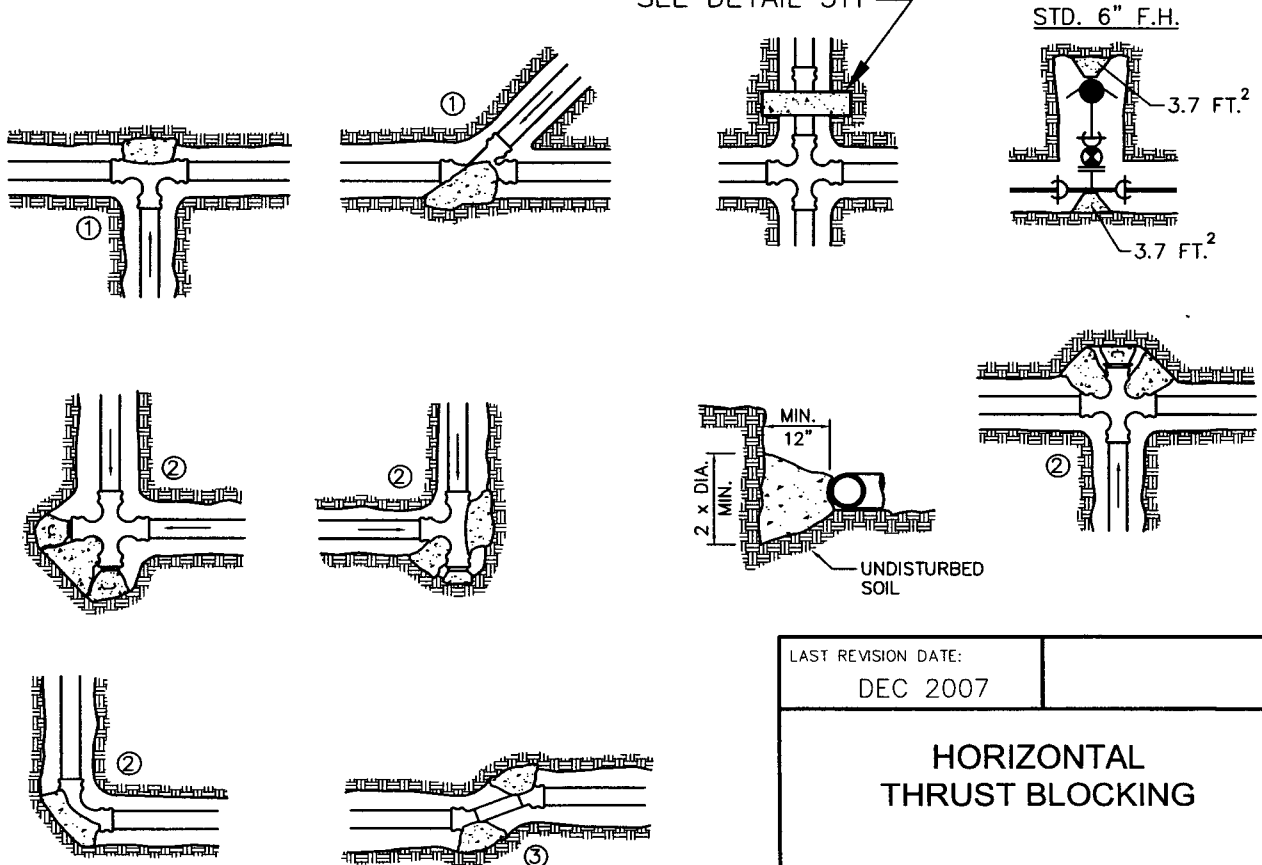
FITTING SIZE (Inches)	TEE, WYE, & ① HYDRANTS	90° BEND ② PLUGGED CROSS TEE PLUGGED-RUNS	45° BEND ③	22 1/2° BEND ③	11 1/4° BEND ③
2	*	*	*	*	*
4	1.7	2.4	1.3	*	*
6	3.7	5.3	2.9	1.5	*
8	6.7	9.5	5.1	2.7	1.3
10	10.5	14.8	8	4.1	2
12	15.1	21.3	11.6	5.9	2.9
16	26.8	37.9	20.5	10.4	5.2
18	33.9	47.9	25.9	12.8	6.7
LARGER	* *	* *	* *	* *	* *
BEARING AREA OF THRUST BLOCKS (sq. ft.)					

- ALL VALUES ARE BASED ON THE FOLLOWING ASSUMPTIONS:
AVG. PRESSURE = 100 PSI x 2 (safety factor); 1500 PSF SOIL BEARING CAPACITY; NORMAL DISTRIBUTION DESIGN VELOCITY NOT TO EXCEED 5 FPS.
- ALL FITTINGS SHALL BE WRAPPED IN PLASTIC PRIOR TO PLACEMENT OF CONCRETE.**
- BEARING SURFACE OF THRUST BLOCKING SHALL BE AGAINST UNDISTURBED SOIL.
- ALL CONCRETE MIX SHALL HAVE A MIN. 28 DAY STRENGTH OF 3300 PSI.
- ALL PIPE ZONES SHALL BE BACKFILLED WITH GRANULAR BACKFILL AND COMPACTED.
- THRUST BLOCKS FOR PLUGGED CROSS AND PLUGGED TEE SHALL HAVE #4 REBAR LIFTING LOOPS INSTALLED AS SHOWN.
- VERTICAL THRUST DETAILS—SEE DWG. 512.
- STRADDLE BLOCK DETAILS—SEE DWG. 511.

* BLOCK TO UNDISTURBED TRENCH WALLS

* * THRUST BLOCKS FOR PIPES LARGER THAN 18" WILL BE INDIVIDUALLY DESIGNED BY THE ENGINEER.

SEE DETAIL 511



LAST REVISION DATE:
DEC 2007

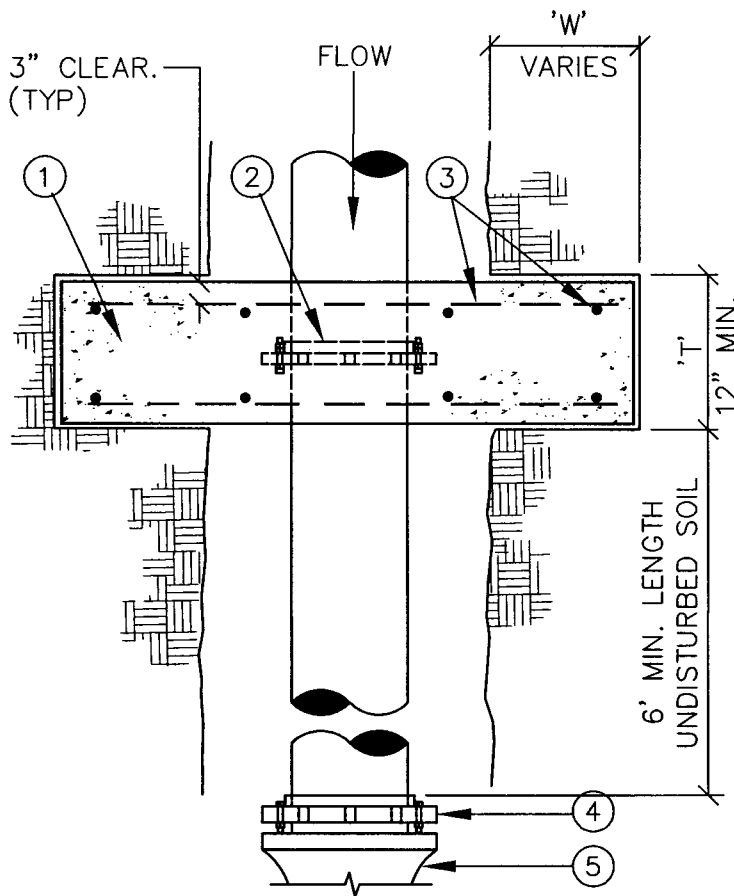
HORIZONTAL THRUST BLOCKING

(NTS)

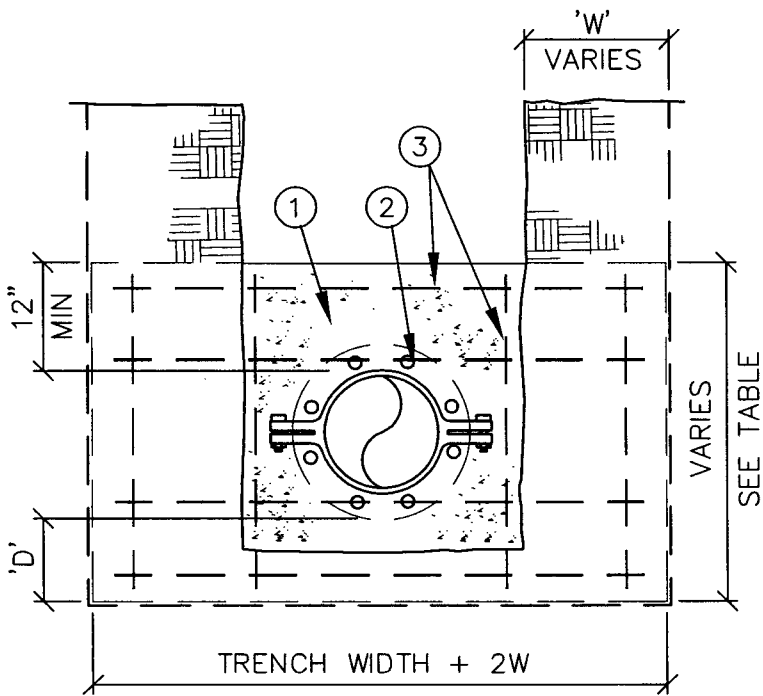
CARLTON, OR

DETAIL NO.

510



TOP VIEW



FRONT VIEW

MATERIALS

- ① CONCRETE STRADDLE BLOCK.
- ② UNI-FLANGE, SERIES 900, CL.125
- ③ #4 REBAR EA. WAY, 12" O.C.
- ④ RETAINER GLAND.
- ⑤ MJ FITTING, VALVE OR BLOWOFF.

PIPE SIZE	'W'	'D'	'T'
6"	12"	8"	12"
8"	16"	10"	12"
10"	20"	12"	12"
12"	24"	18"	18"
>12"	BY ENGINEER		

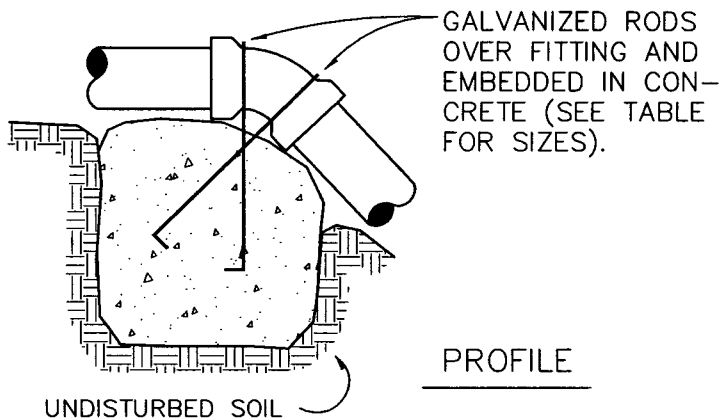
NOTES:

- 1. STRADDLE BLOCKS FOR >12" PIPE SHALL BE DESIGNED INDIVIDUALLY BY THE ENGINEER AND SHALL BE BASED ON THE FOLLOWING:
 - a.) 200 PSI WATER PRESSURE.
 - b.) SOIL BRG. CAPACITY, STEEL SIZE & SPACING BY THE ENGINEER.
- 2. BEARING AREA OF BLOCK SHALL BE AGAINST UNDISTURBED SOIL.
- 3. STRADDLE BLOCK SHALL HAVE A MINIMUM OF 18" COVER.
- 4. CONCRETE SHALL HAVE A MIN. 28 DAY STRENGTH OF 3300 PSI.

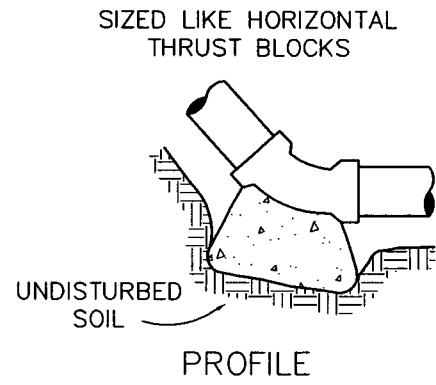
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STRADDLE BLOCK FOR 12" AND SMALLER PIPE	
(NTS)	
CARLTON, OR	DETAIL NO. 511

NOTES:

1. GRAVITY VERTICAL THRUST BLOCKS SHALL BE DESIGNED BY THE ENGINEER.
2. **KEEP CONCRETE CLEAR OF JOINT AND JOINT ACCESSORIES. FITTINGS SHALL BE WRAPPED IN PLASTIC PRIOR TO PLACEMENT OF CONCRETE.**
3. CONCRETE THRUST BLOCKING SHALL BE POURED AGAINST UNDISTURBED EARTH.
4. CONCRETE MIX SHALL HAVE A MIN. 28 DAY STRENGTH OF 3000 P.S.I.
5. THRUST BLOCK VOLUMES FOR VERTICAL BENDS HAVING UPWARD RESULTANT THRUSTS ARE BASED ON TEST PRESSURE OF 150 P.S.I.G. AND THE WEIGHT OF CONCRETE = 4050 LBS./CU.YD.
6. VERTICAL BENDS THAT REQUIRE A THRUST BLOCK VOLUME EXCEEDING 5 CUBIC YARDS REQUIRE SPECIAL BLOCKING DETAILS. SEE PLANS FOR VOLUMES SHOWN INSIDE HEAVY LINE IN TABLE.
7. ALL REBAR SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM-123 (MIN. 3.4 MIL). REBAR SHALL BE BENT BEFORE GALVANIZATION, AND LAST 4" OF BAR SHALL BE BENT 90 DEGREES WITH A 1/2" RADIUS BEND. REBAR SHALL BE TIGHTLY FIT TO RESTRAINED FITTING.
8. FOR HORIZONTAL THRUST BLOCK DETAILS SEE DRAWING NO. 510.



GRAVITY VERTICAL THRUST BLOCK

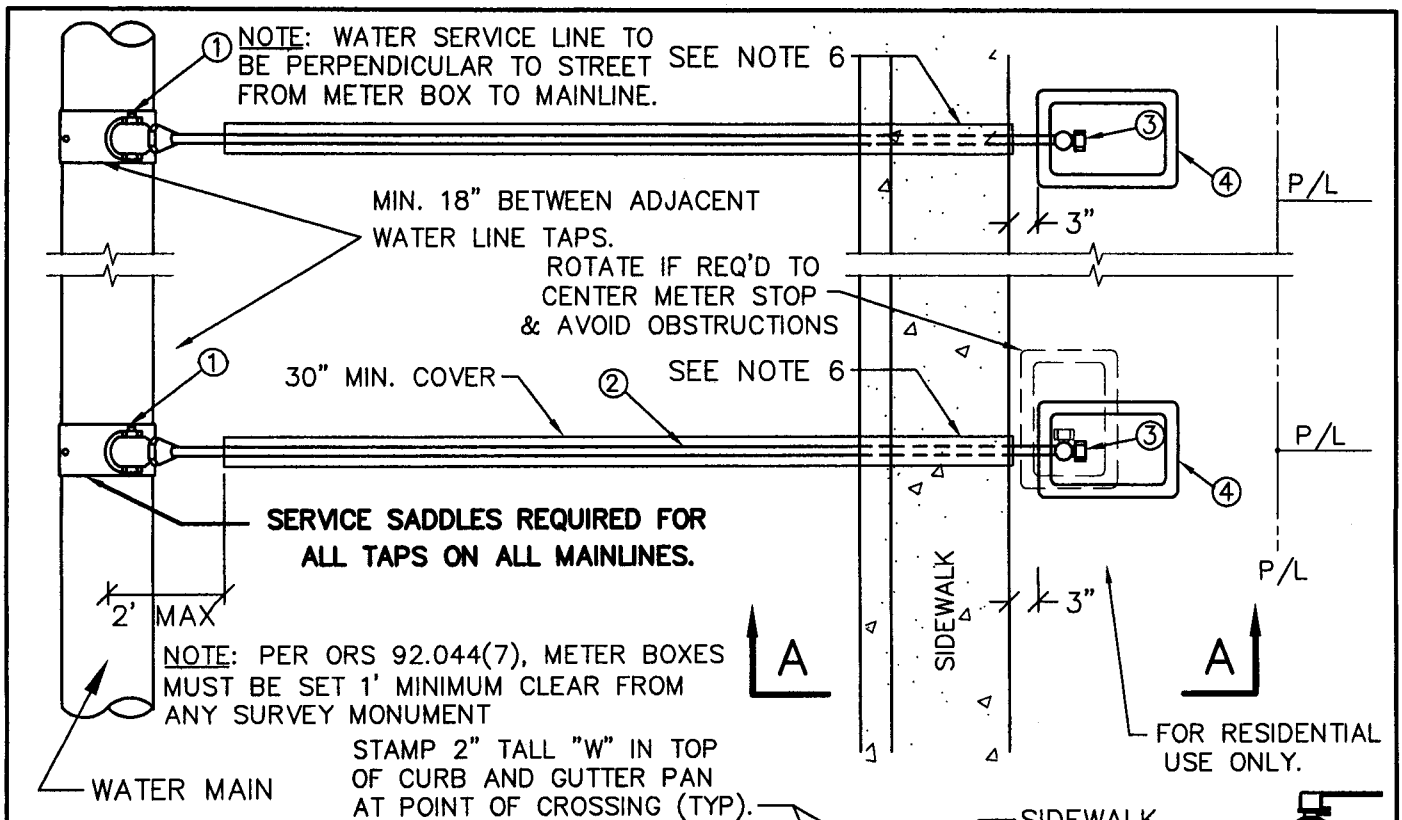


NORMAL VERTICAL THRUST BLOCK

VOLUME OF THRUST BLOCK IN CUBIC YARDS (VERTICAL BENDS)			
FITTING SIZE	BEND ANGLE		
	45°	22 1/2°	11 1/4°
4	1.1	0.4	0.2
6	2.7	1.0	0.4
8	4.0	1.5	0.6
10	6.0	2.3	0.9
12	8.5	3.2	1.3
14	11.5	4.3	1.8
16	14.8	5.6	2.3

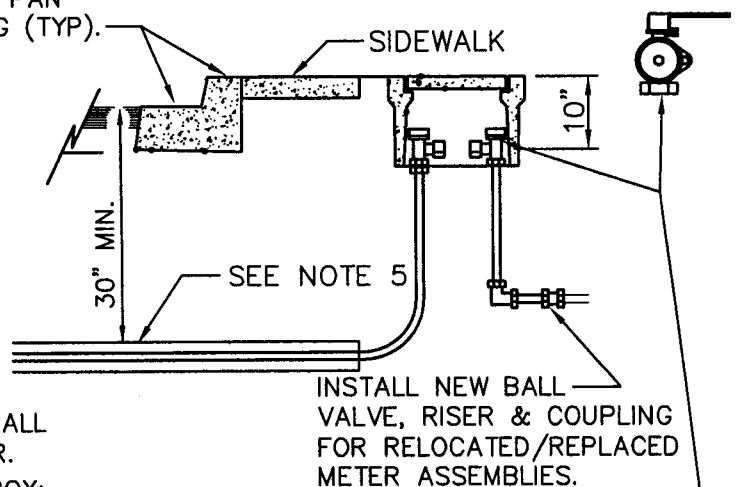
FITTING SIZE	ROD SIZE	EMBEDMENT
12" AND LESS	#6	30"
14" - 16"	#8	36"

LAST REVISION DATE: DEC 2007	
VERTICAL THRUST BLOCKING	
(NTS)	
CARLTON, OR	DETAIL NO. 512



MATERIALS:

- ① BALL STYLE CORPORATION STOP FORD FB-1100. SET AT 30° ANGLE UP FROM HORIZONTAL.
- ② SOFT TEMPER TYPE 'K' COPPER TUBING COMPLYING W/ASTM B-88. SINGLE RESIDENTIAL SERVICE: 1" (TYP)
- ③ BALL STYLE LOCKING ANGLE METER STOP, FORD BA43-444WG OR EQUAL. PROVIDE ALL SERVICES WITH 1" x 3/4" METER ADAPTER.
- ④ ARMORCAST POLYMER CONCRETE METER BOX:
A6001946PCX12 W/A60001866 LID IN TRAFFIC AREAS
A6001946PCX12 W/A60001866R LID ELSEWHERE
PROVIDE ALL METER BOXES WITH PLUGS IN ANY TOUCH-READ KNOCKOUTS.



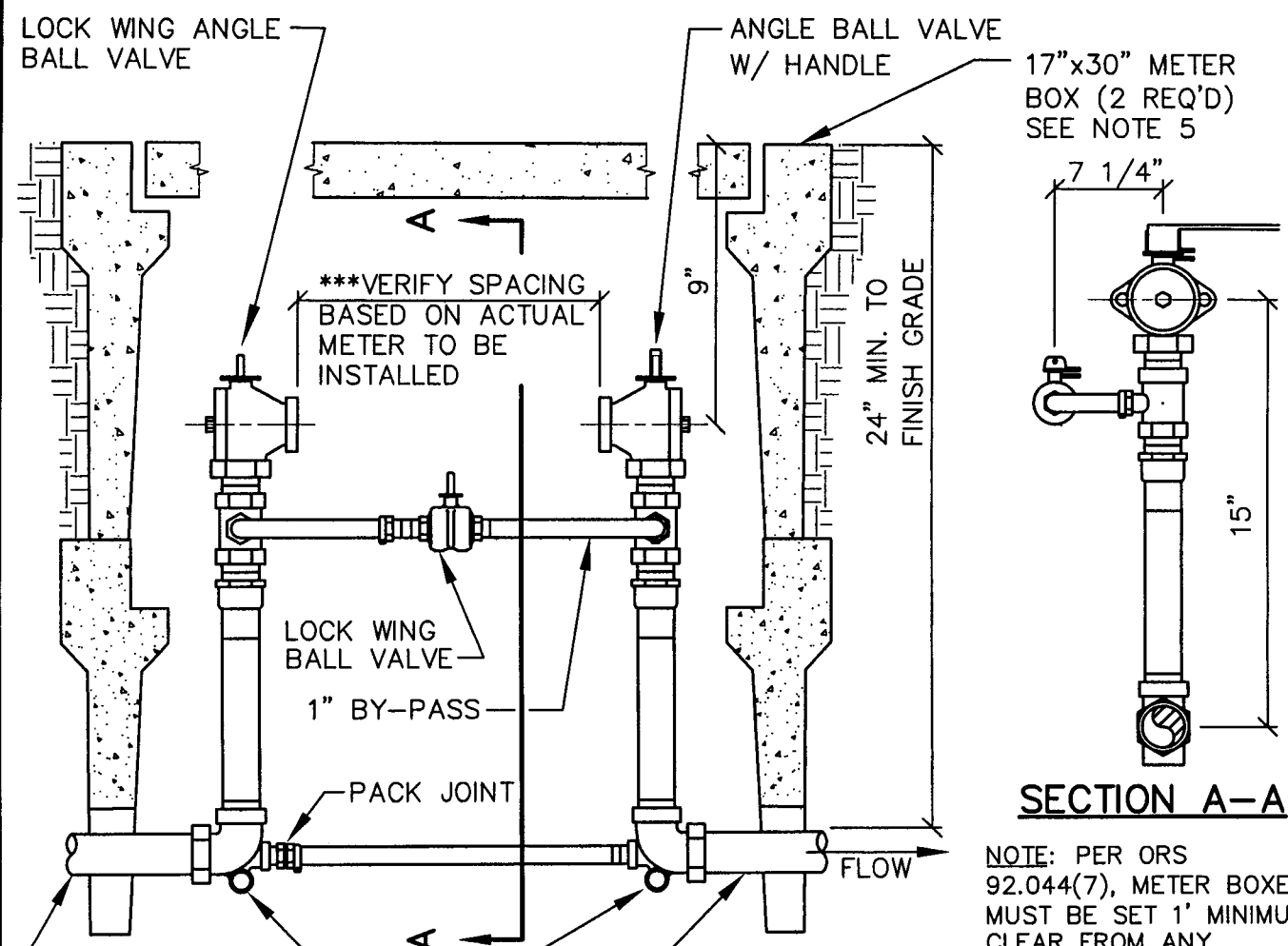
SECTION A-A

NOTES:

- 1. SUBSTITUTES FOR ANY MATERIALS SHOWN SHALL BE APPROVED BY THE CITY ENGINEER.
- 2. ALL PIPE AND BACKFILL ZONES SHALL BE BACKFILLED USING 3/4" MINUS GRANULAR MATERIAL AND COMPACTED TO 92% MAX. DENSITY DETERMINED BY AASHTO T-180.
- 3. SET FRONT OF METER BOX 3-INCHES BEHIND BACK OF SIDEWALK LOCATION FOR CURBLINE WALKS.
- 4. METER BOX SHALL BE CENTERED OVER THE COMPLETED METER ASSEMBLY.
- 5. MIN. SIZE COMMERCIAL SERVICES SHALL BE 1-INCH.
- 6. FAR SIDE COMMERCIAL SERVICES SHALL BE INSTALLED IN A 4" MIN DIA SCHED 40 PVC SLEEVE WHICH BEGINS 2' FROM MAIN AND EXTENDS TO BACK OF FAR SIDE SIDEWALK.

ANGLE BALL VALVE W/HANDLE. INSTALL PRIOR TO WATER METER INSTALLATION.

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TYPICAL 1" WATER SERVICE	
(NTS)	
CARLTON, OR	DETAIL NO. 515



SECTION A-A

NOTE: PER ORS 92.044(7), METER BOXES MUST BE SET 1' MINIMUM CLEAR FROM ANY SURVEY MONUMENT

NOTES:

1. METERS SET TO BE FORD 70 SERIES COPPERSETTER, #VBB76-12HB-11-66 (1 1/2") OR #VBB77-12HB-11-77 (2") WITH RAISED LOCKING BYPASS OR APPROVED EQUAL.
2. SUBSTITUTES FOR ANY MATERIALS SHOWN SHALL BE APPROVED BY THE CITY ENGINEER.
3. ALL PIPE AND BACKFILL ZONES SHALL BE BACKFILLED USING 3/4" MINUS GRANULAR MATERIAL AND COMPACTED TO 92% OPTIMUM DENSITY PER AASHTO T-180.
4. SET FRONT OF METER BOX 3-INCHES BEHIND SIDEWALK (TYPICAL) FOR CURBLINE WALKS. NO METERS ON PRIVATE PROPERTY WITHOUT A RECORDED EASEMENT.
5. METER BOX SHALL BE CENTERED OVER THE COMPLETED METER ASSEMBLY. METER BOX TO BE ARMORCAST -A6001640PCX12 W/A6001975 LID IN NON-TRAFFIC AREAS -TRAFFIC RATED VERSION OF BOX/LID IN TRAFFIC AREAS. PROVIDE WITH PLUGS IN ANY TOUCH-READ KNOCKOUTS.
6. COPPERSETTER, METER BOX, & ALL FITTINGS PROVIDED BY CONTRACTOR. CONTRACTOR TO VERIFY DIMENSIONS & CLEARANCE BASED ON ACTUAL METER TO BE PROVIDED BY THE CITY. WATER METER INSTALLED BY CONTRACTOR UNDER CITY INSPECTION & APPROVAL.
7. SEE DETAIL 517 FOR TAPPING REQUIREMENTS.

***TYPICAL METER LENGTHS (VERIFY)
 1 1/2" COMPOUND - 13" TYP,
 2" COMPOUND - 15 1/4" TYP,
 2" TURBINE - 17" TYP.

LAST REVISION DATE:
 JAN 2010

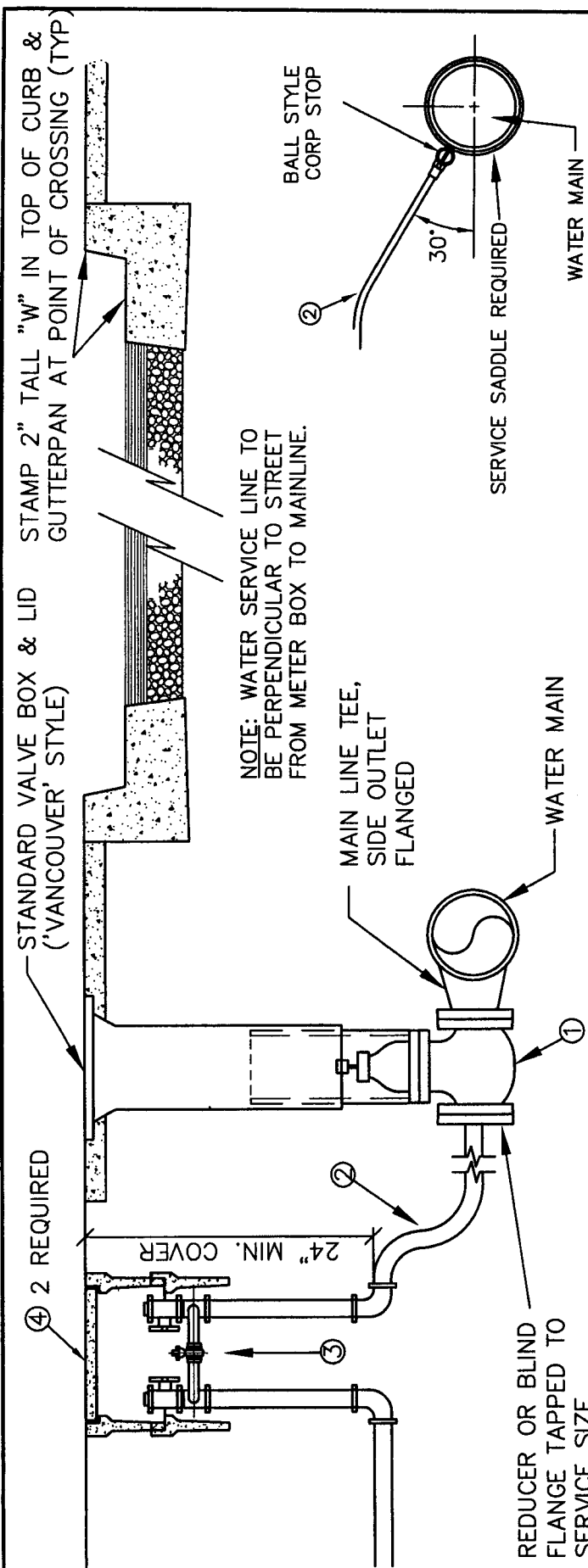
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**1-1/2" AND 2" METER SET
 W/1" HIGH BY-PASS**

(NTS)

CARLTON, OR

DETAIL NO.
516



2" & LARGER SERVICE

NOTE: PER ORS 92.044(7), METER BOXES MUST BE SET 1' MINIMUM CLEAR FROM ANY SURVEY MONUMENT

1-1/2" SERVICE

MATERIALS

- ① FLG X FLG RESILIENT WEDGE GATE VALVE PER AWWA C-509, 4" DIA. OR SERVICE SIZE, WHICHEVER IS LARGER. EPOXY COATED PER AWWA C-550.
- ② HARD COPPER (TYPE K) W/OUT JOINTS OR SCHEDULE 80 PVC PIPE & FITTINGS.
- ③ METER STOP ASSEMBLY W/BYPASS PER PUBLIC WORKS REQUIREMENTS. SEE DETAIL 516 FOR 1-1/2 & 2" SERVICES.
- ④ METER BOX FOR 1-1/2" AND 2" SHALL BE PER DETAIL 516. USE TRAFFIC RATED VERSION OF BOX/LID FOR TRAFFIC AREAS. METER VAULT FOR LARGER SERVICES PER PUBLIC WORKS REQUIREMENTS. PROVIDE PLUGS IN ANY TOUCH-READ SENSOR KNOCKOUTS.

NOTES

- 1. SUBSTITUTES FOR ANY MATERIAL SHOWN SHALL BE APPROVED BY THE CITY ENGINEER.
- 2. ALL PIPE AND STRUCTURE ZONES SHALL BE BACKFILLED USING 3/4" MINUS GRANULAR MATERIAL AND COMPACTED TO 95% MAX DENSITY AS DETERMINED BY ASHTO T-180.
- 3. METER BOX SHALL BE CENTERED OVER THE COMPLETED METER AND FITTING ASSEMBLY.
- 4. CUSTOMER SHALL INSTALL AN APPROVED BACKFLOW PREVENTION DEVICE ON PRIVATE PROPERTY IMMEDIATELY DOWNSTREAM OF WATER METER.

LAST REVISION DATE: JAN 2010		COPYRIGHT 1996 WESTECH ENGINEERING, INC.	
TAPPING REQUIREMENTS, 1-1/2" AND LARGER SERVICE			
		(NTS)	
CARLTON, OR	DETAIL NO.		517

OSHA APPROVED GALVANIZED
LADDER

CAST-IN-PLACE CONCRETE
THRUST COLLAR WITH
RETAINER GLAND CENTERED IN
CONCRETE (TYPICAL BOTH ENDS)

ACCESS DOOR SHALL BE UTILITY VAULT
MODEL 2-332AL (2'10" x 5'6" MIN) OR
APPROVED EQUIV. DOOR SHALL BE H-20
RATED ALUMINUM.

NOTES:

1. METER VAULT & PIPING SHALL CONFORM TO REQUIREMENTS OF PUBLIC/PRIVATE AGENCIES HAVING JURISDICTION.
2. WATER METER & VAULT INSTALLATION SHALL MEET STATE OF OREGON, DEPARTMENT OF HUMAN RESOURCES, HEALTH DIVISION REQUIREMENTS.
3. ALL MATERIALS INCLUDING THE TEMPORARY SPOOL, SHALL BE FURNISHED & INSTALLED BY THE CONTRACTOR.

4. THE CONTRACTOR SHALL INSTALL A TEMPORARY SPACER SPOOL BETWEEN THE GATE VALVES SHOWN. THE TEMPORARY SPOOL SHALL MATCH THE LENGTH OF THE METER, INCLUDING THE STRAINER.
5. ALL M.J. CONNECTIONS SHALL BE ASSEMBLED WITH RETAINER GLANDS.
6. THE BYPASS PIPE SHALL BE THE SAME SIZE AS THE MAIN SERVICE PIPE OR A MIN OF 3", WHICHEVER IS LARGER.

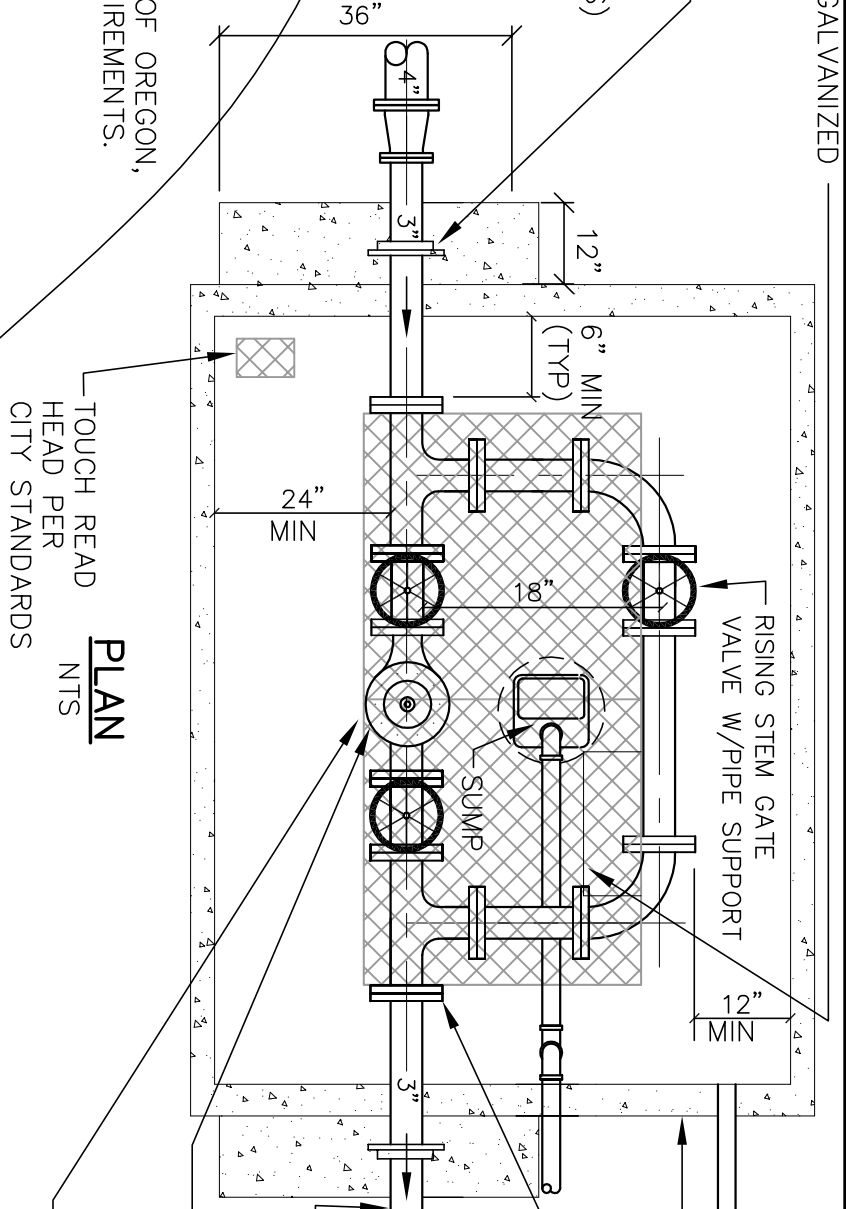
7. METER INSTALLATIONS SHALL INCLUDE TEST PORTS AS AS REQUIRED BY THE PUBLIC/PRIVATE AGENCY HAVING JURISDICTION. UNLESS OTHERWISE REQUIRE BY AGENCY, TEST PORTS SHALL CONSIST OF SADDLES, I.P.X.I.P. CORP STOPS & CAPS. CORP STOPS SHALL BE FORD MODEL FB500.
8. MAIN VALVES SHALL BE NON-RISING-STEM STYLE AND SHALL BE INSTALLED W/HAND WHEELS. HAND WHEELS SHALL MEET ALL REQUIREMENTS OF AWWA C509-94, SEC. 4.11.

9. ALL PIPE OPENINGS SHALL BE SEALED WITH NON-SHRINK WATERTIGHT GROUT.
10. BENDS, CROSSES AND TESS SHALL NOT BE INSTALLED WITHIN 5 FEET OF OUTSIDE OF VAULT WALL.

11. ALL VAULTS SHALL MEET OR EXCEED ASTM C-857. ALL CONC. TO BE 4500 PSI @ 28 DAYS. REBAR TO BE ASTM A-615 GRADE 60.

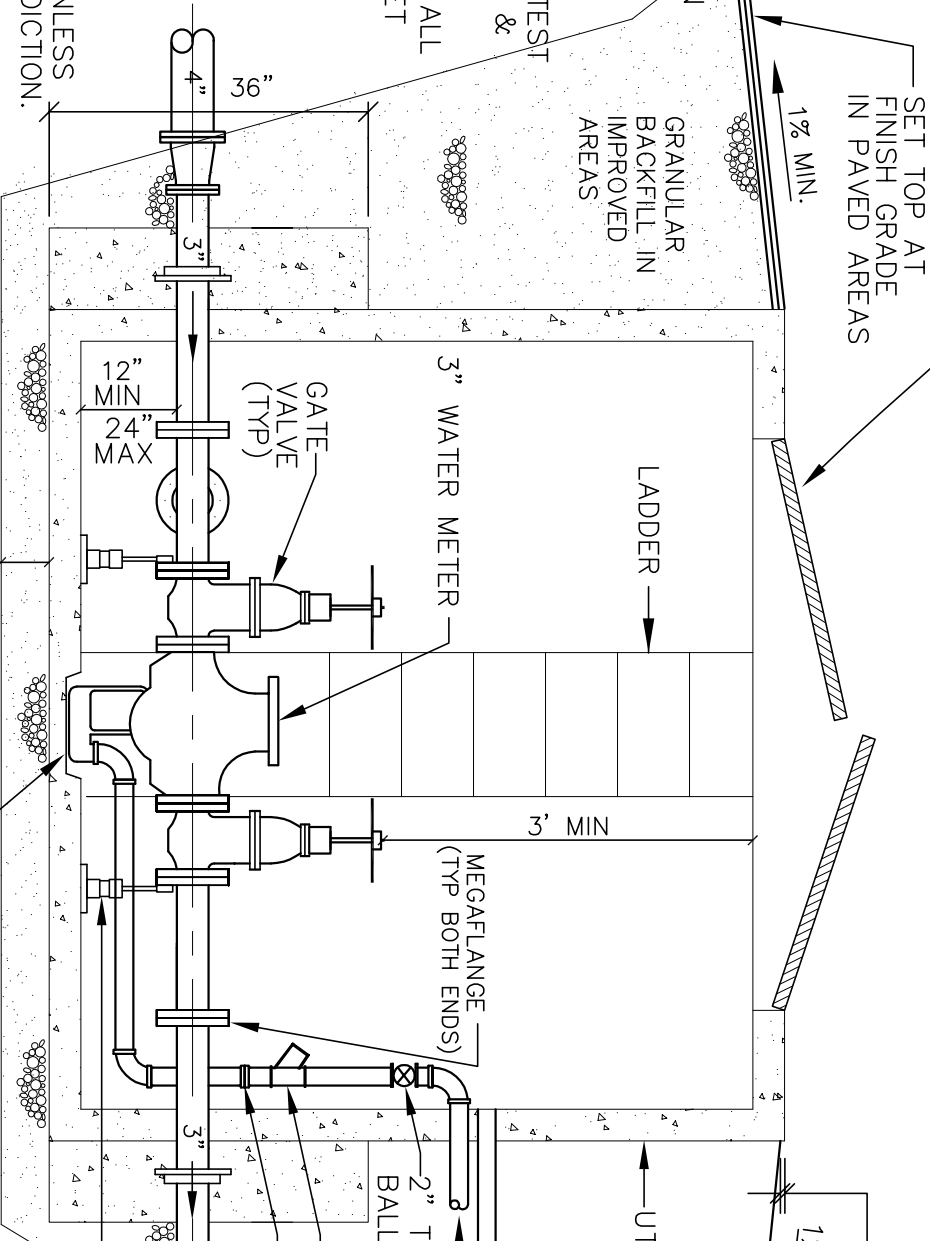
12. SUMP PUMP WITH POWER SUPPLY SHALL BE INSTALLED UNLESS OTHERWISE APPROVED BY ENGINEER OR GOVERNING JURISDICTION.
13. SUMP DISCHARGE SHALL BE PLUMBED TO GRAVITY STORM DRAIN PER UPC REQUIREMENTS.

14. SUMP PUMP POWER SHALL BE INSTALLED IN SCHEDULE 40 RIGID CONDUIT PER NEC REQUIREMENTS.



PLAN
NTS

ELECTRICAL CONDUIT
SUMP PUMP
UTILITY VAULT 687-WA
(6'0" x 8'0" ID) W/H-20
RATED LID, OR EQUIVALENT.
MEGAFLANGE (TYP BOTH ENDS)
3" x 4" REDUCER (MjxMj
W/RETAINER GLANDS)
(TYP BOTH ENDS)
PLAIN END SPOOL (TYP)
WATER METER AS SPECIFIED BY CITY W/
METER REGISTER AND REMOTE TOUCH READ
HEAD PER CITY STANDARD (METER PROVIDED
BY CITY, INSTALLED BY CONTRACTOR UNDER
CITY INSPECTION & APPROVAL)
ACCESS COVER SHALL BE LOCATED
OVER THE METER REGISTER



SECTION
NTS

SET TOP AT
FINISH GRADE
IN PAVED AREAS
1% MIN.
GRANULAR
BACKFILL IN
IMPROVED
AREAS
LADDER
3" WATER METER
GATE
VALVE
(TYP)
MEGAFLANGE
(TYP BOTH ENDS)
UTILITY VAULT
1% MIN.
SET TOP 1" MIN. ABOVE FG.
OUTSIDE PAVED AREAS
ELECTRICAL CONDUIT FOR SUMP
PUMP POWER. SEE ELECTRICAL
PLANS. PROVIDE MIN. 30" COVER.
2" SCH. 40 PVC SUMP PUMP DISCHARGE
LINE. PLUMB TO GRAVITY STORM DRAIN.
PROVIDE 30" MINIMUM COVER.
2" TRUE UNION
BALL VALVE
2" BALL CHECK VALVE
STANDON MODEL S89
FLANGE SUPPORT OR
APPROVED EQUAL
(TYP).
MIN 5 GPM SUMP PUMP WITH POWER
SUPPLY. CONTRACTOR TO COORDINATE
WITH BUILDING CONTRACTOR TO CONNECT
SUMP PUMP TO BUILDING POWER.

LAST REVISION DATE:	JO #
NOV 2009	STANDARD

**3" DOMESTIC
WATER METER**
(NTS)

DAYTON, OR	DETAIL NO.
	523

PAD MOUNTED FIBERGLASS INSULATED ENCLOSURE W/HEATER, HOT BOX MODEL AS SHOWN ON TABLE (OR APPROVED EQUIVALENT). ANCHOR ENCLOSURE TO CONCRETE PAD PER MANUFACTURER'S REQUIREMENTS.

RPBA DIAMETER	HOT BOX MODEL
1"	HB1
1½"	HB1
2"	HB1.5

NOTE: VERIFY HB SIZE FOR OTHER MODEL RPBA DEVICES.

ELECTRICAL RECEPTICAL FOR HEAT TAPE (GFI). PROVIDE HEAT TAPE OR ENCLOSURE HEATER FOR ALL ABOVE GRADE PIPING

REDUCED PRESSURE BACKFLOW ASSEMBLY (RPBA) MFR'D BY FEBCO, MODEL 825YA (OR APPROVED EQUAL)

DO NOT OBSTRUCT ENCLOSURE OPENINGS (TYP)

4" CONCRETE PAD

SURFACE PER PLAN SLOPE TO DRAIN

MIN. 2" COMPACTED GRANULAR BASEROCK
COMPACTED SUBGRADE

12" MIN. TYP (ALL WAYS)

SCH 80 PVC PIPE, TYPICAL BOTH VERTICAL RISERS

12" MIN

30" TYP

3" PIPE SLEEVE FIELD LOCATE (TYP 2)

ELECTRICAL CONDUIT TO POWER SOURCE. COORDINATE AS REQ'D TO PROVIDE 120V POWER.

SCHEDULE 40 PVC FROM WATER SERVICE, SIZE AS SHOWN ON PLANS

SCHEDULE 40 PVC TO BUILDING. SIZE AS SHOWN ON PLANS

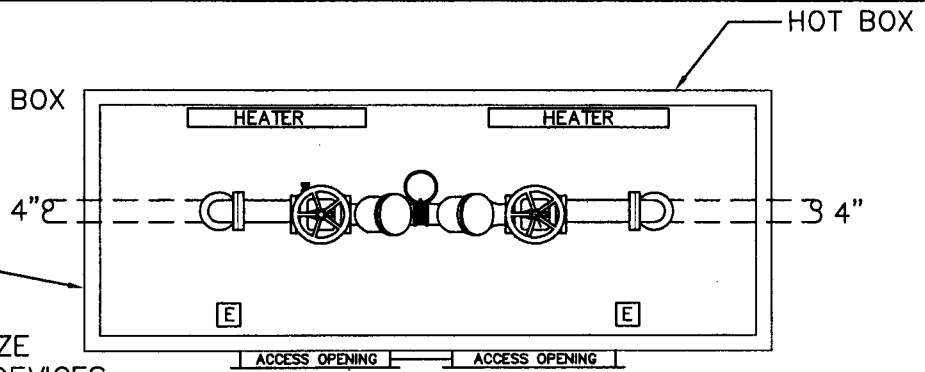
NOTES:

1. RPBA- REDUCED PRESSURE BACKFLOW ASSEMBLY.
2. RPBA & VAULT INSTALLATION SHALL MEET STATE OF OREGON, DEPARTMENT OF HUMAN RESOURCES, HEALTH DIVISION REQUIREMENTS.
3. CONTRACTOR SHALL HAVE RPBA TESTED AND CERTIFIED PRIOR TO APPROVAL BY THE CITY.
4. RPBA & VAULT SHALL CONFORM TO REQUIREMENTS OF PUBLIC/PRIVATE AGENCIES HAVING JURISDICTION.
5. VAULTS SHALL HAVE A MINIMUM OF 3' CLEARANCE FROM ALL OTHER VAULTS OR STRUCTURES.
6. VERIFY ENCLOSURE DIMENSIONS ARE ADEQUATE FOR CLEARANCE BASED ON HEIGHT OF REDUCED PRESSURE ASSEMBLY.
7. ENCLOSURE SHALL BE CENTERED OVER THE COMPLETED REDUCED PRESSURE BACKFLOW ASSEMBLY.
8. POWER SHALL BE INSTALLED IN SCHEDULE 40 RIGID CONDUIT PER NEC REQUIREMENTS.
9. ALL CONCRETE SHALL HAVE 3,300 PSI COMPRESSIVE STRENGTH @ 28 DAYS.
10. HOT BOX DRAINAGE OPENINGS SHALL NOT BE OBSTRUCTED BY GRADING OR PLANTINGS.
11. RPBA SHALL BE INSTALLED A MIN. OF 12 INCHES ABOVE THE 100-YEAR FLOOD ELEVATION AS DETERMINED BY FEMA.

LAST REVISION DATE: JAN 2010	JO # STANDARD
2" AND SMALLER REDUCED PRESSURE BACKFLOW ASSEMBLY (NTS)	
CARLTON, OR	DETAIL NO. 541

MODEL NO. HB4E AS
 MANUFACTURED BY HOT BOX
 (1-800-736-0238)
 ANCHOR ENCLOSURE
 TO CONCRETE PAD
 PER MANUFACTURER'S
 REQUIREMENTS.

NOTE: VERIFY VAULT SIZE
 FOR OTHER MODEL BF DEVICES.



PLAN
 NTS

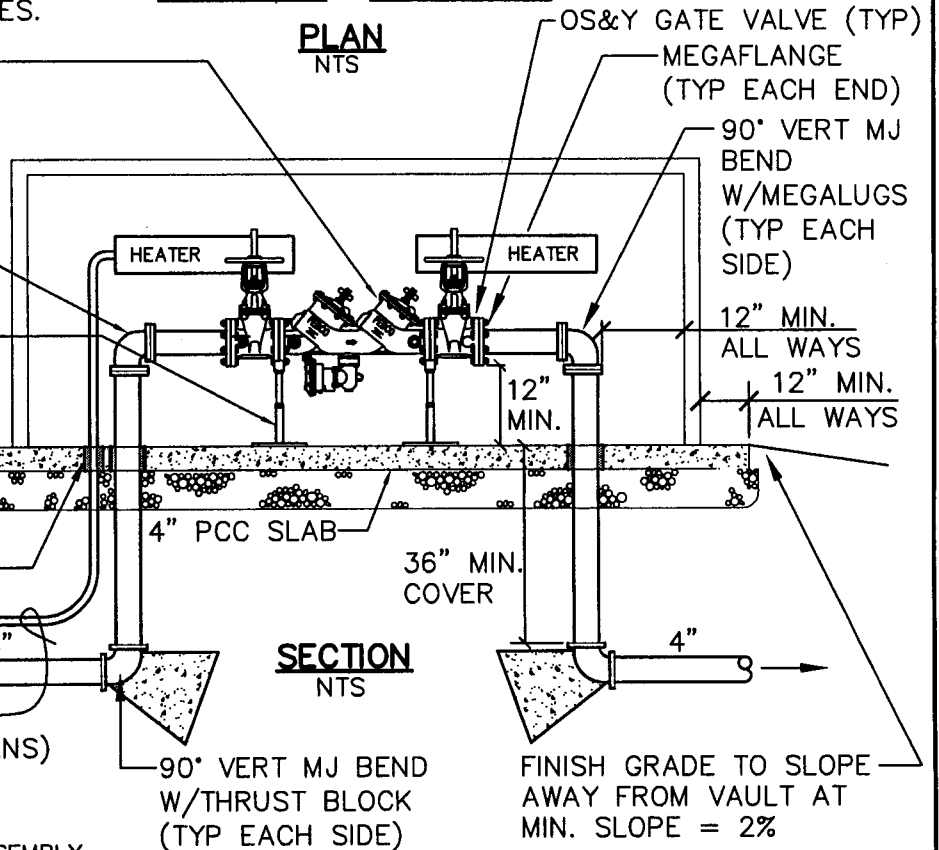
4" FEBCO 860 REDUCED
 PRESSURE ASSEMBLY (OR
 APPROVED EQUAL) WITH 2
 OS&Y GATE VALVES (TYP)
 90° VERT MJ BEND
 W/MEGALUGS
 (TYP EACH SIDE)

STANDON MODEL S89
 FLANGE SUPPORT OR
 APPROVED EQUAL (TYP).

6" MIN. COMPACTED
 GRANULAR BASEROCK

PROVIDE EXPANSION
 JOINT FILLER AT PIPE
 PENETRATIONS (TYP)

ELECTRICAL CONDUIT
 (SEE ELECTRICAL PLANS)

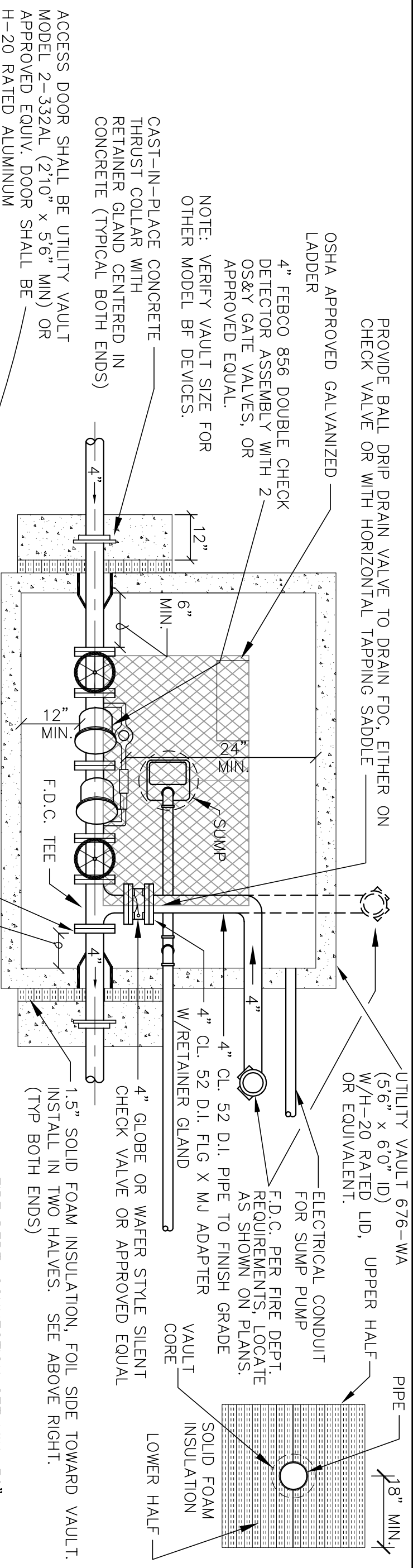


SECTION
 NTS

NOTES:

1. RPA- REDUCED PRESSURE ASSEMBLY
2. RPA & VAULT INSTALLATION SHALL MEET STATE OF OREGON, DEPARTMENT OF HUMAN RESOURCES, HEALTH DIVISION REQUIREMENTS.
3. CONTRACTOR SHALL HAVE RPA TESTED AND CERTIFIED PRIOR TO APPROVAL BY THE CITY.
4. RPA & VAULT SHALL CONFORM TO REQUIREMENTS OF PUBLIC/PRIVATE AGENCIES HAVING JURISDICTION.
5. VAULTS SHALL HAVE A MINIMUM OF 3' CLEARANCE FROM ALL OTHER VAULTS OR STRUCTURES.
6. VERIFY ENCLOSURE DIMENSIONS ARE ADEQUATE FOR CLEARANCE BASED ON HEIGHT OF REDUCED PRESSURE ASSEMBLY.
7. ENCLOSURE SHALL BE CENTERED OVER THE COMPLETED REDUCED PRESSURE ASSEMBLY.
8. POWER SHALL BE INSTALLED IN SCHEDULE 40 RIGID CONDUIT PER NEC REQUIREMENTS.
9. 'E' INDICATES THE ELECTRICAL RECEPTACLE. IT SHALL BE MOUNTED A MIN. OF 18" ABOVE THE SLAB.
10. ALL CONCRETE SHALL HAVE 3,300 PSI COMPRESSIVE STRENGTH @ 28 DAYS.
11. HOT BOX DRAINAGE OPENINGS SHALL NOT BE OBSTRUCTED BY GRADING OR PLANTINGS.
12. RPA SHALL BE INSTALLED A MIN. OF 12 INCHES ABOVE THE 100-YEAR FLOOD ELEVATION AS DETERMINED BY FEMA.

LAST REVISION DATE: JAN 2010	JO # STANDARD
4" REDUCED PRESSURE ASSEMBLY	
(NTS)	
CARLTON, OR	DETAIL NO. 544



NOTE: VERIFY VAULT SIZE FOR OTHER MODEL BF DEVICES.

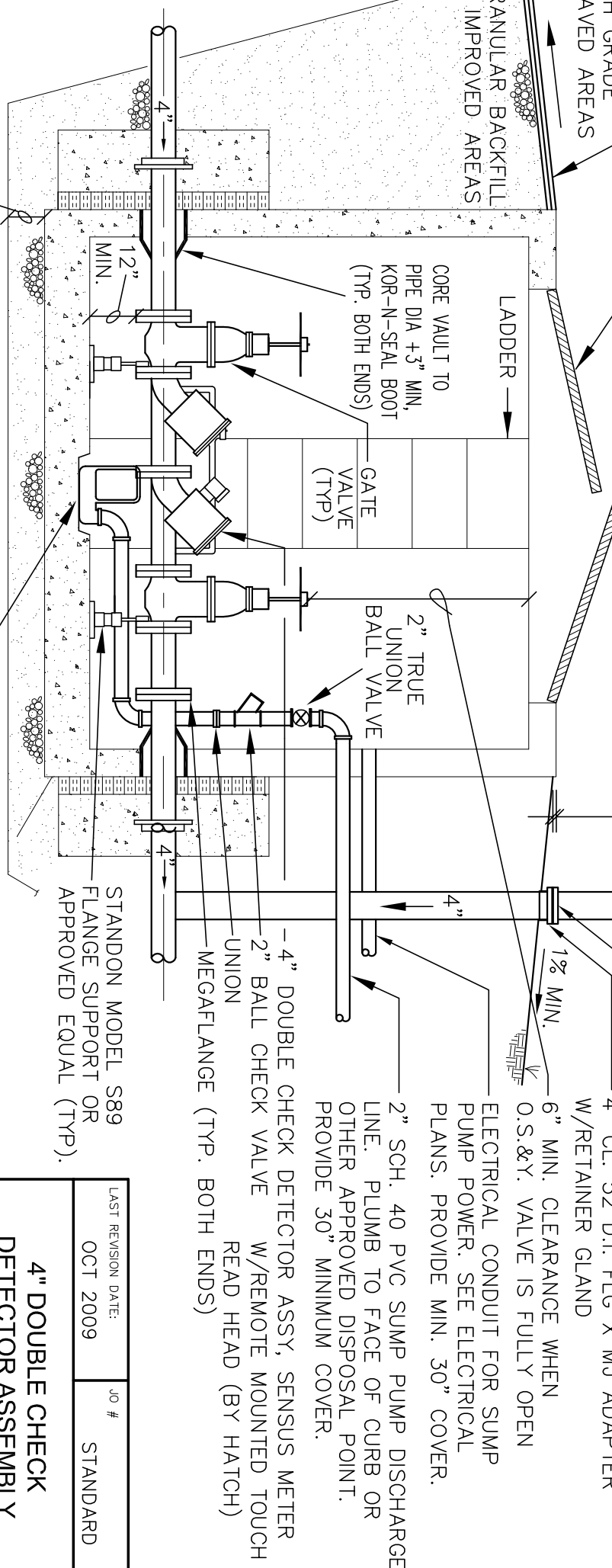
ACCESS DOOR SHALL BE UTILITY VAULT MODEL 2-332AL (2'10" x 5'6" MIN) OR APPROVED EQUIV. DOOR SHALL BE H-20 RATED ALUMINUM

PLAN

NTS

SET TOP AT FINISH GRADE OR 1" MAX. ABOVE FG. OUTSIDE PAVED AREAS

1. DCDA- DOUBLE CHECK DETECTOR ASSEMBLY FDC- FIRE DEPARTMENT CONNECTION.
2. DCDA SHALL CONFORM TO REQUIREMENTS OF PUBLIC/PRIVATE AGENCIES HAVING JURISDICTION.
3. DCDA & VAULT INSTALLATION SHALL MEET STATE OF OREGON, DEPARTMENT OF HUMAN RESOURCES, HEALTH DIVISION REQUIREMENTS.
4. CONTRACTOR SHALL HAVE DCDA TESTED AND CERTIFIED PRIOR TO FINAL APPROVAL BY CITY.
5. FDC SHALL NOT EXIT THROUGH THE TOP OF THE VAULT.
6. ALL PIPE OPENINGS SHALL BE SEALED WITH NON-SHRINK WATERTIGHT GROUT.
7. BENDS, CROSSES AND TEES SHALL NOT BE INSTALLED WITHIN 5 FEET OF THE OUTSIDE VAULT WALL.
8. ALL VAULTS SHALL MEET OR EXCEED ASTM C-857. ALL VAULT CONCRETE TO BE 4500 PSI @ 28 DAYS. REBAR TO BE ASTM A-615 GRADE 60.
9. SUMP PUMP WITH POWER SUPPLY SHALL BE INSTALLED UNLESS OTHERWISE APPROVED BY PUBLIC WORKS.
10. SUMP DISCHARGE SHALL BE PLUMBED TO FACE OF STREET CURB OR OTHER DISPOSAL POINT APPROVED BY LOCAL JURISDICTION (SEE OAR 333-061-0071.3.f).
11. SUMP PUMP POWER SHALL BE INSTALLED IN SCHEDULE 40 RIGID CONDUIT PER NEC REQUIREMENTS.
12. THRUST COLLAR CONCRETE SHALL BE 3300 PSI @ 28 DAYS.
13. UNLESS WAIVED BY PUBLIC WORKS, PROVIDE REMOTE READER FOR DETECTOR LOOP PER PUBLIC WORKS REQUIREMENTS.



SECTION

NTS

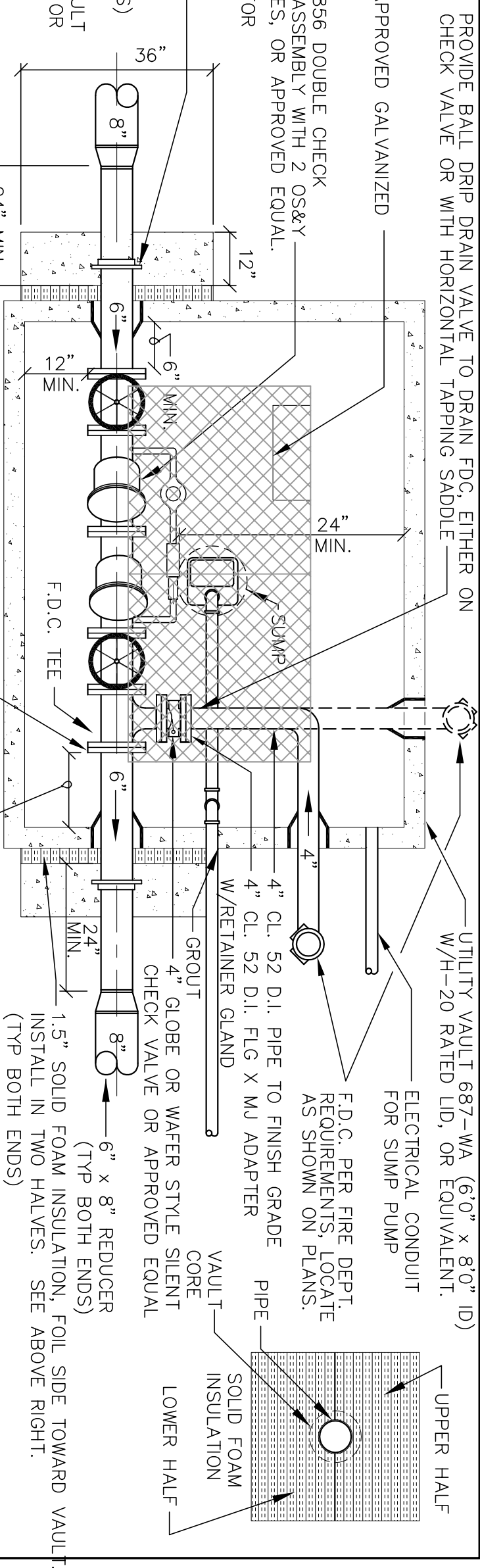
LAST REVISION DATE: OCT 2009

JO # STANDARD

4" DOUBLE CHECK DETECTOR ASSEMBLY W/FDC

(NTS)

DAYTON, OR DETAIL NO. 554



NOTE: VERIFY VAULT SIZE FOR OTHER MODEL BF DEVICES.

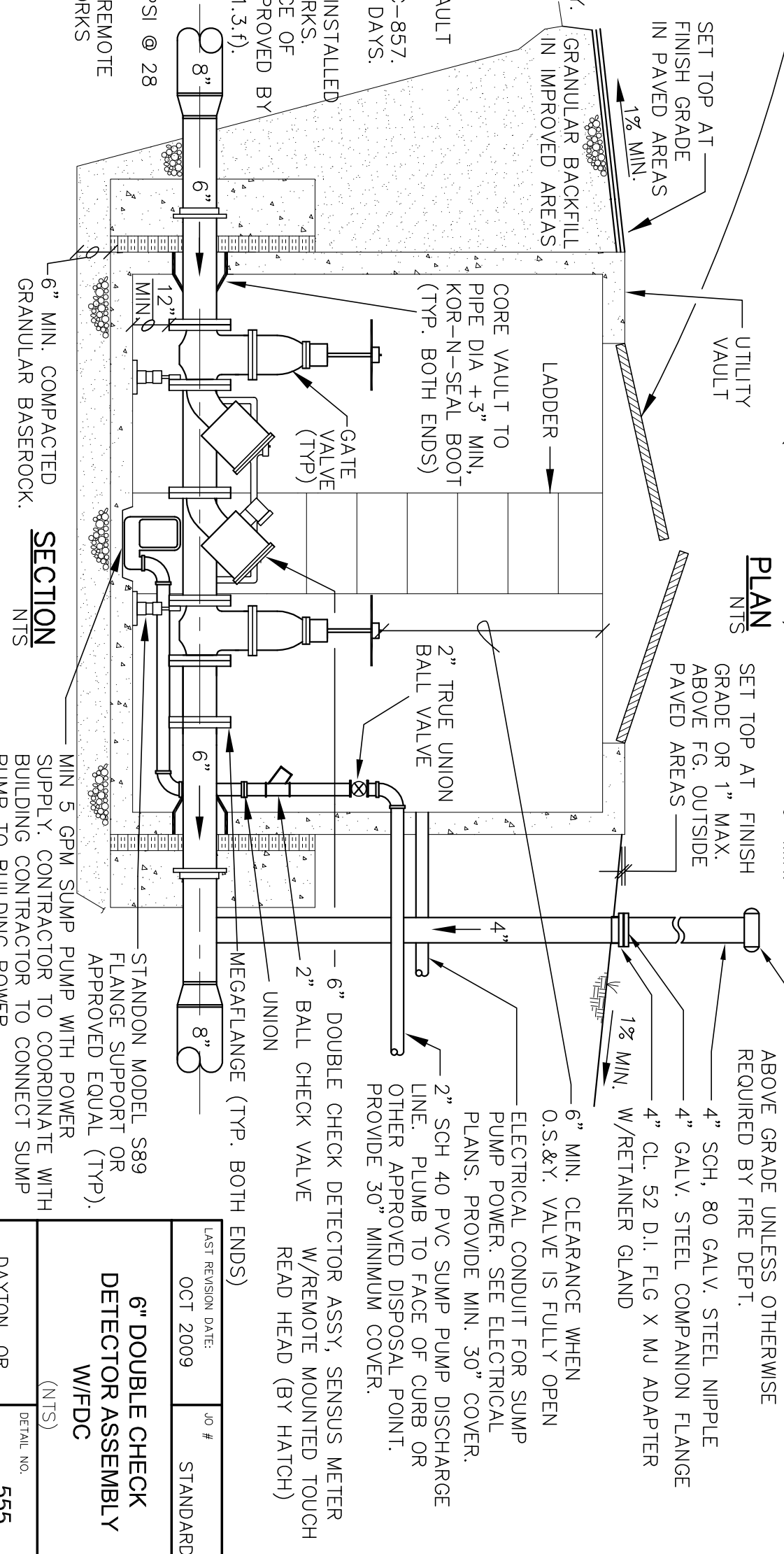
6" FEBCO 856 DOUBLE CHECK DETECTOR ASSEMBLY WITH 2 OS&Y GATE VALVES, OR APPROVED EQUAL.

CAST-IN-PLACE CONCRETE THRUST COLLAR WITH RETAINER GLAND CENTERED IN CONCRETE (TYPICAL BOTH ENDS)

ACCESS DOOR SHALL BE UTILITY VAULT MODEL 2-332AL (2'10" x 5'6" MIN) OR APPROVED EQUIV. DOOR SHALL BE H-20 RATED ALUMINUM.

NOTES:

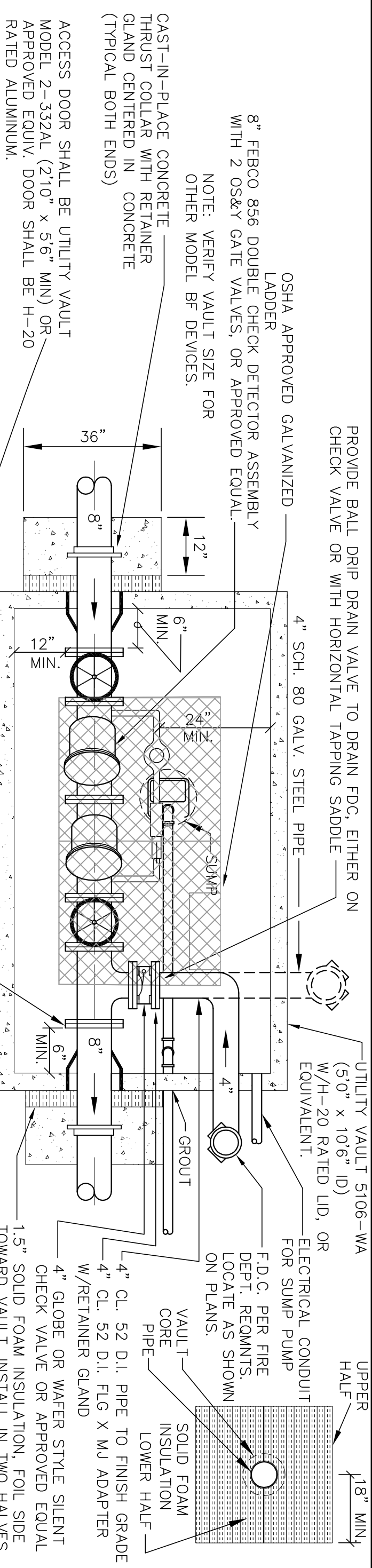
1. DCDA - DOUBLE CHECK DETECTOR ASSEMBLY
2. DCDA - FIRE DEPARTMENT CONNECTION.
3. DCDA SHALL CONFORM TO REQUIREMENTS OF PUBLIC/Private AGENCIES HAVING JURISDICTION.
4. DCDA & VAULT INSTALLATION SHALL MEET STATE OF OREGON, DEPARTMENT OF HUMAN RESOURCES, HEALTH DIVISION REQUIREMENTS. CONTRACTOR SHALL HAVE DCDA TESTED AND CERTIFIED PRIOR TO FINAL APPROVAL BY CITY.
5. FDC SHALL NOT EXIT THROUGH THE TOP OF THE VAULT.
6. ALL PIPE OPENINGS SHALL BE SEALED WITH NON-SHRINK WATER-TIGHT GROUT.
7. BENDS, CROSSES AND TEES SHALL NOT BE INSTALLED WITHIN 5 FEET OF THE OUTSIDE VAULT WALL.
8. ALL VAULTS SHALL MEET OR EXCEED ASTM C-857. ALL VAULT CONCRETE TO BE 4500 PSI @ 28 DAYS. REBAR TO BE ASTM A-615 GRADE 60.
9. SUMP PUMP WITH POWER SUPPLY SHALL BE INSTALLED UNLESS OTHERWISE APPROVED BY PUBLIC WORKS.
10. SUMP DISCHARGE SHALL BE PLUMBED TO FACE OF STREET CURB OR OTHER DISPOSAL POINT APPROVED BY LOCAL JURISDICTION (SEE OAR 333-061-0071.3.f).
11. SUMP PUMP POWER SHALL BE INSTALLED IN SCHEDULE 40 RIGID CONDUIT PER NEC REQUIREMENTS.
12. THRUST COLLAR CONCRETE SHALL BE 3300 PSI @ 28 DAYS.
13. UNLESS WAIVED BY PUBLIC WORKS, PROVIDE REMOTE READER FOR DETECTOR LOOP PER PUBLIC WORKS REQUIREMENTS.



SECTION
NTS

MIN 5 GPM SUMP PUMP WITH POWER SUPPLY. CONTRACTOR TO COORDINATE WITH BUILDING CONTRACTOR TO CONNECT SUMP PUMP TO BUILDING POWER.

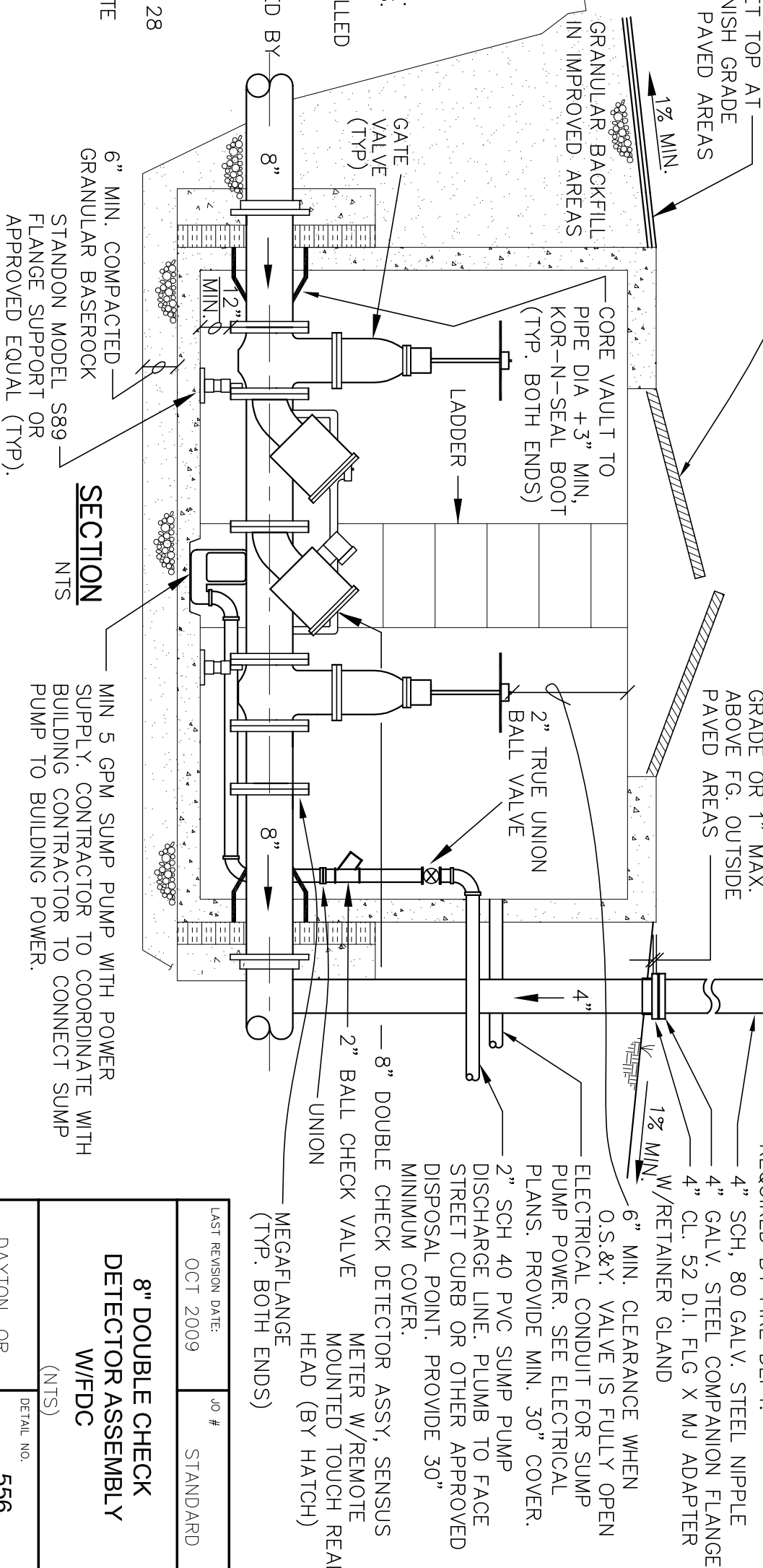
LAST REVISION DATE: OCT 2009	JO # STANDARD
6" DOUBLE CHECK DETECTOR ASSEMBLY W/FDC	
(NTS)	
DAYTON, OR	DETAIL NO. 555



NOTES:

1. DCDA - DOUBLE CHECK DETECTOR ASSEMBLY FDC - FIRE DEPARTMENT CONNECTION.
2. DCDA SHALL CONFORM TO REQUIREMENTS OF PUBLIC/Private AGENCIES HAVING JURISDICTION.
3. DCDA & VAULT INSTALLATION SHALL MEET STATE OF OREGON, DEPARTMENT OF HUMAN RESOURCES, HEALTH DIVISION REQUIREMENTS. CONTRACTOR SHALL HAVE DCDA TESTED AND CERTIFIED PRIOR TO FINAL APPROVAL BY CITY.
4. FDC SHALL NOT EXIT THROUGH THE TOP OF THE VAULT.
5. ALL PIPE OPENINGS SHALL BE SEALED WITH NON-SHRINK WATERTIGHT GROUT.
6. BENDS, CROSSES AND TEES SHALL NOT BE INSTALLED WITHIN 5 FEET OF THE OUTSIDE VAULT WALL.
7. ALL VAULTS SHALL MEET OR EXCEED ASTM C-857. ALL VAULT CONCRETE TO BE 4500 PSI @ 28 DAYS. REBAR TO BE ASTM A-615 GRADE 60.
8. SUMP PUMP WITH POWER SUPPLY SHALL BE INSTALLED UNLESS OTHERWISE APPROVED BY PUBLIC WORKS.
9. SUMP DISCHARGE SHALL BE PLUMBED TO FACE OF STREET CURB OR OTHER DISPOSAL POINT APPROVED BY LOCAL JURISDICTION (SEE OAR 333-061-0071.3.f).
10. SUMP PUMP POWER SHALL BE INSTALLED IN SCHEDULE 40 RIGID CONDUIT PER NEC REQUIREMENTS.
11. THRUST COLLAR CONCRETE SHALL BE 3300 PSI @ 28 DAYS.
12. UNLESS WAIVED BY PUBLIC WORKS, PROVIDE REMOTE READER FOR DETECTOR LOOP PER PUBLIC WORKS REQUIREMENTS.

PLAN
NTS



**8" DOUBLE CHECK
DETECTOR ASSEMBLY
W/FDC**
(NTS)

LAST REVISION DATE: OCT 2009	JO # STANDARD
8" DOUBLE CHECK DETECTOR ASSEMBLY W/FDC (NTS)	
DAYTON, OR	DETAIL NO. 556

WATERLINE PRESSURE TEST REPORT

Project Location:	Project Name:	Date:
Inspector: (Print)	Waterline to be tested. From Station:	To Station:
Verify that all in-line valves, including hydrant mainline valves, are open? Yes / No		
Verify that all corp stops are open? Yes / No		
Verify that pressure gauge is mounted at high point of line to be tested? Yes / No If no, correct for elevation difference (ie. add 0.433 psi per foot elevation difference).		
System Static Pressure (psi):	Starting Pressure (psi): (greater of 150 psi or 1.5 times static)	Ending Pressure (psi):
Test Length: (2 hours minimum)	Starting Time:	Ending Time:
Volume Required to Reach Initial Test Pressure (gal):	Allowable Leakage (gal): (2 times table value below)	Measured Leakage (gal):
TEST RESULTS: Pass / Fail		

ALLOWABLE LEAKAGE PER 1,000 FEET OF PIPELINE - gph

Test Pressure <i>psi</i>	NOMINAL PIPE DIAMETER - in.									
	3	4	6	8	10	12	14	16	18	20
200	0.32	0.43	0.64	0.85	1.06	1.28	1.48	1.70	1.91	2.12
175	0.30	0.40	0.59	0.80	0.99	1.19	1.39	1.59	1.79	1.98
150	0.28	0.37	0.55	0.74	0.92	1.10	1.29	1.47	1.66	1.84

If the pipeline under test contains various diameters, the allowable leakage shall be the sum of the allowable leakage for each size. No additional leakage allowance will be given for fire hydrant assemblies or valves.

Allowable leakage based on : $L = SD(P)^{1/2} / 133,200$

Where:

L = allowable leakage, in gallons per hour

S = length of pipe tested, in feet

D = nominal diameter of the pipe, in inches

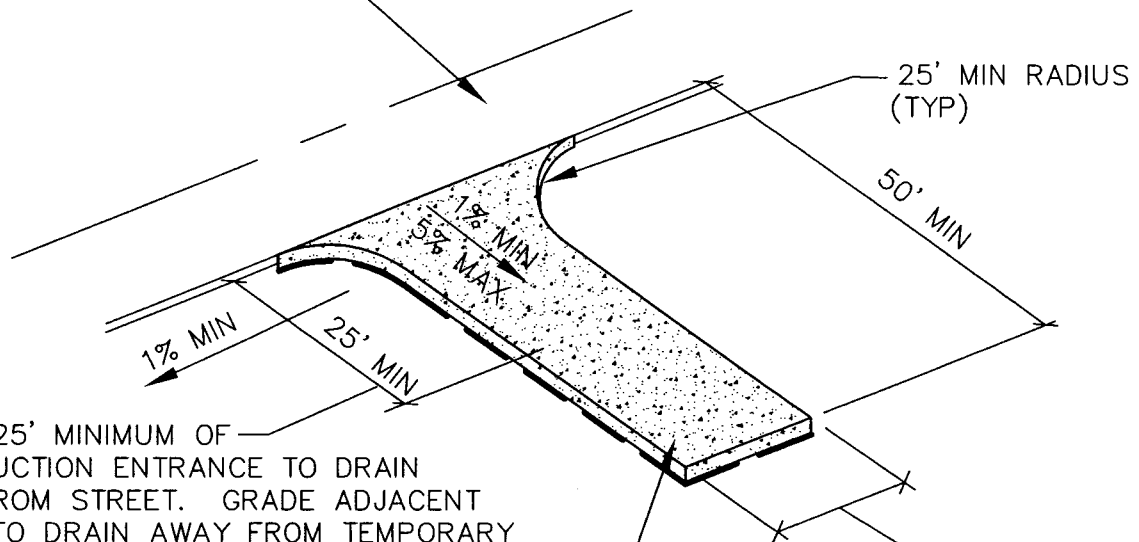
P = test pressure during the leakage test, in psig

Regardless of leakage, maximum pressure drop during test period shall not exceed 5 psi/hour.

TEST PROCEDURE

1. Apply hydrostatic pressure by pumping water from an auxiliary supply basin. Accurately determine the amount of water required to reach the initial test pressure by refilling the supply basin with a calibrated container following pressurization of pipeline.
2. Monitor test pressure for 2 hour period.
3. At the completion of the test period, re-pressurize the pipeline by pumping water from the auxiliary supply basin. Accurately determine the amount of water required to reach the test pressure by refilling the supply basin with a calibrated container following pressurization of pipeline. If the measured leakage is less than the allowable leakage, the test is successful.

EXIST. PUBLIC ROAD OR APPROVED ACCESS POINT



GRADE 25' MINIMUM OF CONSTRUCTION ENTRANCE TO DRAIN AWAY FROM STREET. GRADE ADJACENT AREAS TO DRAIN AWAY FROM TEMPORARY CONSTRUCTION ENTRANCE.

PLACE 3"-0 GRANULAR MATERIAL OVER 8-OUNCE NON-WOVEN GEOTEXTILE FABRIC AS FOLLOWS:

DRY WEATHER ACCESS

14-INCH MIN. DEPTH OVER COMPACTED SUBGRADE & FABRIC

WET WEATHER ACCESS

24-INCH MIN. DEPTH OVER UNDISTURBED SUBGRADE & FABRIC

FULL WIDTH OF PROPOSED STREET OR ACCESS (25' MINIMUM)

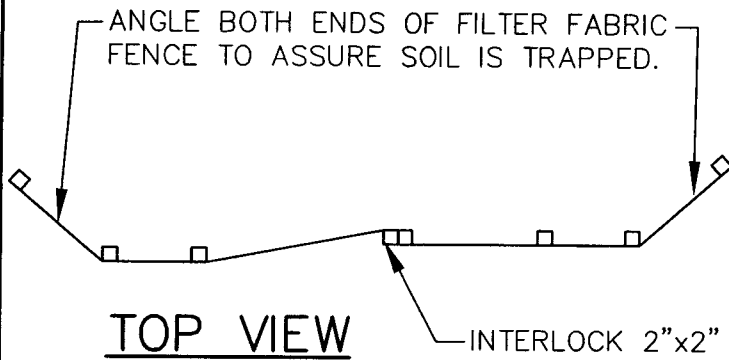
CONSTRUCTION NOTES:

1. THE AREA OF THE CONSTRUCTION ENTRANCE SHALL BE STRIPPED OF ALL TOPSOIL, VEGETATION, ROOTS, AND OTHER NON-COMPACTABLE MATERIAL.
2. SUBGRADE SHALL BE COMPACTED AND PROOFROLLED PRIOR TO PLACEMENT OF GRANULAR MATERIAL. FAILURE TO PASS PROOFROLL WILL REQUIRE USE OF WET WEATHER SECTION.
3. FAILURE OR PUMPING OF THE DRY WEATHER SECTION WILL REQUIRE REMOVAL OF THE GRANULAR MATERIAL AND INSTALLATION OF THE WET WEATHER SECTION.

MAINTENANCE NOTES:

1. THE ENTRANCE SHALL BE MAINTAINED IN A CONDITION THAT WILL PREVENT TRACKING OR FLOW OF SEDIMENT ONTO PUBLIC RIGHT-OF-WAY. THIS MAY REQUIRE PERIODIC TOP DRESSING WITH 2-INCH STONE AS CONDITIONS DEMAND, AND REPAIR AND/OR CLEANOUT OF STRUCTURES USED TO TRAP SEDIMENT.
2. ALL MATERIALS SPILLED, DROPPED, WASHED OR TRACKED FROM VEHICLES ONTO ROADWAYS OR INTO STORM DRAINS MUST BE REMOVED IMMEDIATELY.
3. ALL TRUCKS TRANSPORTING SATURATED SOILS SHALL BE WELL SEALED. WATER DRIPPAGE FROM TRUCKS MUST BE REDUCED TO 1 GALLON PER HOUR PRIOR TO LEAVING THE SITE.

LAST REVISION DATE: DEC 2007	
TEMPORARY CONSTRUCTION ENTRANCE (NTS)	
CARLTON, OR	DETAIL NO. 610

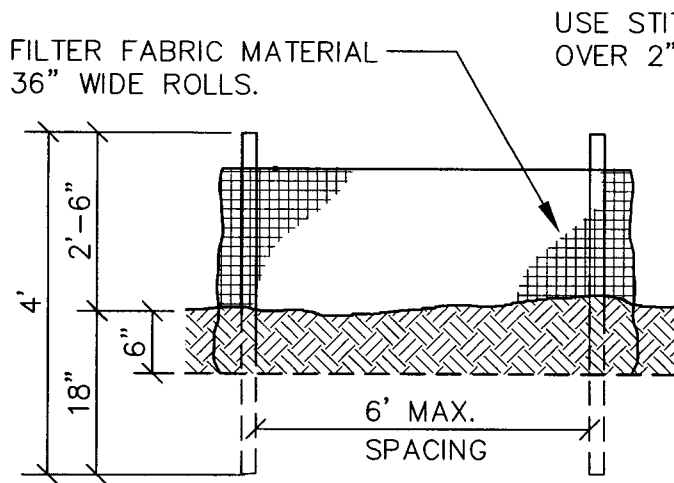


TOP VIEW

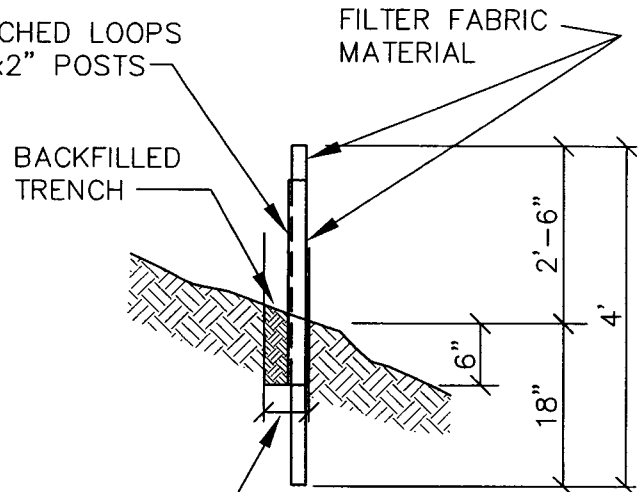
INTERLOCK 2"x2" POSTS AND ATTACH.

SILT FENCE NOTES:

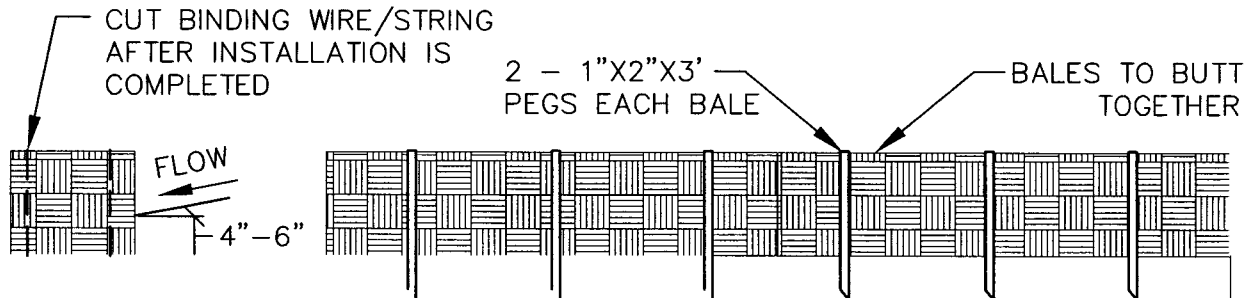
1. BURY BOTTOM OF FILTER FABRIC 6" VERTICALLY BELOW FINISHED GRADE.
2. TRENCH TO BE DUG WITH DITCH-WITCH, BY HAND OR OTHER METHOD AS REQUIRED TO MINIMIZE WIDTH.
3. BACKFILL & COMPACT NATIVE SOIL IN TRENCH AFTER FENCE INSTALLATION.
4. STITCHED LOOPS TO BE INSTALLED TO THE UPHILL SIDE OF THE FENCE.



FRONT VIEW



SIDE VIEW



STRAW BALE OR BIOFILTER BAG OPTION

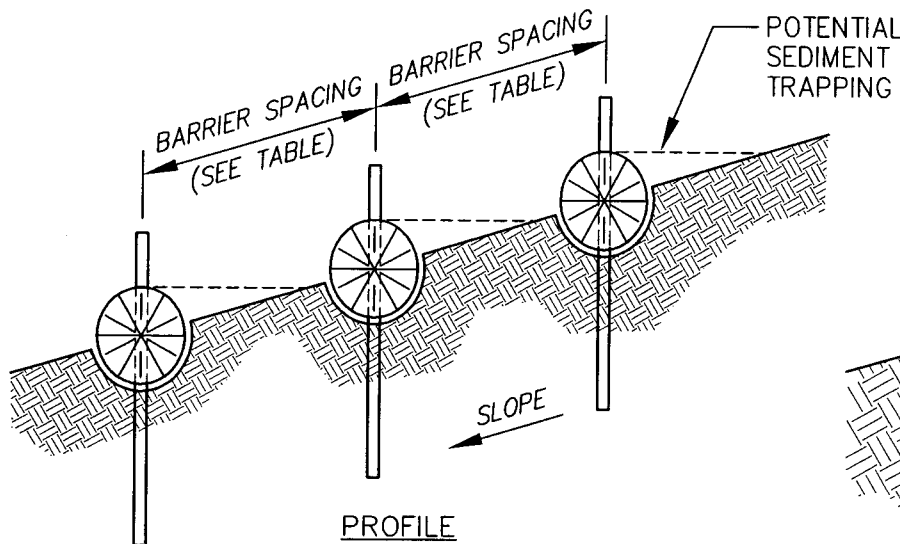
STRAW BALE NOTES:

1. EMBED BALES 4"-6" BELOW GROUND SURFACE.
2. DRIVE STAKES MIN. 12" INTO GROUND SURFACE, AND FLUSH W/ TOP OF BALES.

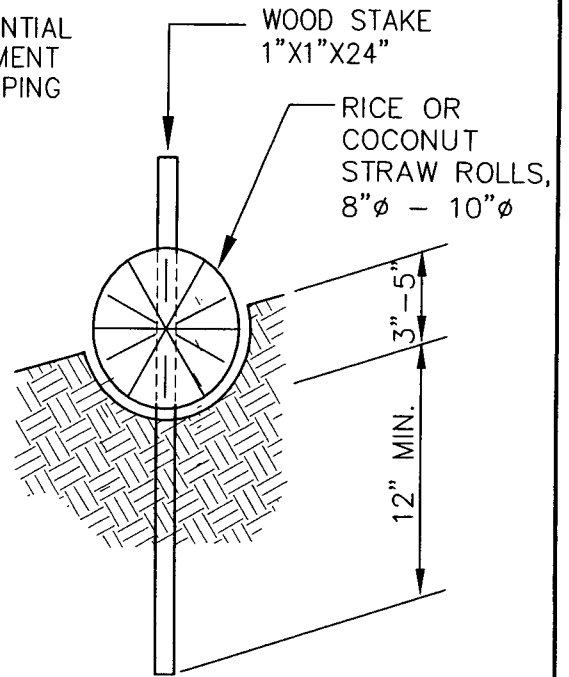
MAINTENANCE NOTES:

SEDIMENT BARRIERS SHALL BE MAINTAINED UNTIL UP-SLOPE AREA IS PERMANENTLY STABILIZED. AT NO TIME SHALL MORE THAN ONE FOOT OF SEDIMENT BE ALLOWED TO ACCUMULATE BEHIND SEDIMENT FENCES, STRAW BALES OR BIOFILTER BAGS. NEW SEDIMENT BARRIERS SHALL BE INSTALLED UPHILL AS REQUIRED TO CONTROL SEDIMENT TRANSPORT.

LAST REVISION DATE:	
DEC 2007	
SEDIMENT BARRIERS	
(NTS)	
CARLTON, OR	DETAIL NO. 611

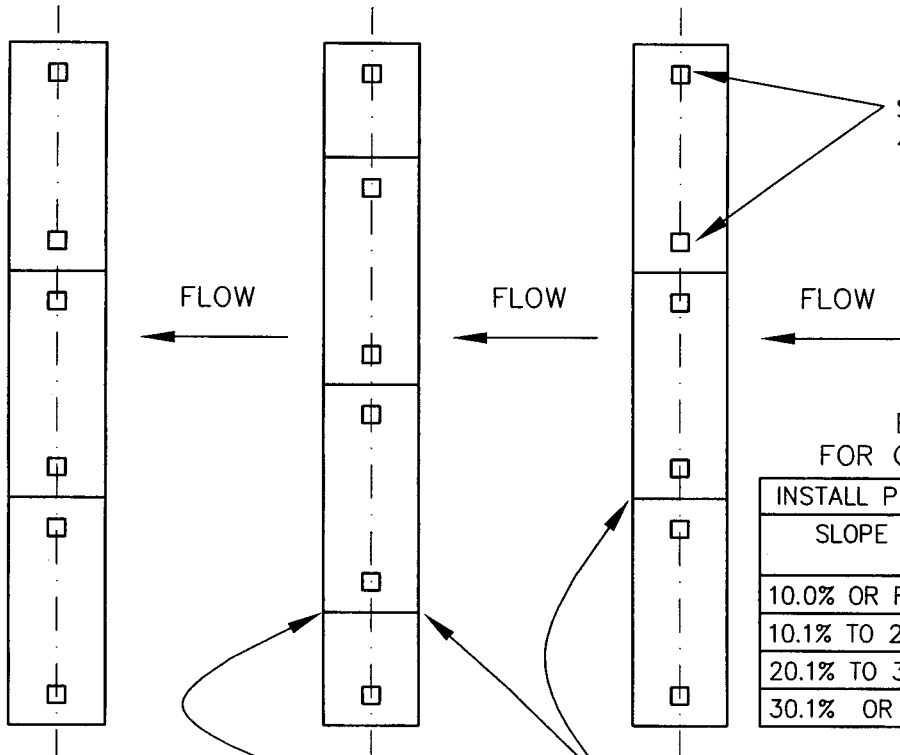


PROFILE
PLACE STRAW WATTLES PARALLEL TO SLOPE CONTOURS



SECTION

STAKE SPACING
4' MAX.



TIGHTLY ABUT
ADJACENT WATTLES

PLAN

STAGGER
JOINTS

BARRIER SPACING
FOR GENERAL APPLICATION

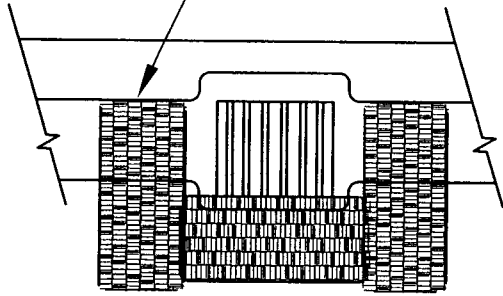
INSTALL PARALLEL TO CONTOURS AS FOLLOWS	
SLOPE RATIO	MAXIMUM SPACING ON SLOPE BETWEEN WATTLES
10.0% OR FLATTER	50' O.C.
10.1% TO 20.0%	25' O.C.
20.1% TO 30.0%	10' O.C.
30.1% OR STEEPER	5' O.C.

NOTES:

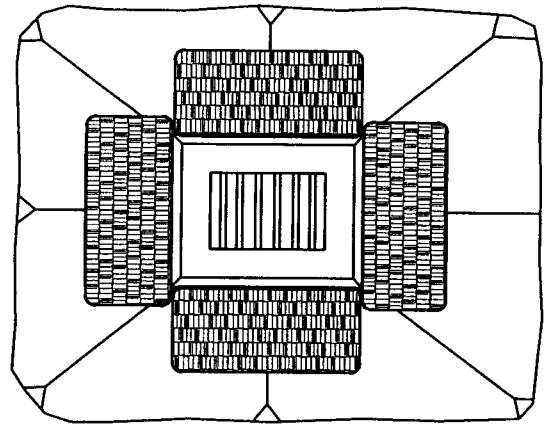
1. ALL MATERIAL SHALL CONFORM TO OSHD STANDARD SPECIFICATIONS, 1996 EDITION.
2. SEDIMENT BARRIERS SHALL BE MAINTAINED UNTIL UP-SLOPE AREA IS PERMANENTLY STABILIZED.
3. AT NO TIME SHALL SEDIMENT BE ALLOWED TO ACCUMULATE ABOVE THE TOP OF THE STRAW WATTLE.
4. NEW SEDIMENT BARRIERS SHALL BE INSTALLED UPHILL AS REQUIRED TO CONTROL SEDIMENT TRANSPORT.

LAST REVISION DATE: DEC 2007	
STRAW WATTLE SEDIMENT BARRIER	
(NTS)	
CARLTON, OR	DETAIL NO. 612

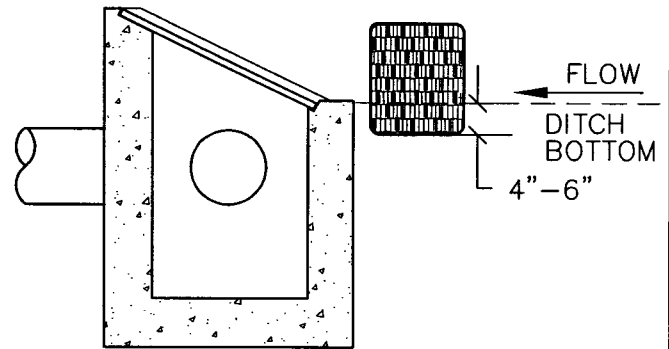
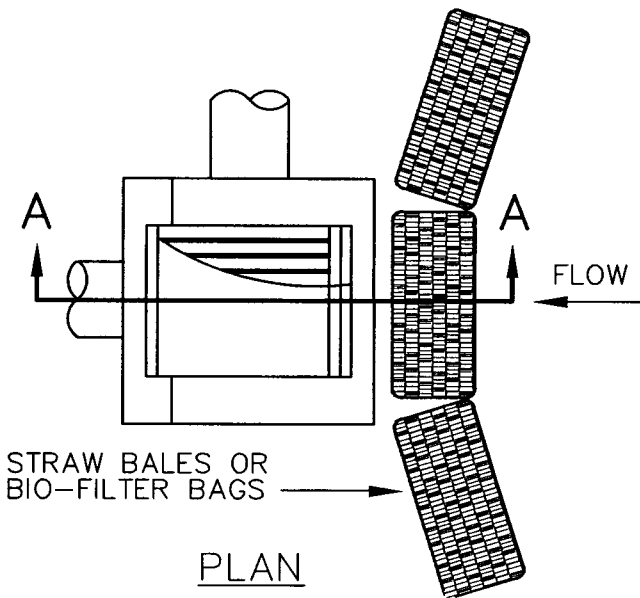
MAY BE USED SHORT TERM
W/UTILITY WORK AND WITH
PHASING OF DEVELOPMENT.



CURB INLET C.B.



AREA DRAIN



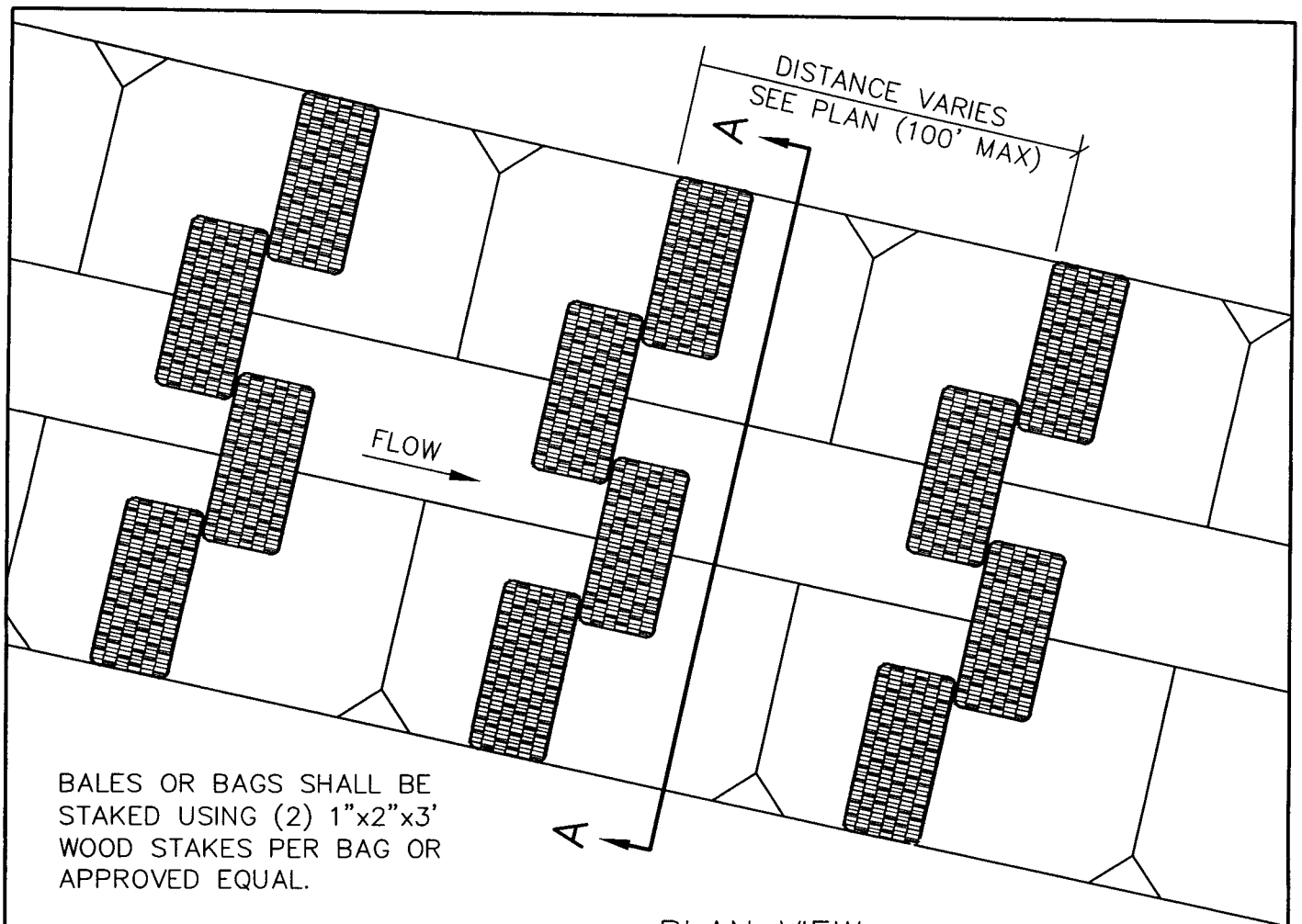
SECTION A-A

DITCH INLET C.B.

MAINTENANCE NOTES:

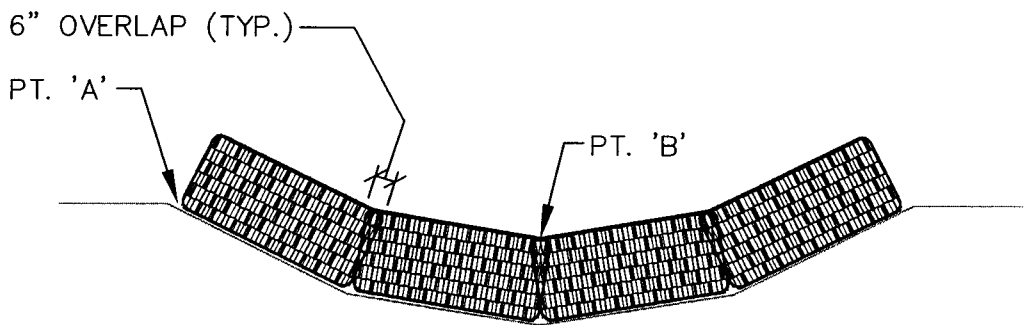
1. SEDIMENT BARRIERS SHALL BE MAINTAINED UNTIL UP-SLOPE AREA IS PERMANENTLY STABILIZED.
2. AT NO TIME SHALL MORE THAN ONE FOOT OF SEDIMENT BE ALLOWED TO ACCUMULATE BEHIND SEDIMENT FENCES, STRAW BALES OR BIOFILTER BAGS.
3. NEW SEDIMENT BARRIERS SHALL BE INSTALLED UPHILL AS REQUIRED TO CONTROL SEDIMENT TRANSPORT.

LAST REVISION DATE: DEC 2007	
INLET SEDIMENT CONTROL	
(NTS)	
CARLTON, OR	DETAIL NO. 613



BALES OR BAGS SHALL BE STAKED USING (2) 1"x2"x3' WOOD STAKES PER BAG OR APPROVED EQUAL.

PLAN VIEW

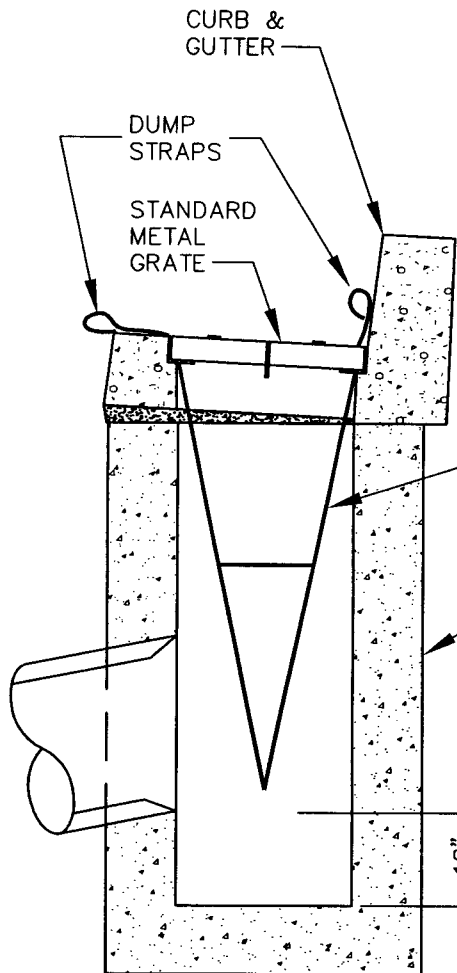


SECTION A-A

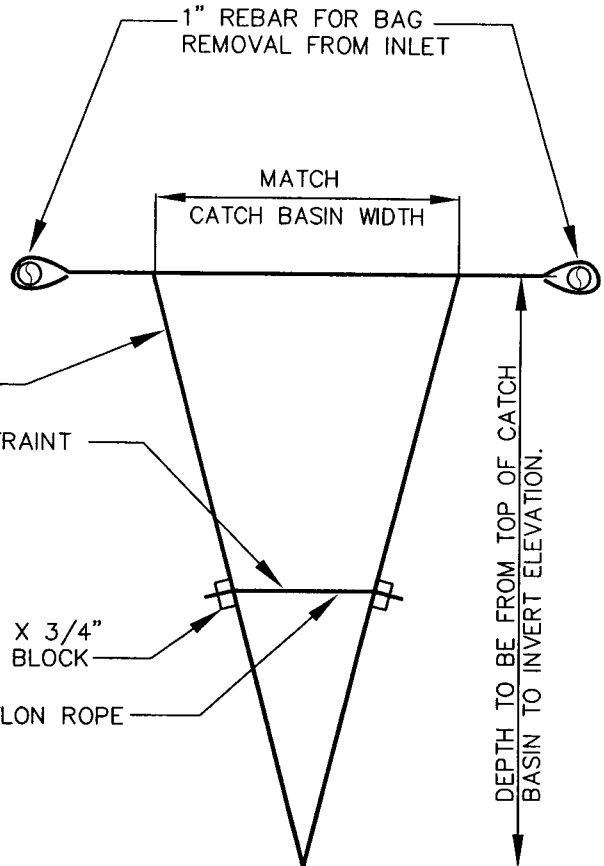
MAINTENANCE NOTES:

1. SEDIMENT BARRIERS SHALL BE MAINTAINED UNTIL UP-SLOPE AREA IS PERMANENTLY STABILIZED.
2. AT NO TIME SHALL MORE THAN ONE FOOT OF SEDIMENT BE ALLOWED TO ACCUMULATE BEHIND STRAW BALES OR BIOFILTER BAGS.
3. NEW SEDIMENT BARRIERS SHALL BE INSTALLED UPHILL AS REQUIRED TO CONTROL SEDIMENT TRANSPORT.
4. PT. 'A' SHALL BE 6" MIN. HIGHER THAN PT. 'B'.

LAST REVISION DATE: DEC 2007	
DITCH AND SWALE PROTECTION	
(NTS)	
CARLTON, OR	DETAIL NO. 614



INSTALLATION DETAIL



BAG DETAIL

NOTES:

1. EMPTY SILT SACK AS NECESSARY.
2. SILTSACK SEDIMENT CONTROL DEVICE AS MANUFACTURED BY ACF ENVIRONMENTAL AND SUPPLIED BY ACF WEST (503) 771-5115 OR APPROVED EQUAL.

LAST REVISION DATE: DEC 2007	
SILT SACK INLET DETAIL	
(NTS)	
CARLTON, OR	DETAIL NO. 615